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DEPARTMENT OF NATIONAL DEVELOPMENT

BUREAU OF MINERAL RESOURCES, GEOLOGY AND GEOPHYSICS



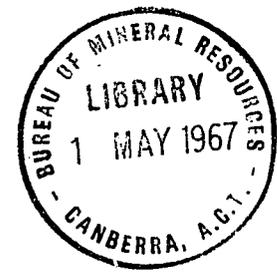
RECORD No. 1967/4

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LAKE BELLFIELD DAM SITE

VIBRATION TESTS,

VICTORIA 1963 AND 1965



by

P.E. MANN

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ILLUSTRATIONS

- Figure 1 Relations between charge E and distances d for acceleration $r=0.2g$ (Drawing No.J54/B5-7) Facing page 3.
- Plate 1 Locality map (Drawing No.J54/B5-5)
- Plate 2 Results of vibration tests made in 1963 and 1965 (J54/B5-6)

SUMMARY

Tests indicate that there is a correlation between the explosive charge and the resulting acceleration multiplied by distance to the power 1.5. By adopting a maximum permissible acceleration of 0.2 g, the relation between permissible explosion distance and explosive charge is derived. At distances of 300, 600, and 1000 feet the corresponding charges should be equal to or less than 50, 400, and 950 lbs.

1. INTRODUCTION

Vibration tests had been made in 1963 for the State Rivers and Water Supply Commission, Victoria (SRWSC), at the Lake Bellfield dam site to determine the maximum permissible explosive charge to be used to excavate a spillway without damaging the concrete lining of a diversion tunnel (Mann, 1964). However, excavation of the partly completed spillway revealed several major faults in the bedrock and the SRWSC considered it necessary to relocate and redesign the spillway. Also other circumstances have delayed lining the tunnel and constructing an outlet tower. Blasting in the new spillway closer than previously will increase the possibility of damage to the concrete works soon to be constructed.

The SRWSC requested the Bureau of Mineral Resources to make further tests to compare the effects of ripple firing with 17-millisecond detonating relays and 100-millisecond L-series short delay electric detonators for charges used to blast the rock. Vibration tests of two charges detonated on the 1st September 1965 were made by the author and C. Shade (an officer of the SRWSC). Subsequent tests in September, October, and November were made by C. Shade, and seismograms of all the tests were interpreted by the author.

2. METHOD AND EQUIPMENT

A Sprengnether portable Blast and Vibration Seismograph, serial No.1577 was used to record the vibrations produced by instantaneous and ripple-fired charges used to blast rock. Different types of blasts as used in the construction programme were recorded at different distances from the seismograph. For each test the seismograph was mounted on a small concrete block poured on fresh bedrock, except at station 6 on the uncompleted earth and rock-fill retaining wall (hachured area in Plate 1). Test 31/65 (Plate 2) was made to gain some information about the vibrations at a private building produced by a weight of charge commonly used for blasting. It was possible to record the vibrations only at a station 1500 ft from the blast i.e. approximately one third of the distance between the blast and the building. Test 33/65 was made to measure the vibrations transmitted to the bedrock by a vibrating flat drum roller that was crushing filter-zone material about 15 ft from an outcrop.

Two Sprengnether seismographs (serial Nos.1577 and 1863) were used during the 1963 tests. A brief description of the instruments and the technique for computing the results has been given by Mann (1964). It was assumed that the magnifications were respectively 100 and 160. However, a check calibration of the instruments showed that the magnification of instruments 1577 and 1863 is 50 and 100 respectively. The data given in Table 1 and formula 3 by Mann (1964) are incorrect. Useful results from the previous tests have been recomputed (Plate 2) and combined with the results obtained in 1965.

In some tests with 100-millisecond delays it is possible to distinguish on a seismogram waves produced by the detonation of individual charges in the firing sequence, e.g. Test 30/65. In this instance the amplitude and frequency of each vibration was treated as being equivalent to the vibration produced by the detonation of a single charge of the same weight; i.e. a delay of 100-milliseconds between charges allows the waves from a blast to decay to a negligible level before waves from the next blast arrive. However, a 100-millisecond delay is too small for this assumption to be strictly valid. The frequency and amplitude of the later waves are greater or less than these parameters for waves from the first blast.

It was attempted to establish a relation between the resultant acceleration r , the charge E , and the distance d of the type:

$$r = \frac{1}{d^m} f(E) \dots\dots\dots(1)$$

By using the data of Plate 2, a plot of E versus d was made, at the same time noting the values of r . It appeared that the values of r could be contoured. Then for constant values of E , $\log r$ was plotted against $\log d^m$ and the best fitting value for m in equation 1 was found to be 1.5. The next step was to make a statistical correlation between E and $rd^{1.5}$, or between $\log E$ and $\log (rd^{1.5})$. Because a plot of E versus $rd^{1.5}$ shows a regular pattern, and also because of the easier computation, the statistical analysis was made on values of E and $rd^{1.5}$, and is given in the Appendix.

The correlation coefficient was found to be 0.58, and the standard error in the correlation coefficient was 0.12. With 32 pairs of values for E and $rd^{1.5}$, and for a correlation coefficient of 0.58, the probability that E and $rd^{1.5}$ are unrelated is less than 1 in 1000 (Young, 1962).

The two regression lines are :

$$E = 60.4 \frac{rd^{1.5}}{10^3} + 220 \pm 260 \dots\dots\dots(2)$$

and

$$\frac{rd^{1.5}}{10^3} = \frac{0.56}{100} E + 0.76 \pm 0.63 \dots\dots\dots(3)$$

in which the terms after \pm indicate the standard error, E is the charge in lbs, d the distance in feet, and r the acceleration in terms of g .

Equation 3 is preferred to equation 2 because it is desired to estimate the permissible explosion distance d in terms of explosive charge E for an adopted acceleration r .

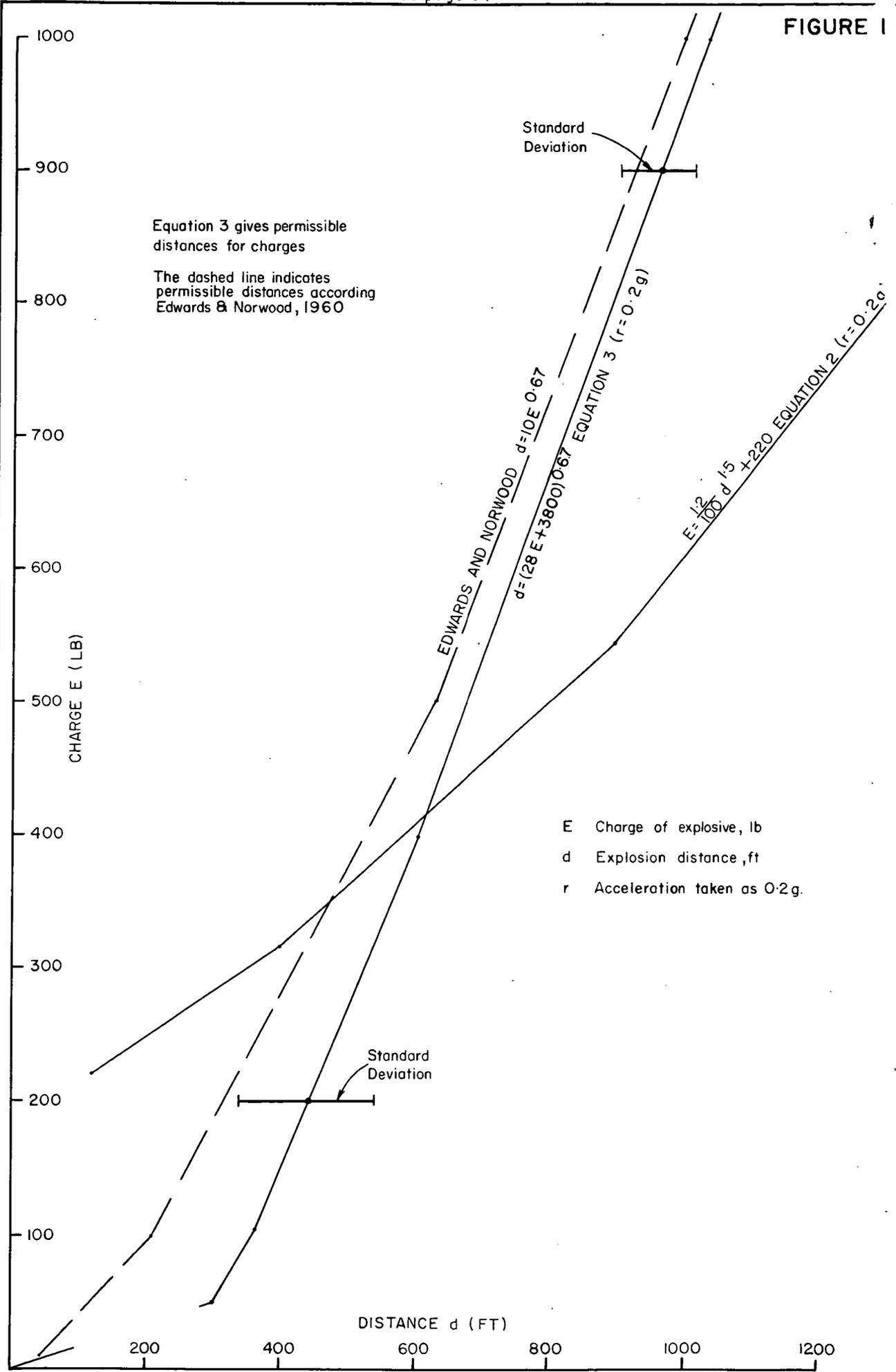
Figure 1 shows a plot of equations 3 and 2 for $r = 0.2g$, and a graph giving the limits according Edwards and Northwood (1960).

It is believed that permissible charges according to equation (3) for $r = 0.1g$ (boundary safe/caution zone) are too small (Thoenen and Windes, 1942), i.e. estimates are too conservative. Hence, the value $r = 0.2g$ was adopted. The resulting estimates for permissible charges then agree closely with the curve given by Edwards and Northwood (1960) as shown in Figure 1.

The tests shown on Plate 2 show features specially worth mentioning :

- 1) Tests 1/63, 3/63, 4/63, and 7/63 at distances of 335 to 450 ft, with charges between 10 and 447 lb, give accelerations of 0.15g to 0.19g.
- 2) Generally, tests for distances less than 300 ft show abnormally high scatter for the acceleration. It is considered that more reliable data are obtained from tests with a shot-distance greater than 300ft.

FIGURE 1



Equation 3 gives permissible distances for charges
 The dashed line indicates permissible distances according to Edwards & Norwood, 1960

E Charge of explosive, lb
 d Explosion distance, ft
 r Acceleration taken as 0.2g.

RELATIONS BETWEEN E AND $(rd)^{1.5}$ FOR $r=0.2g$

3. CONCLUSIONS

An empirical relation between charge E, resultant acceleration r, and distance d in the form

$$rd^{1.5} = 5.6E + 760$$

derived from the data can be used to determine the permissible distance for blasting if an acceptable ground acceleration is known. A ground acceleration of 0.2g or less is considered to be safe. The following table shows the suggested permissible charges for charges without delay times. If delay blasting is applied the same table can be used but delay times should be 100 milliseconds or greater.

Distance (ft)	Permissible explosive charge (lb)
300	50
400	150
600	400
800	670
1000	950
1200	1200

The relation is not considered reliable for distances of less than 300 ft, when the permissible charge can only be estimated from experience. The chance that the variables are unrelated is less than 1 in 1000, and their correlation coefficient is 0.6.

4. REFERENCES

- EDWARDS, A.T. and
NORTHWOOD, T.D. 1960 Experimental studies of the effects of blasting on structures. The Engineer 210, 538 - 546.
- MANN, P.E. 1964 Lake Bellfield dam site vibration tests, near Halls Gap, Victoria 1963. Bur. Min.Resour.Aust.Rec. 1964/71.
- THOENEN, J.R. and
WINDES, S.L. 1942 Seismic effects of quarry blasting. Bull.U.S.Bur. Min. 442.
- YOUNG, H.D. 1962 STATISTICAL TREATMENT OF EXPERIMENTAL DATA. New York, McGraw Hill Book Comp. Inc.

APPENDIXSTATISTICAL ANALYSIS OF RESULTS

The 32 results used in the statistical analysis are listed on the opposite page.

For convenience the following substitutions are made:

$$x = \frac{rd^{1.5}}{10^3} \quad \text{and} \quad y = E$$

The average values of x and y are $X_0 = 3.35$ and $Y_0 = 423$

$$\Delta x = x - X_0$$

$$\sum (\Delta x)^2 = 313$$

$$\Delta y = y - Y_0$$

$$\sum (\Delta y)^2 = 3,403,515$$

Standard deviation

$$S_x = \left(\frac{\sum (\Delta x)^2}{n} \right)^{\frac{1}{2}} = 3.13; \quad S_y = \left(\frac{\sum (\Delta y)^2}{n} \right)^{\frac{1}{2}} = 326$$

$$\sum (\Delta x \Delta y) = 18,506$$

$$\text{The correlation coefficient } C = \frac{\sum \Delta x \Delta y}{n S_x S_y} = 0.58$$

$$\text{The standard error in } C \text{ is } M_c = \frac{1-C^2}{\sqrt{n}} = 0.12$$

The two regression lines are therefore:

$$y - Y_0 = C \frac{S_y}{S_x} (x - X_0)$$

and

$$x - X_0 = C \frac{S_x}{S_y} (y - Y_0)$$

which become

$$E = 0.06rd^{1.5} + 220 \pm 260$$

and

$$rd^{1.5} = 5.6E + 760 \pm 630$$

When $r = 0.2$, the equations become

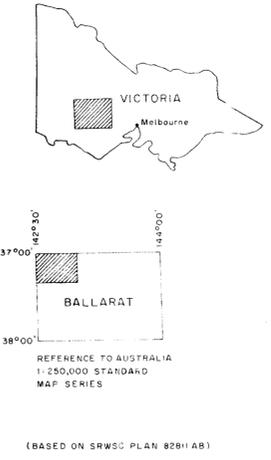
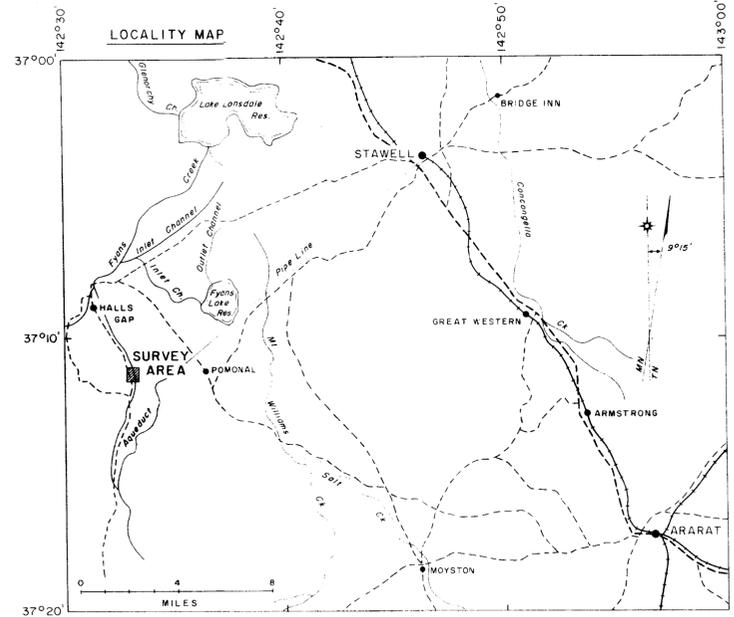
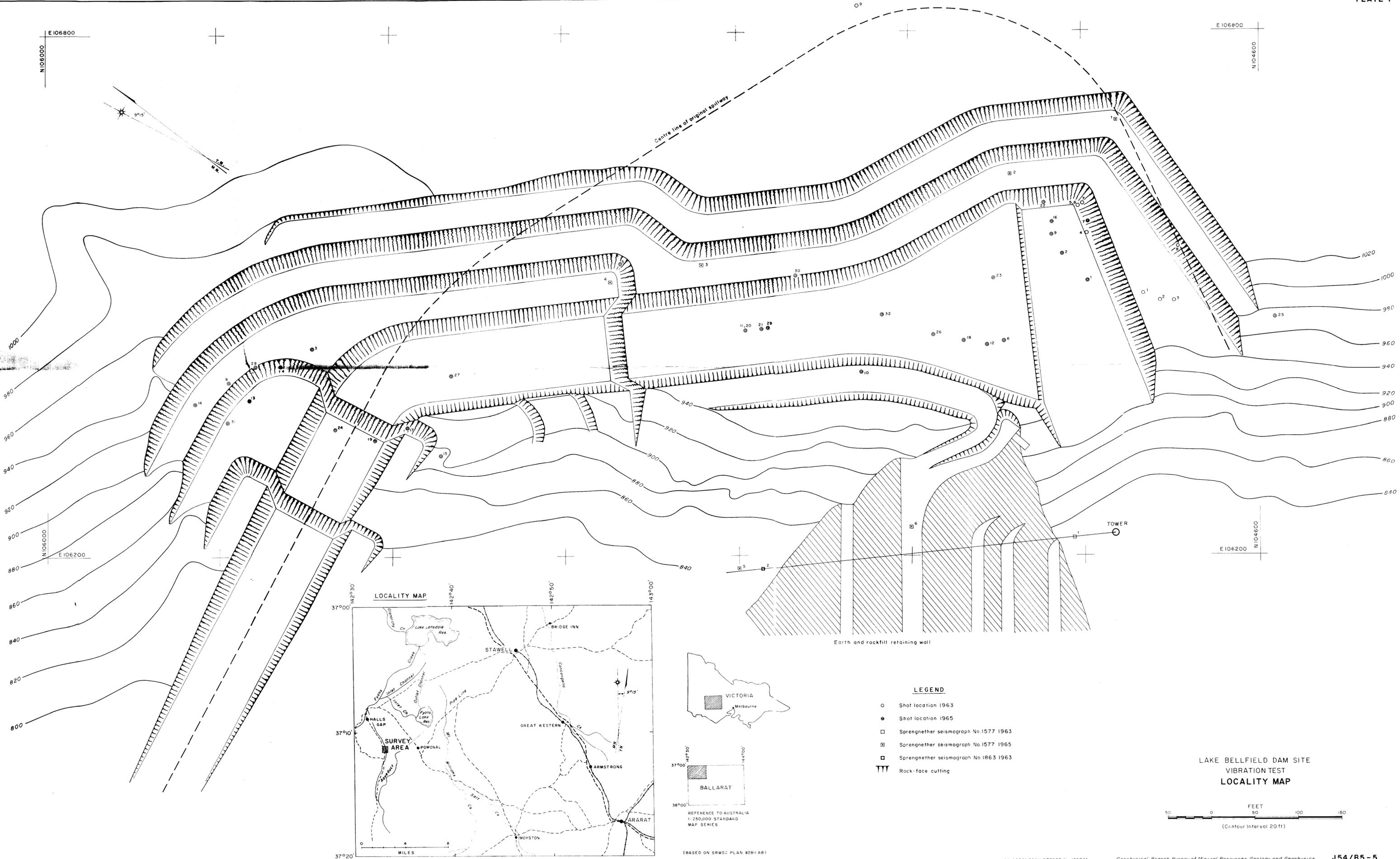
$$E = \frac{1.2}{100} d^{1.5} + 220 \text{ ----- (2)}$$

and

$$d = (28E + 3800)^{0.67} \text{ ----- (3)}$$

Equation 3 is used to compute the permissible charges.

Test No. and year	d(ft)	r(g)	x	y	ΔY	ΔX
			$\frac{rd^{1.5}}{10^3}$	E(lb)		
1/63	335	0.15	0.92	10	-413	-243
3/63	345	0.18	1.13	40	-383	-222
4/63	430	0.18	1.58	100	-323	-177
7/63	450	0.19	1.81	447	+ 24	-154
7/63	610	0.09	1.36	Same	+ 24	-199
8/63	420	0.25	2.15	444	+ 21	-120
8/63	650	0.13	2.16	Same	+ 21	-119
9/63	715	0.20	3.84	500±	+ 77	+ 49
1/65	207	0.62	1.82	27	-396	-153
3/65	460	0.05	0.49	48	-375	-286
4/65	500	0.44	4.95	800±	+377	+160
5/65	95	0.72	0.69	16	-407	-266
6/65	250	0.40	1.55	470±	+ 47	-180
7/65	410	0.09	0.75	45	-378	-260
8/65	810	0.47	1.08	940±	+517	-227
13/65	600	0.08	1.19	150	-273	-216
14/65	665	0.22	3.77	730±	+307	+ 22
17/65	395	>0.78	>6.12	175?	-248	+277
18/65	315	0.66	3.66	660±	+237	+ 31
19/65	435	1.3	11.77	870±	+447	+842
20/65	360	0.11	7.45	84	-339	+410
21/65	340	0.47	2.93	170±	-253	-42
23/65	335	1.6	9.85	940±	+517	+650
24/65	675	0.38	6.67	760±	+337	+332
25/65	290	0.42	2.07	665±	+242	-128
26/65	230	0.54	1.90	835±	+412	-145
27/65	560	0.14	1.85	270	-153	-150
28/65	780	0.09	1.97	650	+227	-138
29/65	330	0.66	3.99	180±	-243	+ 64
30/65	275	0.30	1.37	300±	-123	-208
31/65	1590	0.2	12.70	1120±	+697	+935
32/65	215	0.5	1.58	205±	-218	-177



- LEGEND**
- Shot location 1963
 - Shot location 1965
 - Sprengnether seismograph No. 1577 1963
 - ▣ Sprengnether seismograph No. 1577 1965
 - ▤ Sprengnether seismograph No. 1863 1963
 - ▨ Rock-face cutting

