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DEPARTMENT OF NATIONAL DEVELOPMENT

BUREAU OF MINERAL RESOURCES, GEOLOGY AND GEOPHYSICS

RECORD No. 1962/67



PALOONA DAM SITE SEISMIC REFRACTION SURVEY, TASMANIA 1960

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SUMMARY

This Record describes a seismic refraction survey along part of the Forth River near Devonport, Tasmania. The aim of the survey, which was requested by the Hydro-Electric Commission of Tasmania, was to ascertain the depth to the bedrock and the type of bedrock on the site proposed for a dam. The plates show the results in the form of cross-sections along the traverses; a comparison between depth obtained by drilling and by the seismic survey is given.

1. <u>INTRODUCTION</u>

The Hydro-Electric Commission of Tasmania proposes to construct a dam and a power station on the lower reaches of the Forth River near Paloona, on the north coast of Tasmania. The purpose of the dam is to control the flow of the water in the river, in order to prevent the flooding of cultivated land by water released from proposed power stations on the Mersey and Forth Rivers. In addition, it is intended to use the fall of water across the dam for the production of electric power.

The Paloona dam site is located approximately 6 miles up-stream from the mouth of the Forth River. The approximate co-ordinates are 423917 (Devonport sheet of Australia 4-mile series).

The Commission requested the Bureau of Mineral Resources, Geology and Geophysics to investigate the site, the object of the survey being to determine the depth to the bedrock and the nature of the rocks in both bedrock and overburden.

In this report, the term 'bedrock' refers to the deepest refractor with the highest seismic wave velocity. The term 'overburden' refers to river gravel, clay, scree material, and completely or partly weathered rock.

The survey was carried out in May 1960 by a geophysical party consisting of E.J. Polak, party leader, and M.J.W. Duggin, geophysicist. The Commission provided field assistants and carried out topographical surveys.

2. GEOLOGY

The proposed site is situated in a sequence of Cambrian chert, greywacke, argillite, and siltstone on the northern limb of a south-easterly plunging fold. The beds dip eastwards at 40 degrees (Paterson, 1961).

The valley is wide with gently sloping hillsides, blanketed by terraced river gravel and scree material (see Plate 1). On the eastern abutment 40 ft of gravel (at B4 and C27) and 26 ft of scree material (at J5) were found by drilling.

Outcrops of the rocks are mostly confined to the river bed. Drilling indicates that chert underlies the scree material on the western abutment, while greywacke, argillate, and siltstone underlie the gravel and scree material on the eastern abutment.

3. METHODS AND EQUIPMENT

The seismic refraction method of exploration was used. A detailed description of this method has been given by Polak and Moss (1959).

The equipment used was a 12-channel SIE refraction seismograph with TIC geophones of natural frequency about 20 c/s.

4. RESULTS

The total length of surveyed traverses was 17,450 ft. Plate 1 shows the arrangement of the geophysical traverses, and Table 1 gives an interpretation of seismic wave velocities in terms of rock types at the Paloona dam site, as shown by drillers' logs.

TABLE 1

Seismic velocity (ft/sec)	Rock type		
1000	Soil		
1000 to 2500	River terrace material, talus, not water saturated.		
4000 to 5500	River terrace material, talus, water saturated.		
3200 to 8000	Very weathered to weathered rock.		
8000 to 17,000	Weathered to unweathered bedrock.		

The depth to the bedrock, the highest seismic velocity refractor, was calculated by the use of apparent velocity values obtained from weathering spreads. The depths thus calculated are plotted on Plates 2 to 6.

Cross-sections across the river (Traverses A to D, Plates 2 and 3) indicate that the thickness of the overburden increases gradually from the river in an easterly direction, reaching a maximum thickness of 132 ft near station B2. On the left (west) bank of the river the overburden is thinner, reaching a maximum of 75 ft near station B39.

As is shown in Table 1, there is an overlap in the seismic velocities in wet river-terrace material and in the weathered rock, and therefore they cannot be distinguished by seismic work. On Plates 2 to 6, velocities between 4000 ft/sec and 5500 ft/sec indicate either weathered rock or river-terrace material saturated with water.

Table 2 shows a comparison between the thickness of the overburden determined from drilling and from seismic data. The inset on Plate 6 shows a graph in which the depth to the bedrock proved in drilling is plotted against the depth calculated from seismic results.

Seismic velocities measured in the bedrock are indicated on Plate 1. The range of velocities is wide. At the westerly end of Traverse B a velocity of 9000 ft/sec was recorded, indicating a weathered and sheared zone. On the south-easterly part of the area the high velocity of 17,000 ft/sec was recorded near DDH 5854, indicating an unweathered and solid rock.

TABLE 2

Station No.	<u>Drillhole</u> <u>No</u> •	HEC Drilling Depth (ft)	Interpretation Rock Type	<u>Seismic</u> <u>Depth</u> (ft)	Survey Seismic Velocity (ft/sec)
B4	5851	0 - 10 10 - 38 38 - 125	Scree material River gravels Weathered	1 - 27 27 - 105	1500 4000
·	Total depth 165 ft	125 -	argillite Fresh grey- wacke	105 - 120	8000 12 , 000
B17	5852 Total depth	0 - 10 10 - 21 21 - 60	Scree material River gravels Weathered grey- wacke	0 - 4 4 - 25 25 - 53	1000 2500 4000
B32	96 ft 5853 Total depth 51 ft	60 0 - 15 15 -	Hard argillite Weathered chert Chert	53 -	17,000 1500 6500 12,000
C14	5854 Total depth 149 ft	0 - 23 23 - 80 80 -	Scree material Weathered grey- wacke Argillite	0 - 13 13 - 62 62 -	1500 3600 13,000
B39	5855 Total depth 80 ft	0 - 3 3 - 74 74 -	Scree material Weathered argillite Jointed chert	0 - 8 8 - 75 75 -	1500 5500 11,000
C27	5856 Total depth 111 ft	0 - 8 8 - 40 40 - 81 81 -	Scree material River gravels Weathered grey- wacke Greywacke, argillite	0 - 8 8 - 60 60 -	1500 3600 13,000
J5	5857 Total depth 91 ft	0 - 26 26 - 83 83 -	Scree material Weathered grey- wacke Greywacke	0 - 11 11 - 89 89 -	1500 4000 13,000

Measurements on cores from a depth of 72 ft in DDH 5852 (Fitzpatrick, 1961) gave Poisson's ratio as 0.24 and the specific gravity as 2.8. Table 3 shows a computation of Young's modulus using the measured values of Poisson's ratio and specific gravity for bedrock generally. The computed figures for Young's modulus are likely to be accurate within 20 per cent.

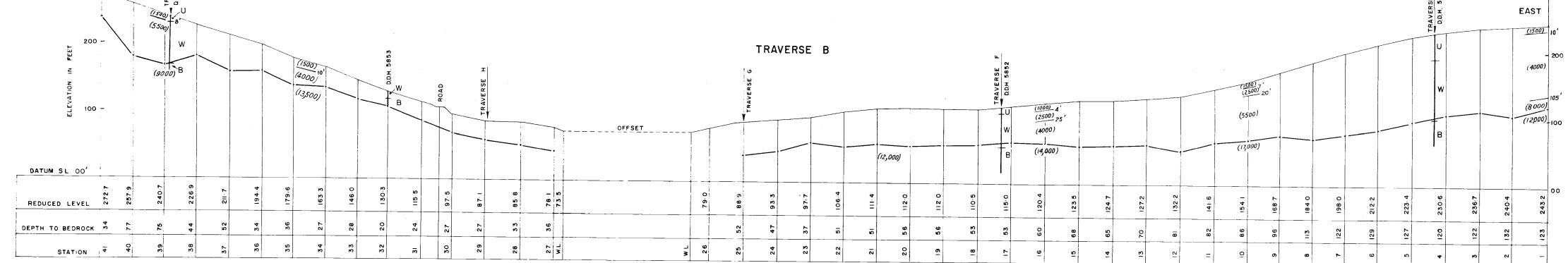
TABLE 3

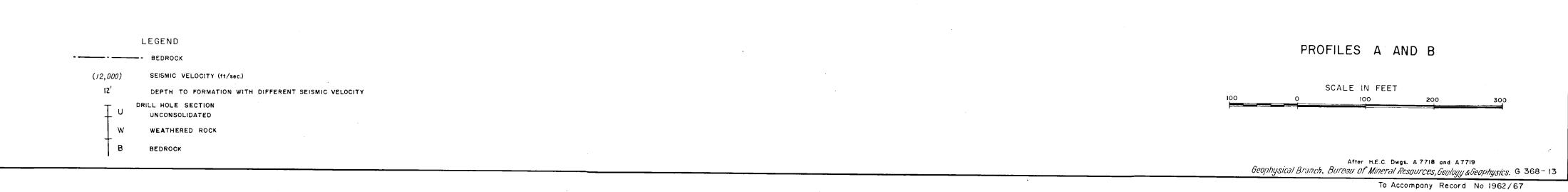
Seismic velocity (ft/sec)	Young's modulus 1b/in 2
9000	1.9 x 10 ⁶
12,000	3.7×10^6
15,000	7.3 x 10 ⁶
17,000	8.5 x 10 ⁶

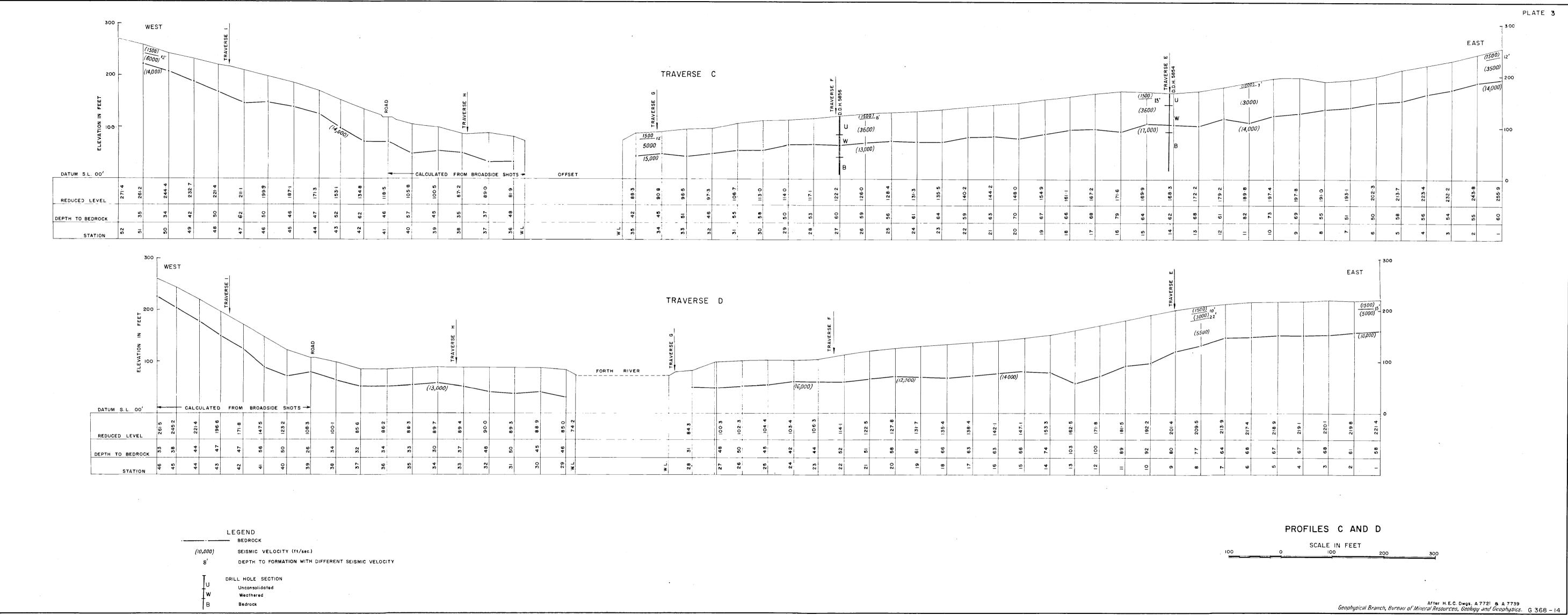
5. CONCLUSIONS

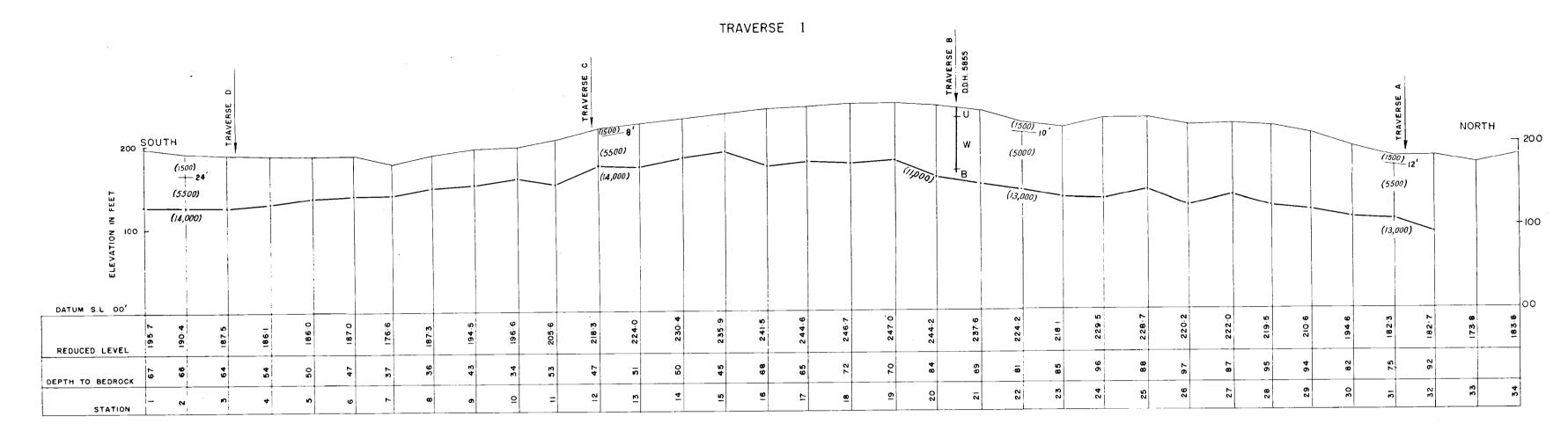
The overburden of soil and scree material is thickest along Traverse E. Close to E9 the thickness is about 138 ft. Accuracy of depth determinations is within 20 per cent of depth. Young's modulus for bedrock exceeds 1.9 x 10⁶ lb/in².

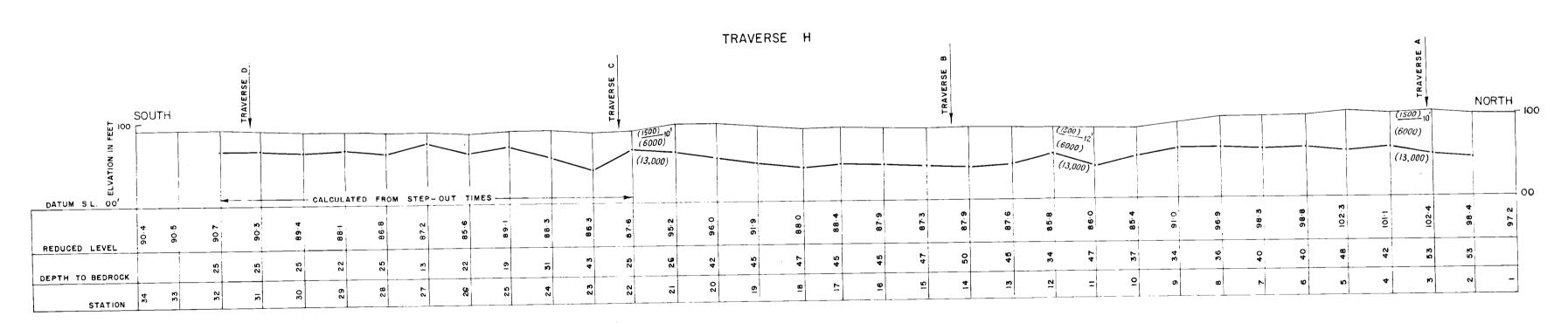
6	REFERENCE	<u> </u>
FITZPATRICK, M.	1961	Personal Communication
PATERSON, S.J.	1961	Personal Communication
POLAK, E.J., and MOSS, F	.J. 1959	Geophysical survey at the Cluny damsite, Derwent River, Tasmania. Bur. Min. Resour. Aust. Records 1959/87.

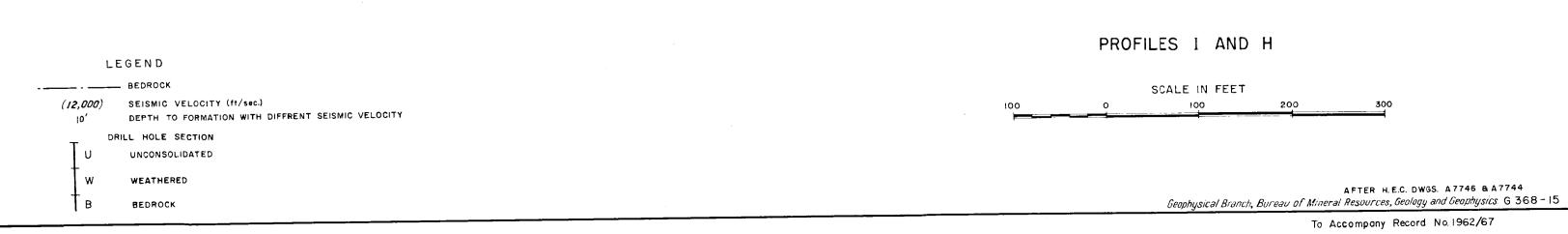


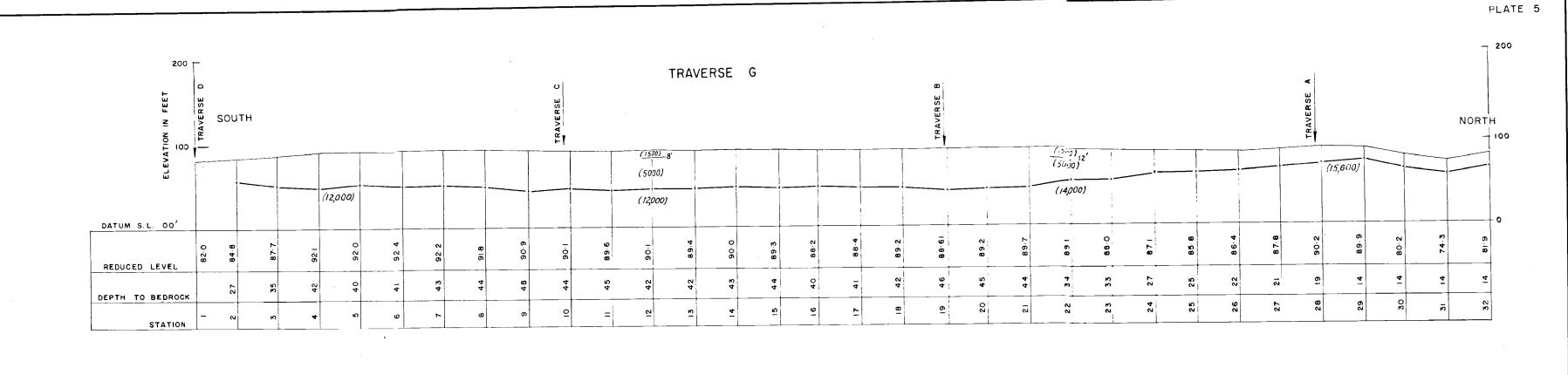


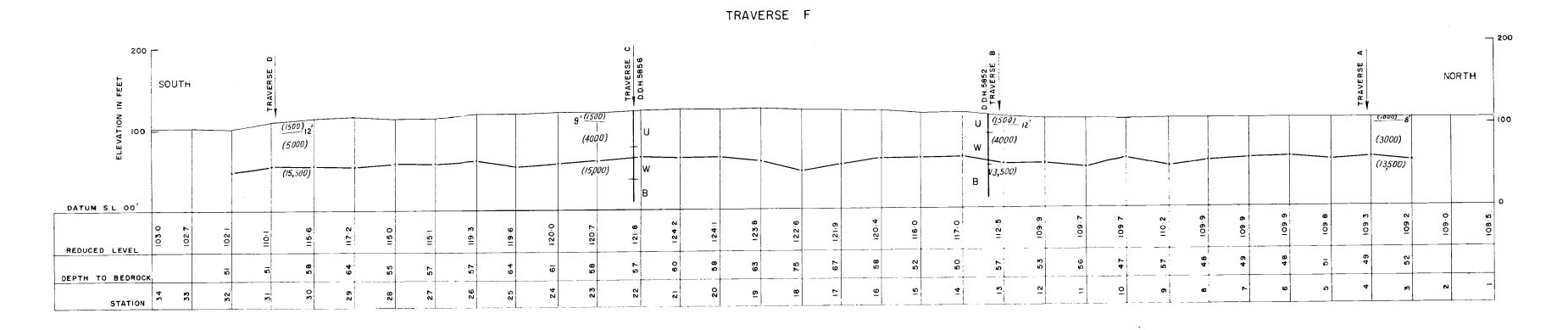












DAMSITE, TAS., 1960

