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DEPARTMENT OF NATIONAL DEVELOPMENT

BUREAU OF MINERAL RESOURCES, GEOLOGY AND GEOPHYSICS.

RECORDS 1959 NO.93





GEOPHYSICAL SURVEY AT THE

BARRON FALLS HYDRO-ELECTRIC SCHEME, KURANDA,

NEAR CAIRNS, QUEENSLAND.

by

E.J. POLAK and P.E. MANN

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#### ABSTRACT

This report covers the results of seismic refraction and resistivity investigations on two alternative tunnel projects for the Barron Falls Hydro-Electric scheme, Kuranda, Queensland, carried out at the request of the Co-ordinator General's Department.

The nature of the subsurface rocks along the tunnel was approximately determined by combining the available geological information with the apparent resistivities and seismic velocities, as measured along the tunnel traverses. It is indicated that 1000 ft. of the projected tunnel of the scheme III may pass through, or go close underneath the weathered zone, whereas the projected tunnel of scheme X will be situated along its whole length in unweathered rock.

Seven determinations of both longitudinal and transverse seismic wave velocities were made from which Poisson's Ratio and Young's Modulus have been calculated. These showed that Poisson's Ratio in the weathered rocks (0.30 to 0.34) is higher than in the unweathered rocks (0.21 to 0.22).

A very good correlation exists between Young's Moduli and longitudinal seismic velocities, which may be used to determine Young's Moduli along the tunnel traverses. The empirical relation between the two is given by log E =  $2.18 \log V - 2.95$ .

#### 1. INTRODUCTION

The existing Hydro-Electric Power Stations on the Barron and Tully Rivers are unable to meet the power requirements of Northern Queensland and the supply is augmented by a thermal station at Townsville.

The Barron Falls power station is located about 10 miles upstream from Cairns and about 1 mile downstream from Kuranda (Plate:1). Here the Barron River descends from the Atherton Tableland in a series of falls to a deep gorge. The existing power station utilises 415 feet of the available 900 ft. through which the river falls.

The Co-ordinator-General's Office (C.O.G.) has investigated the possibility of constructing a new power station, and ten possible schemes have been considered, of which two, Schemes III and X, have been selected for more detailed investigation. Finally scheme X has been adopted.

In response to an application from the C.O.G. the Bureau of Mineral Resources, Geology and Geophysics, carried out a geophysical survey to determine the depth to and nature of the fresh rock along two tunnel lines. Seismic refraction and resistivity (constant spacing) methods were used. The survey was carried out between 1/8/58 and 10/9/58 by a geophysical party comprising E.J. Polak, party leader, and P.E. Mann, geophysicist. Field assistants were provided by the C.O.G., who also carried out a topographical survey of the traverse lines.

It is desired to acknowledge help given by officers of the C.O.G. at Kuranda and Koombooloomba and by the staff of the Cairns Regional Electricity Board.

#### 2. GEOLOGY

A general geological survey of the area was made by J.K. Brooks of the Geological Survey of Queensland (Brooks, 1957). The results of this work are shown on Plate 2. A further detailed examination of the outcrops of rocks along the two tunnel lines was carried out by F.J. Carter (Carter, 1958). The result of this work forms the basis for correlating seismic velocities with rock type as shown on Plates 5, 6, 7, 8, 11, 12 and 13. Six diamond drill-holes were put down, the sections of which are shown on Plates 4 and 10.

The terrain is covered by rain-forest and except for the Barron River and a few creeks, the exposure of rocks is limited to road and railway cuttings (Plate:2).

The rock throughout the area is of Lower Palaeozoic age and is mostly metamorphosed. The principal rock-types are meta-greywacke, quartzite, slate and phyllite. The following summary is based on analyses of thin sections of rocks carried out by R.M. Tucker of the Geological Survey of Queensland (Brooks, 1957).

Meta-greywacke: The rock is slightly metamorphosed, clearly showing sedimentary characteristics. It is generally fine-grained, the grains being subangular. The rock is well-cemented, hard and massive. There are

two types of meta-greywacke recognised in the area - the lithic greywacke, containing quartzite, slate and chert, and the felsphatic greywacke, containing plagioclase, microcline and orthoclase, indicating that it is partially derived from granitic rock.

Quartzite: Quartzite forms only a small part of the sedimentary succession, apart from a thick layer near the Hydro sidings. This bed which is intensely folded and interbedded with slate shows an apparent thickness of 1000 ft. The quartzite is white to brown in colour and the grain size is very fine. Certain horizons of quartzite contain manganese.

Slate and phyllite: These rocks weather easily, except when thinly interbedded with other rocks. They are very poorly cleaved and are often silicified. Below the weathering zone the rock is strong and solid.

Epidote-Chlorite Rock: This rock occurs only on the Cairns-Kuranda railway near Robbs Monument. In addition to epidote and chlorite, a high proportion of magnetite is often present. The rock is hard and massive.

Meta-conglomerate: This consists of pebbles of quartzite in a chlorite or phyllitic matrix. It occurs in the Barron Gorge, where it is interbedded with greywacke.

All the above mentioned rocks are very hard, tough and massive when located below the zone of weathering. The jointing is well-developed with three main directions - two vertical and one horizontal.

The regional strike is north-north-west and the regional dip is approximately 70 degrees west-south-west to vertical. Evidence of folding is confined to the quartzite horizons.

## 3. METHODS AND INSTRUMENTS

## (A) Seismic refraction.

The seismic method of exploration depends on the contrast in the velocity of elastic waves through different rock formations. As regards the physical laws involved, seismic phenomena are comparable with optical phenomena, in that they deal with a type of energy propagated in waves.

The seismic recording equipment used in the survey was a 12-channel reflection-refraction seismograph manufactured by the Mid-Western Geophysical Laboratory, Tulsa, Oklahoma. "Mid-western" geophones having a natural frequency 8 c.p.s. were used to detect longitudinal waves and "S.I.E." geophones having a natural frequency 6 c.p.s. were used to detect transverse waves.

The seismic refraction method was used in this survey. When an explosive charge is fired in a shallow hole the seismic waves radiate in all directions. Geophones placed on the ground detect these waves and send a

corresponding electric impulse along a cable, to be amplified and photographically recorded by an oscillograph.

The waves are identified by their different characteristics, longitudinal and transverse waves alone being considered in this report. The fastest wave arriving at the geophone is the longitudinal or compressional wave which consists of a forward and backward movement of particles along the direction of propagation. The transverse or shear wave is the second to arrive at the geophones and consists of a movement of particles normal to the direction of propagation. Both waves undergo refraction, reflection or may change from one type to another at the discontinuity. Transverse waves may be polarised.

Fig. 1 shows a sample record obtained during the survey. The vertical lines are timing lines at two millisecond intervals. The shot instant "SI" is shown on trace No. 6. Following the shot instant the deflections on the traces indicate the arrival. times of the long-itudinal waves, which may be measured relative to the shot instant. The arrival time of the longitudinal wave at each geophone is plotted against the distance of the geophone from the shot-point thus giving a Time-Distance curve. Fig. 2a shows the time-distance curve derived from Fig. 1. The slope of the curve is the reciprocal of the seismic wave velocity.

Fig. 2b represents a typical four layer instance with parallel, horizontal layers in which the seismic velocities are laterally constant. In this instance, the velocities indicated by the time-distance curve equal the true velocities through the layers. However, where the thickness of the layers increases away from the shot point, the down dip velocity on the time-distance curve is lower than the formation velocity and vice versa.

An analysis of the paths of various seismic rays of the longitudinal wave indicates the following pattern for the first impulse recorded by the successive geophones (see fig:2a & b).

- (i) Geophones G1 and G2 indicate the velocity  $V_1 = 1600$  ft/sec. The seismic wave travels from the shot-point A to point B with velocity  $V_0$  and thence along the refractor with velocity  $V_1$ . The position of point B is determined by the critical angle of incidence iO1 where sin iO1 =  $\frac{V_0}{V_1}$ . As this refracted wave travels along the refractor, part of its area.
- refractor, part of its energy is continually refracted back at the critical angle iO1. This wave reaches geophone  $G_1$  through point C.
- (ii) Geophones  $G_3$  to  $G_6$  indicate a velocity of  $V_2 = 3200$  ft/sec. The wave travels from point A to point  $B_1$ , with velocity  $V_0$  then to point D with velocity  $V_1$ . At point D it is critically refracted and travels with velocity  $V_2$  to point E, A refracted wave from point E travels to point  $C_1$ , with velocity  $V_1$ , and thence to geophone  $G_3$  with velocity  $V_0$ .
- (iii) Geophones  $G_7$  to  $G_{12}$  indicate a velocity of 15000 ft/sec. It is a wave which penetrates still deeper and it is refracted from the fourth horizon.

Where the bedrock is formed by rocks of different seismic velocity, the geometry of the paths of the seismic waves is more complicated than is shown on Fig. 2b. Fig. 3 shows an instance where a bed of uniform thickness and uniform velocity  $V_{\rm O}$  overlies strata of varying velocities with vertical boundaries at which  $V_{\rm O}$   $<\!V_{\rm 1}<\!V_{\rm 2}<\!V_{\rm 3}$ . When a time-distance curve for the seismic waves is plotted the points do not fall on one line. The refractor velocity indicated by the time-distance curve is usually corrected where the upper layer is not of uniform thickness. If, as indicated in Fig. 3, the vertical layers (e.g.  $V_{\rm 1}$  &  $V_{\rm 2}$ ) are too thin in relation to the geophone spacing, no accurate velocity measurements can be made. For instance in Fig. 3, the velocities  $V_{\rm 2}$  &  $V_{\rm 1}$  cannot be deduced from the data recorded by geophones  $G_{\rm 2}$ ,  $G_{\rm 3}$ ,  $G_{\rm 4}$  and  $G_{\rm 5}$ . However,  $V_{\rm 3}$  can be deduced from data recorded by geophones  $G_{\rm 5}$ ,  $G_{\rm 6}$  and  $G_{\rm 7}$ . It follows that for any velocity measurements applicable to one medium at least three points on the corrected time-distance curve must be located on a straight line, as for instance x, y and z (Fig. 3).

The field arrangement and corresponding calculation method known as the "Method of Differences" was used. (Heiland, 1946 p. 548). The technique is illustrated in Fig. 4. A shot is fired at point A and the travel times are recorded at points B and C. A shot is then fired at point C and the travel times recorded at points A and B. The depth d measured to the refracting interface normally below point B is calculated from the formula:

$$d = \frac{1}{2} (T_{AB} + T_{CB} - T_{AC}) V_a$$

where

 $T_{AB}$ = time of travel of refracted wave from A to B.

$$T_{CB}$$
= " " " " C to B.

V<sub>a</sub> = the apparent velocity of the seismic wave within the overlying rocks.

The following types of spreads were shot:

- (a) Weathering spreads: These were used to obtain the thickness and seismic velocity of the soil and surface layer. Geophones were spaced 10 ft. apart and shots were fired 5 and 100 ft. from each end of the spread and in line with it.
- (b) Normal spreads: Geophones were spaced 25 or 50 ft. apart. Shots were fired 25 ft. and 250 ft. or more from each end of the spread and in line with it.

In engineering seismic investigations the velocities of longitudinal and transverse waves can frequently be correlated with and used to identify or indicate different rock-types, degree of weathering jointing or shearing near the surface.

Theoretically, the velocity of longitudinal and transverse waves in elastic media is given by the following formulae (Leet, 1950 pp. 45/46.

$$V_{L} = \frac{1}{12}$$
  $\sqrt{\frac{E}{\delta}} \frac{1 - \sigma}{(1 + \sigma)(1 - 2\sigma)}$ 
 $V_{t} = \frac{1}{12}$   $\sqrt{\frac{E}{\delta}} \frac{1}{2(1 + \sigma)}$ 

where

V<sub>L</sub> = longitudinal velocity is ft/sec.

V<sub>t</sub> = transverse velocity in ft/sec.

E = Young's Modulus in 1b/sq.in

σ = Poisson's Ratio.

 $\delta$  = density  $\frac{1b \sec^2 in^{-1}}{in^3}$ 

It is therefore possible to calculate from the longitudinal and transverse velocities all the dynamical properties of the rock.

G = Modulus of rigidity (shear modulus) in lb/sq.in.

B = Bulk modulus (incompressibility) in lb/sq.in.

$$\left(\frac{v_L}{v_t}\right)^2 = \frac{\sigma^{-1}}{\sigma^{-\frac{1}{2}}}$$

$$E = (12V_L)^2$$
,  $\delta$ ,  $(1 + 6)$   $(1 - 26)$ .

$$G = \frac{E}{2(1+\sigma)}$$

$$B = \frac{E}{3(1-20^{\epsilon})}$$

#### B Drill-hole velocity logs

Seismic velocities were recorded in two drill-holes on Traverse X. A small explosive charge was detonated at several depths in the drill hole and the resulting waves were detected by geophones at the ground surface adjacent to the drill-hole. Longitudinal and transverse waves were recorded and from the travel times and the depth of the shot vertical velocities were calculated. These velocities are measured in a direction approximately normal to the direction in which velocities are measured in seismic refraction work. Where the vertical velocity differs, in some cases, from the velocities obtained from weathering and normal spreads, the differences may be accounted for by anisotropy of the rock, associated with depositional planes, folding, faulting and jointing.

## (C) Resistivity.

Differences in the structure and composition of rocks produce variations in their electrical resistivity. Hard non-porous and unweathered rocks as a rule have a high resistivity. Shearing and fracturing result in localised weathered zones which in turn usually give rise to a relatively high salinity in the contained water and thus produce a corresponding relatively low resistivity. In general, it may be said that the resistivity of a rock is inversely proportional to the product of its porosity and the salt content of the solution in the pores.

The equipment used was the Megger Earth Resistivity Tester manufactured by Evershed and Vignoles Ltd., London.

The Wenner arrangement of electrodes was used for measuring resistivity. This is illustrated in Fig. 5. Four electrodes are equally spaced along a straight line. An alternating current is introduced into the ground through the current electrodes, C<sub>1</sub> and C<sub>2</sub>, and the difference in potential is measured across the potential electrodes, P<sub>1</sub> and P<sub>2</sub>. The depth of penetration of the current is controlled by the distance "a" between the electrodes. The technique used in this survey is known as resistivity traversing (Heiland 1946 page 644 et seq.). The four electrodes are moved as a unit along a traverse, centred on consecutive stations where readings are taken. In the interpretation, absolute values of resistivity are not so important as sudden changes, which usually indicate a change in rock type, such as a change from unweathered rock to sheared or fractured and weathered rock or a change from sandstone to shale.

In this survey two constant electrode spacings namely 50' and 100' were used.

#### (D) Magnetic method.

In the Barron Falls area short magnetic tests were carried out on proposed schemes III and X.

The measured magnetic intensity at any point on the earth's surface may be expressed as a vector. As such, it is mainly the resultant of two vectors, an induced magnetic intensity vector in the direction of the earth's magnetic field and a remanent magnetic intensity vector inherent to the rock, which may lie in any direction. Magnetic measurements may indicate, in certain areas, such features as faults and boundaries between near-surface formations and it is sometimes also possible to obtain rough depth estimates from these measurements.

The test in the Barron Falls area indicated that no useful information could be obtained by the use of this method, since apparently the difference in the magnetic susceptibility of the various rocks in the area is not sufficient to differentiate between them.

A Watts vertical force variometer, manufactured by Hilgar and Watts Ltd., London, was used.

Table 1 shows the total length of traverses covered with each geophysical method -

### TABLE 1.

Me thod	Scheme III	Scheme X	Total
Seismic refraction	7000 ft.	7450 ft.	14450 ft.
Seismic hole logging	-	197 ft. 2 holes	197 ft. 2 holes
Resistivity	9300 ft.	5350 ft.	14650 ft.
Magnetic	1500 ft.	1200 ft.	2700 ft.

## 4. SCHEME III

## (A) Lay-out. (Plate:3).

It is proposed in this scheme to divert water from the Barron River at a point near Fairyland through 9500 ft. of tunnel driven under the Macalister Range and thence through penstock pipes down to a power-station located on Deep Creek. The gross-head would vary with the height of the proposed weir, but would be at least 918 feet.

#### (B) Geology.

The geological information along the tunnel lines is very scarce. A few outcrops have been examined by F.J. Carter and the results of this examination are shown on Plates 5 to 8. A general prediction of rock types along the tunnel line (Brooks, 1957) is as follows:

- Stations 0 to 40 Meta-greywacke, fine to medium grained with slate bands.
- Stations 40 to 120 Slate, phyllite and interbedded greywacke. Three drill-holes have been drilled along this section and indicate that the tunnel would be driven in fresh rock, below the weathered zone (see Plate:4).
- Stations 120 to 140 Quartzite containing beds of slate and phyllite.
- Stations 140 to 195 Meta-greywacke with bands of quartzite and slate.

The beds dip steeply between 70 and 90 degrees, and the strike is nearly at right angles to the direction

of the traverse.

## (C) Results.

Seismic refraction method was used on Scheme III between stations 0 to 140. Further north the proposed tunnel of Scheme III goes deep into the rocks, so that there is no danger of the tunnel being in weathered rocks. The results of the seismic work are shown on Plates 5 to 7. The results of the constant electrode spacing resistivity survey are shown on Plates 5 to 8. The whole length of the proposed tunnel of Scheme III was surveyed using resistivity method.

## (i) Depth determination.

The results of the seismic survey were interpreted as a four-layer structure as illustrated by Figure 2b.

Top layer. This is interpreted as soil with a velocity of around 1000 ft/sec. The layer seems to be thin on sections where it was measured. The maximum thickness of 10 ft. was found near station 14, but close to stations 85 and 132 the top layer appears either to be absent altogether or to be too thin to be recorded.

2nd layer. This is interpreted as completely decomposed to highly weathered rock with seismic velocities of from 1400 ft/sec. to 2800 ft/sec. This layer reaches (where measured) a maximum depth of 50 ft. on the section between stations Nos. 40 and 50.

3rd layer. This is interpreted as a weathered rock, with seismic velocities from 3200 ft/sec. to 5800 ft/sec. The maximum estimate of depth to its base is 98 ft. at stations Nos. 54 and 56.

4th layer. This is interpreted as fresh rock with a seismic velocity of from 10000 ft/sec. to 20000 ft/sec.

The error in depth determination is considered to be less than ±20 per centum of the true depth. This estimate is based on experience of results in other areas where comparable geological conditions exist. Although the error in absolute depth may be fairly large the error in relative depth of fresh rock from station to station along the traverse is likely to be much smaller and the profile shown for the surface of the fresh rock is expected to be quite accurate.

#### (ii) The character of the fresh rock.

The detailed interpretation of the rock types below the weathered zone is shown on Plates 5 to 7.

Table 2 shows the interpretation rules used to deduce the nature of the fresh rock from the apparent resistivity and the seismic velocity in the lowest refractor.

The rules were obtained by reviewing the geophysical data with the known geological data.

### TABLE 2.

Rock type	Resistivity in ohm/cm.	Seismic velocity in ft/sec.
Slate	below 20000	below 14000
Meta-greywacke	20000 to 60000	over 14000
Quartzite	over 60000	over 14000

The interpretation rules shown on Table 2 must be used cautiously for the following reasons:-

- (a) In the low lying parts of the traverse, meta-grey-wacke may have been mis-named as slate because the increased moisture content of the near surface layers may have lowered the apparent resistivity to below the 20000 ohm/cm limit.
- (b) Narrow, low resistivity sections, which are interpreted as slates, may represent a shear zone, through which water penetrates deeper into the strata.
- (c) The position of the boundary between rock types is approximate, as can be understood from the sections on seismic and resistivity methods.

Plate 8 shows a geophysical interpretation based on resistivity traversing and reconnaissance geological mapping only.

As an important result, the survey indicates that, between stations 20 and 30, and between stations 67 to 79 (plates 5 and 6) the proposed tunnel may pass through weathered rock, or may pass close to it.

#### 5. SCHEME X

## (A) Lay-out. (Plate:9).

In scheme X it is proposed to direct water from the Barron River at the existing pondage weir through 5350 ft. of tunnel, thence through a pressure tunnel down to an underground power station at the bottom of the gorge near Surprise Creek. The gross-head would be at least 933 ft.

#### (B) Geology.

A few rock outcrops have been examined by F.J. Carter and the results of this examination are shown on

Plates 11 to 13.

A general prediction of rock types along the tunnel-line is as follows (Brooks, 1957):-

Stations 0 to 24 - Slate and phyllite with interbedded greywacke and quartzite.

Stations 24 to 66 - White and brown quartzite with some slate and phyllite bands.

Stations 66 to 107 - Fine grained meta-greywacke with interbedded slate and quartzite.

Massive epidote-chlorite rock to be encountered near the outlet of the tunnel in the Robb's Monument area.

The beds dip between 70 to 90 degrees and the strike is nearly north-south, thus cutting the proposed tunnel-line at a slight angle (see Plate 2).

#### (C) Results.

The results of the seismic work are shown on Plates 11 to 13. The results of constant electrode spacing resistivity survey are shown on Plates 11 and 12.

## (i) Depth determinations.

The following layers with different velocities were recognised along the traverse (see Fig. 2b).

Top layer. Soil with a seismic velocity of about 1000 ft/sec. This layer seems to be thin (where measured) and on some sections completely missing. Vertical sections of this layer exposed in the railway cutting show non-uniform thickness merging gradually with the next layer.

2nd layer. Highly weathered rock containing some pieces
of the original rock. This latter has a seismic velocity
of from 1400 to 3100 ft/sec.

 $\underline{3rd\ layer}$ . Partly weathered rock with a seismic velocity of up to 8000 ft/sec.

4th layer. Fresh rock with a seismic velocity of from 15000 to 20000 ft/sec.

The velocities listed above were determined on weathering and normal spreads along a direction corresponding roughly with the direction of the strike of the rocks. The velocity of the longitudinal wave was measured in two drill-holes, the results of these measurements are shown on Table 3.

#### TABLE 3.

Drill Hole No.	Depth	Time millisecond	Average velocit <b>y</b> ft/sec	Depth interval ft	Time interval millisecond	
9	147	35	4200	۲0	1, 1	11200
	97	30 <del>2</del>	3200	50	72	***
1	THE RESERVE AND ADDRESS.			17	2	8500
	80	28글	2800	30	3 <u>1</u>	8500
	50	25	2000		/	2800
	34	19	1800	16		2000
	J1		1000	9	5	1800
	25	14	1800			
6	50	18	2770			

The seismic velocity is different when measured vertically and horizontally in sedimentary rocks. The vertical velocity in fresh rock determined on normal spreads exceeds that in borehole No. 9. This may be because the horizontal velocity was measured along the strike of the strata, while the vertical velocity was measured across the stratification planes. In sedimentary rocks the seismic velocity generally has the highest value parallel to the bedding planes of the rock.

To calculate the depth to fresh rock an apparent velocity obtained from weathering and normal spreads was used. The locations where an apparent velocity was obtained are indicated on the cross-section (Plates 11 and 12) as sub-divisions of the overburden. The apparent velocity compares well with the velocity obtained in drill-holes.

The error of the determination of the depth to fresh rock is expected to be within ± 15 per centum. This estimate is based on the results of other surveys where similar geological conditions exist. However, the accuracy may be substantially lower on the section from station 85 to station 101. Over this section shots were fired only on one side of the spread so as to avoid firing too close to Kuranda-Cairns railway line. When shots are fired from one direction only it is necessary to assume the seismic velocity of the rock to calculate the thickness of overburden. A small error in the assumed value of the velocity can produce large errors in final results.

The survey shows that over its whole length the tunnel in Scheme X will be well below the weathered zone.

Table 4 compares drilling data and seismic data in three drill-holes along the traverse line of scheme X.

#### TABLE 4.

Drill-Hole No.		n feet ontinuity drilling	Character of the rock below discontinuity (after F.J. Carter)
6	37 ,	40	Solid fresh rock, some iron-stained joints (Pelitic Phyllite, Green-schists)
9	62	64	Moderately to slightly weathered slates and phyllites with metagreywacke at 69-70'. Manganese stained joints and quartz stringers.
7	110	114	Phyllite (fresh) and slate with joints.

## (ii) The character of the unweathered rock.

The interpretation of the probable rock types below the weathered zone is shown on Plates 11 to 13. The method of interpretation of geophysical data as described for scheme No. III was used here and the values of seismic velocity and resistivity, as listed on Plate 2, served as the basis for this interpretation. Since the strike of the rocks is more parallel with the traverse, the limiting velocity of 15000 ft/sec. for meta-greywacks and quartzite was accepted. All the other limitations apply here as well.

The interpretation of rock types on the pressure tunnel line (Plate:13) is based on seismic velocities and the projection of geological boundaries noted in the railway cutting and Barron River Gorge.

## (iii) The dynamic properties of the rocks.

Table 5 shows the values of the dynamic properties of the rocks, calculated from the longitudinal and transverse wave velocities obtained in drill-holes and on normal spreads. The formulae used were given earlier in the report (part 3A).

Many rocks show velocity anisotropy. The velocity measured for a rock will depend on the direction in which the velocity is measured relative to the bedding or stratification planes. Hence it may be expected that the dynamic elastic constants of the rock vary in the same way. This means that the dynamic constants given in Table 5 for D.D.H.No.6 and D.D.H.No.9 are only valid for a direction parallel to the axis of the borehole.

Location	Dep-					sverse Interval Velocities			E		\ B		G		
•	th	in	Velo- city ft/sec	in	city ft/sec	in	in	0	10 <sup>6</sup> lbs /in <sup>2</sup>	10 <sup>11</sup> dyne /cm <sup>2</sup>	10 <sup>6</sup> lbs /in <sup>2</sup>	10 <sup>11</sup> dyne /cm <sup>2</sup>	10 <sup>6</sup> lbs /in <sup>2</sup>	10 <sup>11</sup> dyne /cm <sup>2</sup>	
Drill- Hole No.9	147	35	4200	65	2250	11000	6700	.21	3•5	2.4	2.05	1.4	1.46	1.0	
	80	29	2750	55	1560		9300	5600	•22	2.5	1.72	1.6	1.1	1.02	0.7
	52	26	2000	50	1080		1240		0.15	:	<u>.</u> 1		0.05	0.04	
	36	20	1800	37	970										
Drill- Hole No.6	50	17	2950	35	1415			•325	0.18	0.12	0.17	0.11	0.07	0.05	
							:	: ! <del>!</del>			! !			· · · · · · · · · · · · · · · · · · ·	
Intake Tunnel X			7700	•	3600			.325	1.34	0.92	1.28	0.88	0.51	0.35	
Near Collin's Bridge	and the second s		6500 8300		3500 4300			.30 .31	1.02				0.39	0.27 0.43	

Table 5 shows that Poisson's Ratio in Weathered rocks tends to be higher than in unweathered rocks (0.21 and 0.22 in unweathered, 0.30 to 0.33 in Weathered rocks). In Fig. 6 Young's Modulus E is plotted against Poisson's Ratio for E between 0.15 x 10 and 3.5 x 10 lb/sq.inch. The figure clearly indicates the higher Poisson's Ratio corresponds with the lower Young's Modulus and vice versa. The relation may be represented by the empirical formula, in the range of E between 0.15 & 3.5 x 10 lb/sq.inch by:-

$$\sigma = -0.05E + 0.36 \pm 0.02$$

On Fig. 7 the values of Young's Modulus are plotted against the longitudinal velocity. Curve A was derived from data scattered in the literature in 1956. Curve B, which is compiled from the data of this survey, is similar to curve A.

Curve B can be used to evaluate Young's Modulus of the rock from the seismic velocities of longitudinal waves recorded on the traverse and shown on Plates 5, 6, 7, 11, 12 and 13.

The empirical formula representing curve B is :-

 $E = 0.00113v^{2.18}$ 

or  $\log E = 2.18 \log V - 2.95$ 

in which E is in lb/sq. inch and V in ft/sec.

### 6. CONCLUSIONS

The geophysical survey provided information on the depth to the bedrock along the tunnel lines of the alternative schemes. The overburden consists of soil, decomposed and highly-weathered, to less-weathered rock. The bedrock is of fresh but jointed rock.

Sections of the proposed tunnel line of scheme III will be situated within the weathered zone while the proposed tunnel line of scheme X is situated well below the weathered zone.

An attempt was made to deduce the character of fresh rock from seismic velocities and apparent resistivity values.

The dynamic elastic properties of the rocks have been calculated from longitudinal and transverse velocities. It was shown that Poisson's Ratio for the unweathered rocks in the area is about 0.21 to 0.22, and for weathered rocks between 0.30 and 0.33.

Fig. 7 shows a relation between Young's Modulus and seismic longitudinal wave velocity, which can be used to derive the value of Young's Modulus for the rock along both tunnel lines.

#### 7. REFERENCES

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Heiland, C.A. 1946	Geophysical Exploration. Prentice Hall, New York.
Leet, L.D., 1950	Earth Waves, Harvard University Press, Cambridge, Mass.
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Power Generation. Dept. of the Co-ordinator General of Public

Works, Queensland.

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ny Records 1959, No. 9.

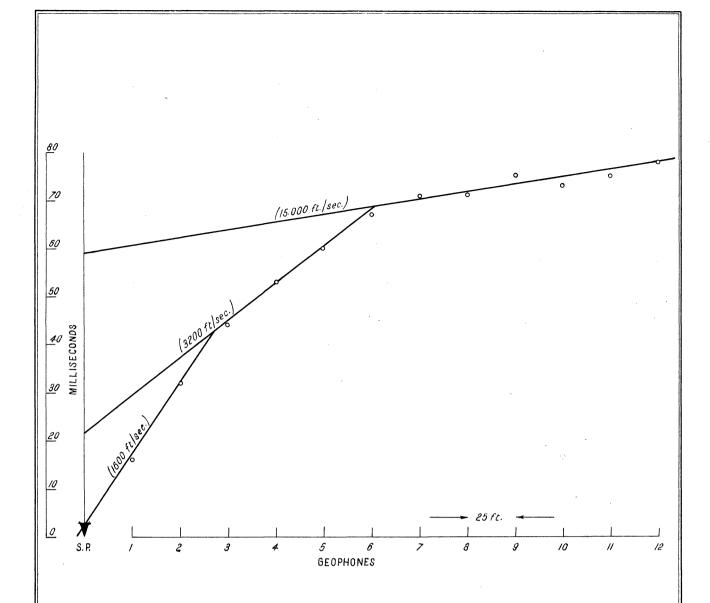


FIGURE 2a.

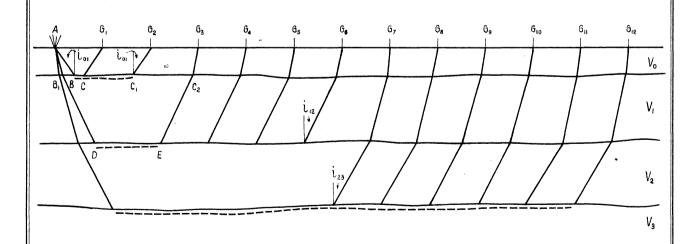


FIGURE 2b.

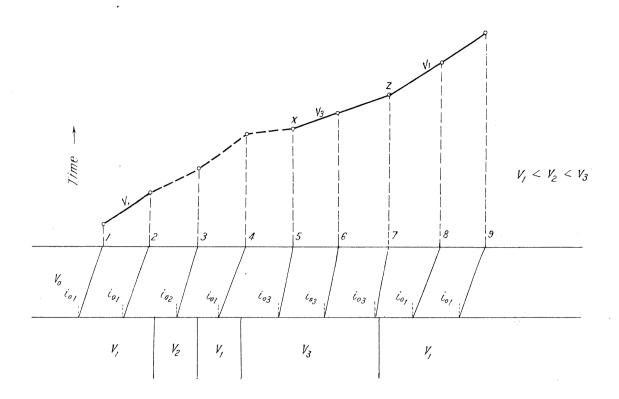


FIGURE 3. REFRACTOR WITH DIFFERING VELOCITIES

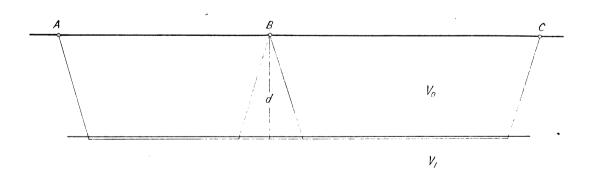


FIGURE 4. METHOD OF DIFFERENCES

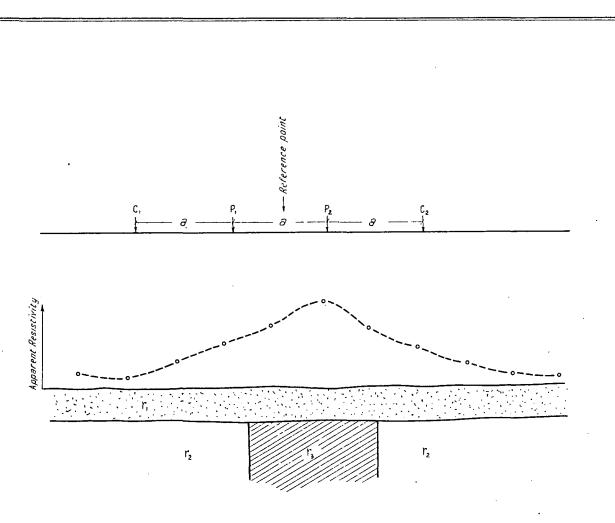
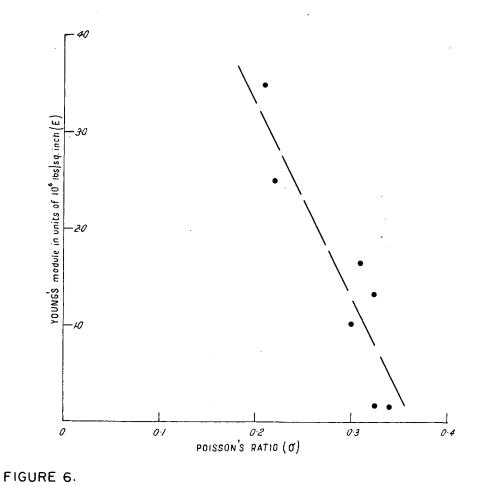
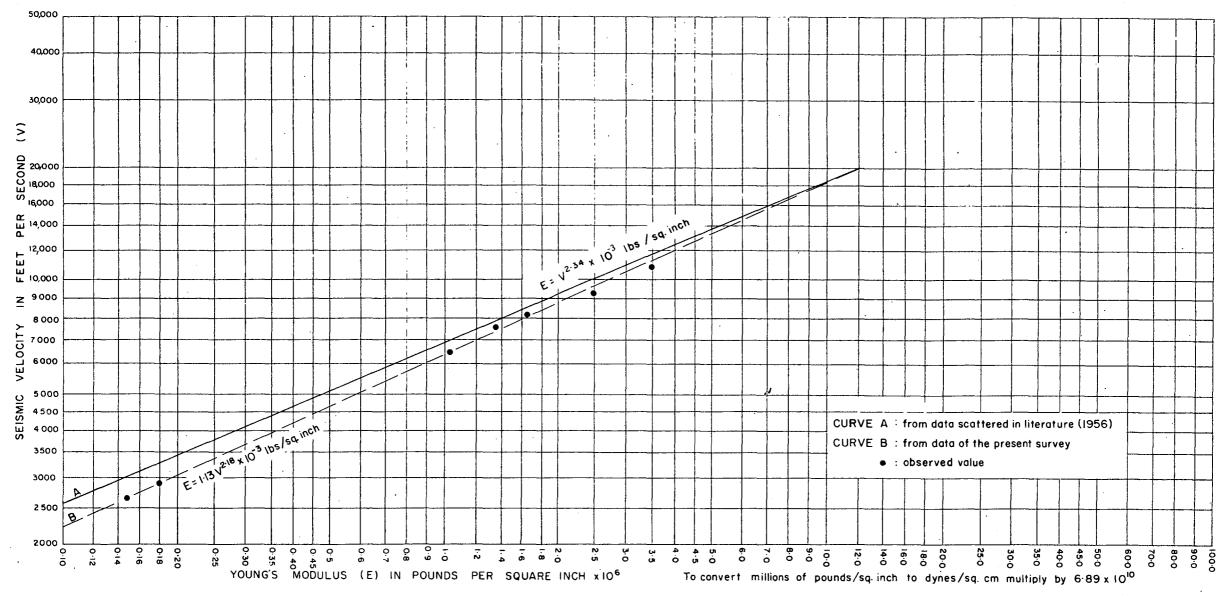


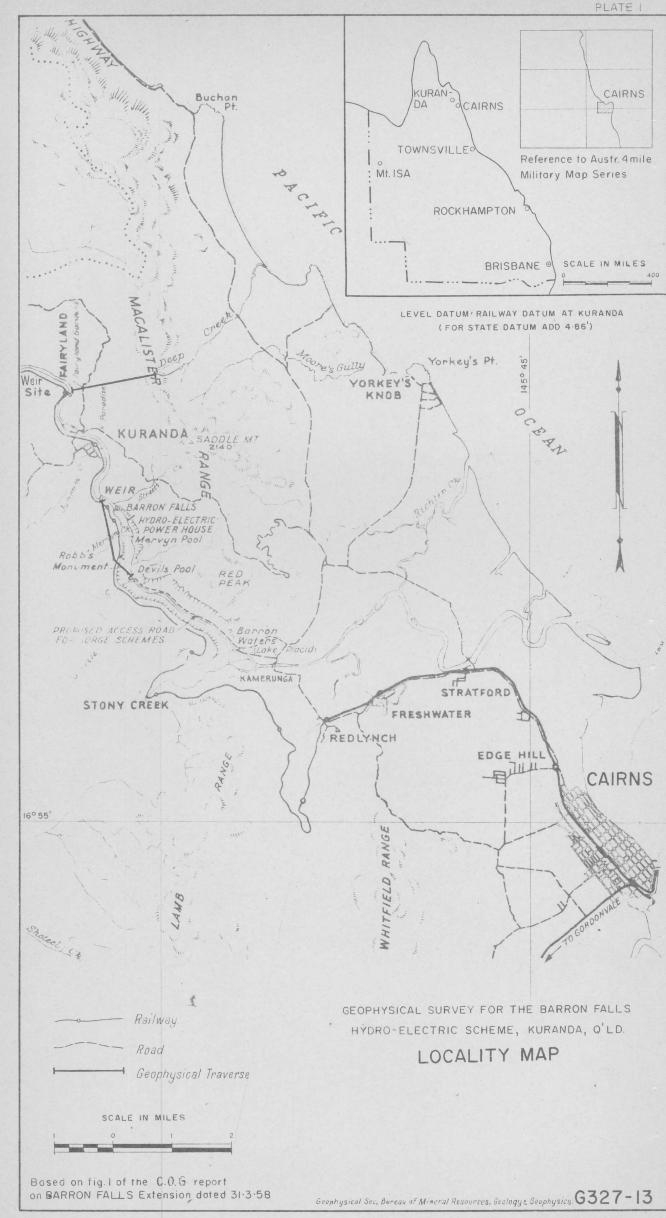
FIGURE 5. Wenner arrangement of electrodes and apparent resistivity curve over a high resistivity bed.

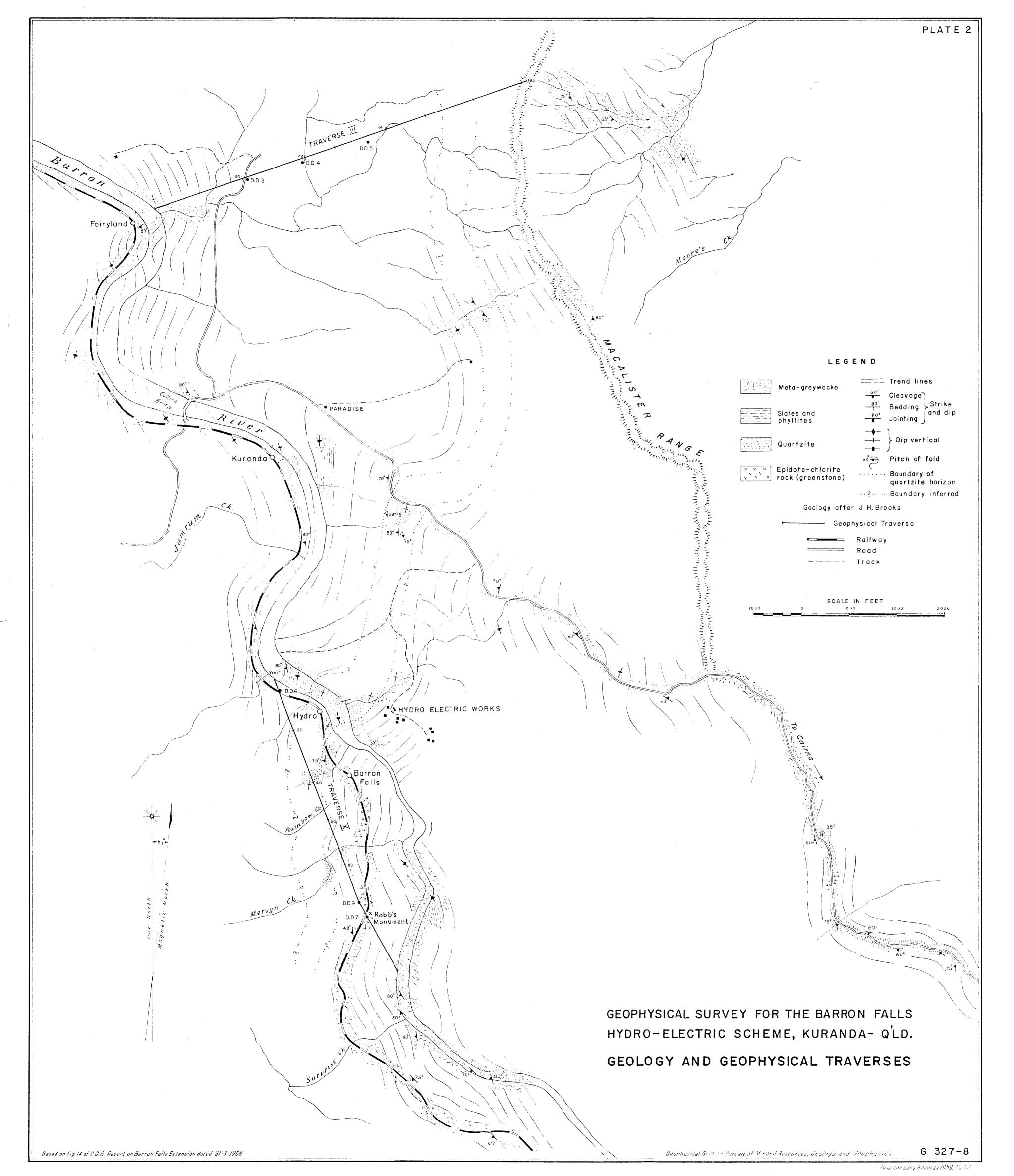


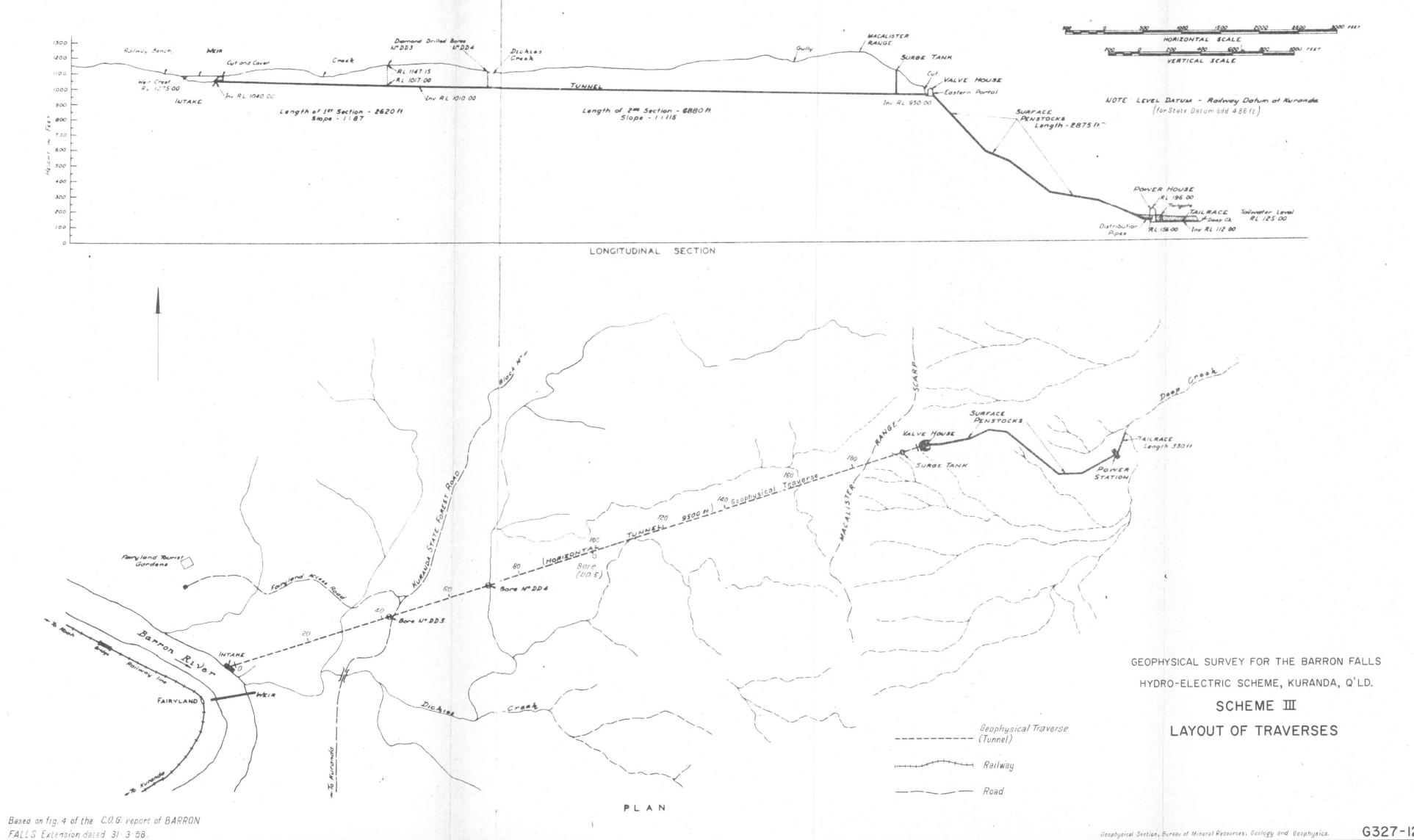


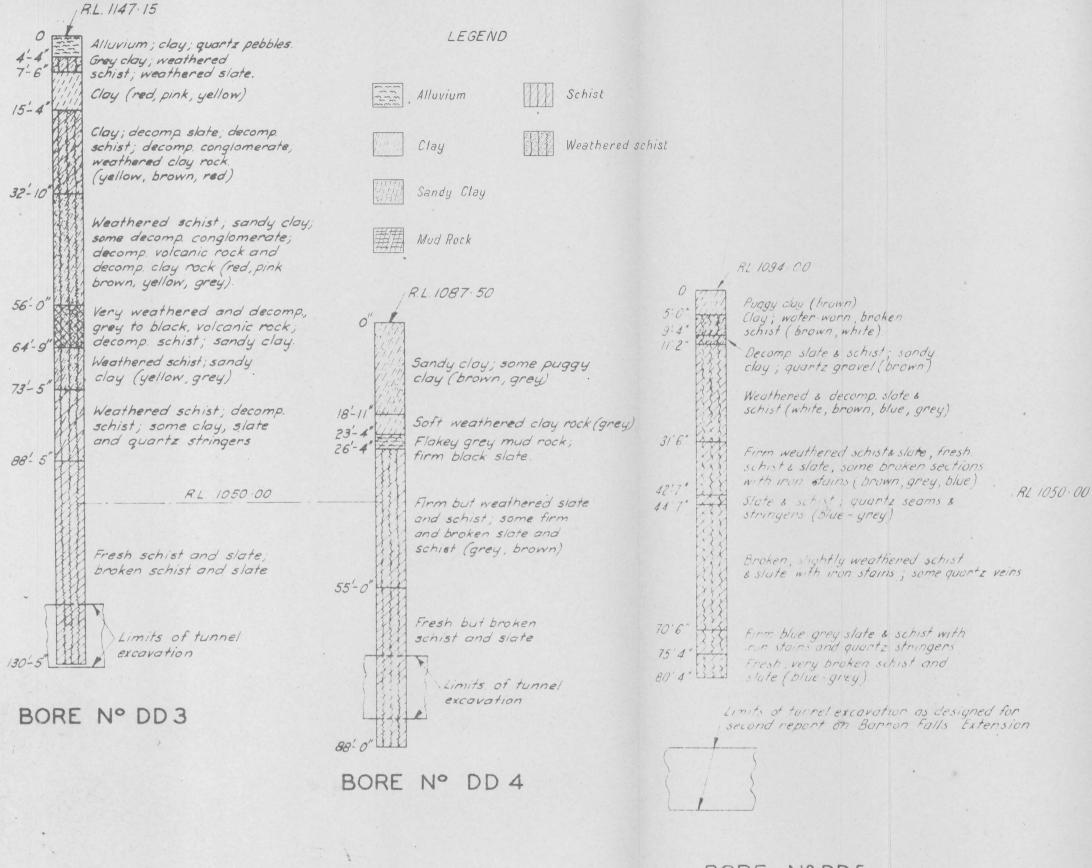
The values of Young's Modulus may be considered to have a maximum error of ±30%. The above relationship is approximately correct for most rock types, other than salts, sits and possibly limestones (Applies to curve A only)

FIGURE 7. EMPIRICAL RELATION BETWEEN YOUNG'S MODULUS
AND THE COMPRESSIONAL WAVE VELOCITY IN ROCKS







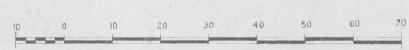


BORE N° DD5.

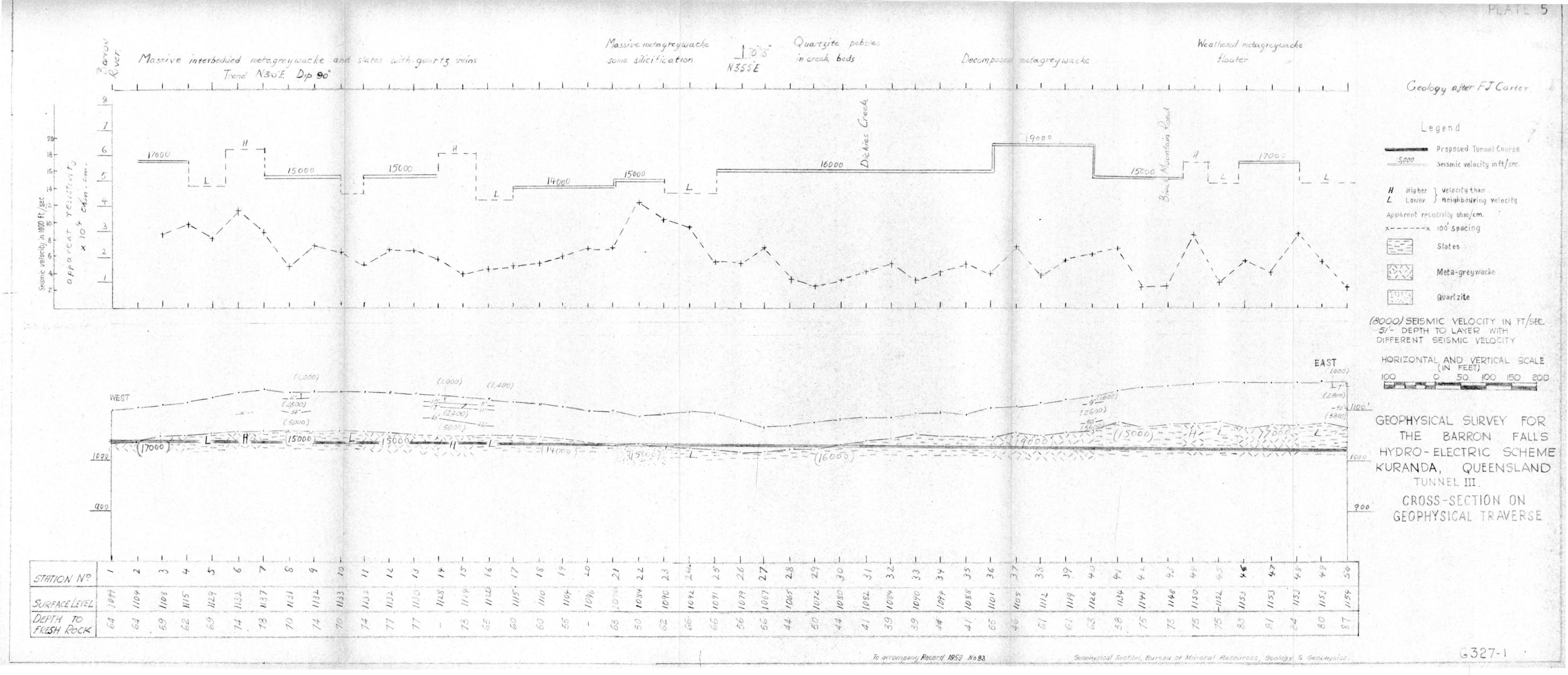
GEOPHYSICAL SURVEY FOR THE BARRON FALLS
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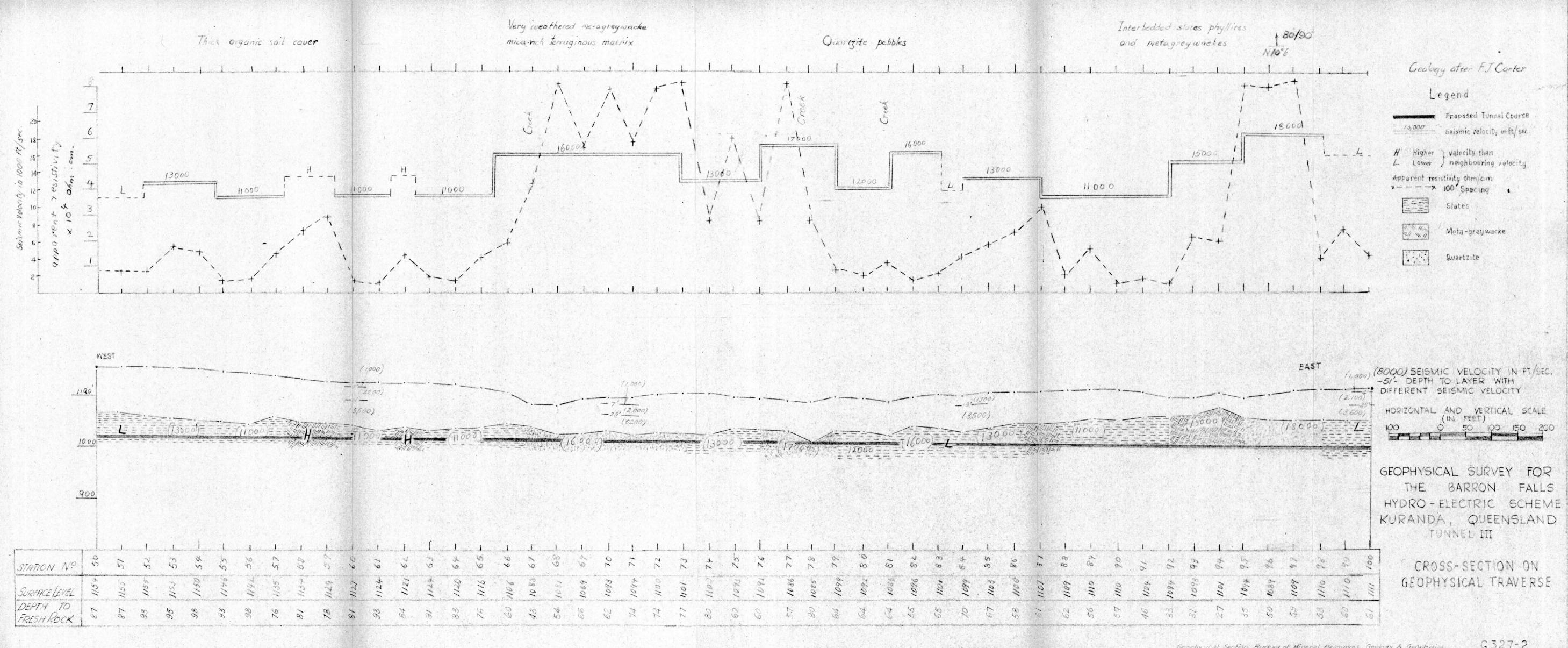
TUNNEL III
DRILL HOLE LOGS

VERTICAL SCALE IN FEET



Based on figs. 6 & 64 of the C.O.G. report on BARRON FALLS Extension dated 31.3.58





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