COMMONWEALTH OF AUSTRALIA

### DEPARTMENT OF NATIONAL DEVELOPMENT

## BUREAU OF MINERAL RESOURCES, GEOLOGY AND GEOPHYSICS

Record No. 1968 / 86

## Lake Buffalo Dam Site Geophysical Survey, Victoria 1966



by

P.E. Mann and M. Wainwright

The information contained in this report has been obtained by the Department of National Development as part of the policy of the Commonwealth Government to assist in the exploration and development of mineral resources. It may not be published in any form or used in a company prospectus or statement without the permission in writing of the Director, Bureau of Mineral Resources, Geology & Geophysics.



Record No. 1968 / 86

# Lake Buffalo Dam Site Geophysical Survey, Victoria 1966

bу

P.E. Mann and M. Wainwright

The information contained in this report has been obtained by the Department of National Development as part of the policy of the Commonwealth Government to assist in the exploration and development of mineral resources. It may not be published in any form or use in a company prospectus or statement without the permission in writing of the Director, Bureau of Mineral Resources, Geology and Geophysics.

## CONTENTS

	SUMMARY	Page					
1.	INTRODUCTION	1					
2.	GEOLOGY	1					
3.	METHODS AND EQUIPMENT	2					
4.	RESULTS	2					
5.	CONCLUSION	5					
6.	REFERENCES	6					
APPENDIX	Seismic velocities of core samples	7					
ILLUSTRATIONS							
Plate 1.	Locality map and geophysical traverses	(J55/B5 <b>-</b> 34)					
Plate 2.	Seismic cross-sections for Traverses A and C	(J55/B5-35)					
Plate 3.	Seismic cross-sections for Traverses D and E	(J55/B5 <b>-</b> 36)					
Plate 4.	Seismic cross-sections for Traverses F and G	(J55/B5 <b>-</b> 37)					
Plate 5.	Seismic cross-sections for Traverses H,J,K,L, and M	(J55/B5-38)					
Plate 6.	Seismic cross-sections for Traverses N,0,P, and Q	(J55/B5 <b>-</b> 39)					
Plate 7.	Location of geophysical traverses and contours of bedrock (14,000 ft/s) based on seismic results	(J55/B5-40)					
Plate 8.	Contour plan of rock with a seismic velocity of more than 6500 ft/s	(J55/B5-42)					

#### SUMMARY

A seismic refraction survey was made at a dam site on the Buffalo River, 12 miles upstream from Myrtleford, at the request of the State Rivers and Water Supply Commission of Victoria.

The unweathered bedrock is too deep to be used as a foundation rock but a rock with a seismic velocity of about 7000 ft/s, corresponding to a compressive strength of about 13,000 lb/in<sup>2</sup> and a Young's modulus of about 10<sup>6</sup> lb/in<sup>2</sup>, is probably sufficiently high for an earth dam.

A contour plan of the 7000-ft/s rock surface indicates the presence of an older and a younger river valley, separated by an island or river terrace.

The survey indicates that the best position for a new dam is upstream from the 800-ft contour, where the present dam is located.

#### 1. INTRODUCTION

The State Rivers and Water Supply Commission of Victoria proposes to enlarge the existing dam on the Buffalo River, about twelve miles upstream from Myrtleford in Victoria. The dam is to provide more water for the irrigation of the rapidly increasing acreage of crops in the Ovens and Murray valleys.

The centre line of the proposed dam is about 500 feet downstream of the existing dam; the approximate co-ordinates of the site are 425623 on the Wangaratta sheet of the Australia 1:250,000 map series.

In response to an application from the Commission, the Bureau of Mineral Resources, Geology and Geophysics made a geophysical investigation of the site to determine the depth and nature of the bedrock. Approximately 40,000 feet of traverse was surveyed with the seismic refraction method and 5000 ft with the resistivity method. The survey was carried out between 2nd March and 16th April 1966 by a party consisting of P.E. Mann, party leader and geophysicist, and D. Tarlinton, field assistant. L. Kevi and M. Wainwright, BMR geophysicists, were attached to the party for part of the survey. The Commission provided four field assistants and ancillary supplies and also surveyed the traverse lines.

The term 'unweathered bedrock' used in this record refers to the deepest refractor with the highest measured seismic velocity, which ranged between 13,000 and 17,000 ft/s. The Commission plans to construct an earth dam, for which a rock of considerably lower seismic velocity is acceptable as foundation.

#### 2. GEOLOGY

The general geology of the dam site is given by Cormack (1963). Additional geological information is given by Currey (1964). Geological logs of drill holes, trenches, benches, auger holes, and shafts are available from investigations made for the existing dam, and other drilling carried out after the seismic survey (Jacobson, 1966).

At the dam site the left bank is very steep; the right bank consists of gradual slopes, possibly old river terraces. The bedrock consists of partly weathered interbedded quartzites, hornfels, and slates, metamorphosed sandstones, siltstones, and mudstones of Ordovician age. The river bed is in hard, fresh metamorphics with a strike of about 330° magnetic and a dip of 60° to 80° to the west. Three principal joint systems exist of which two are of major importance: one parallel to the bedding strike but with a small upstream dip; the second one with a strike of about 40° and approximately a vertical dip has a marked effect on the course of the Buffalo River. Several small shear zones have been mapped.

Drilling results suggest a buried river channel on the right bank. In recent time this channel was filled and covered with gravel, sand, and clay. Later the river degraded its old bed by vertical and lateral erosion, successively cutting lower terraces. Today the river flows against the western side of the valley with its present bed about 30 feet below the older bed.

#### 3. METHODS AND EQUIPMENT

The seismic refraction method has been described in a previous report to the Commission (Sedmik, 1961).

A 24-channel refraction seismograph manufactured by South-Western Industrial Electronics Co. was used with Technical Instruments Co. geophones with a natural frequency of 20 c/s. To determine the transverse velocity, three-component Hall-Sears geophones, model HS-1-LP 3D with a natural frequency of 14 c/s, and a miniature three-component Hall-Sears geophone, model HS-1-LPJ 3D with a natural frequency of  $7\frac{1}{2}$  c/s, were used. The following types of spread were used:

- (1) Weathering spreads: to obtain the thickness and seismic velocity of the near-surface layers. Geophones were spaced 10 feet apart and shots were fired at 10, 50, and 200 ft in line at each end of the spread.
- (2) Normal spreads: Geophones were spaced 50 ft apart and shots were fired at 50 and 200 ft in line from each end of the spread. A shot was also fired in the centre of the spread wherever practicable. On some traverses additional shots at 600 ft were detonated to record the highest velocity refractor.
- (3) To determine the elastic constants in situ, a spread 150 ft long consisting of vertical and three-component geophones, spaced 25 feet apart, was laid out on the excavated surface of the secondary spillway of the existing dam. Shot distances of 25, 100, and 350 ft were used.

Some laboratory measurements were made on drill hole cores. The bulk velocity was determined on cores in the wet and dry state. The bar velocity of the cylindrical specimens could not be measured because they were too short. The equipment and the measuring technique has been described by Kevi (1966).

Resistivity traversing with an electrode spacing of 100 feet, used on Traverses C and D, did not give any significant results, and the resistivity profiles are not presented in this Record.

#### 4. RESULTS

Plate 1 shows the locality and the layout of the seismic traverses.

#### Seismic velocities

Seismic velocities are a good measure for rock quality or rock strength (Wiebenga and Manganwidjoyo, 1960).

The following table gives an interpretation in geological terms, of the seismic refractors recorded during the survey.

Velocity (ft/s)	Rock type
1000	Soil
2000 to 3000	Clayey material. This could be alluvial clay or clay as a final stage of weathering on the hill side.
3100 to 5600	Alluvium: the lower the velocity the higher the clay content. Alluvial sands and gravels saturated with water have a velocity of 5000 ± 600 ft/s.
3800 to 5900	Very weathered, decomposed bedrock or alluvial hillside material.
6000 to 11000	Jointed weathered bedrock to slightly weathered bedrock. Weathering starts on the joints.
13000 to 17000	Unweathered, jointed to unjointed rock.

It may be observed that the geological interpretation of seismic velocities can be quite ambiguous, e.g. a 5000 ft/s velocity could mean:

- (a) alluvial sand or gravel deposit, water saturated.
- (b) weathered, decomposed bedrock underneath sediments.
- (c) weathered, washed out, alluvial hillside material.

The above ambiguities also make the checking of the seismic results against drill hole information of little use. For instance, the place in a drill hole where sedimentary sands or gravels change into weathered bedrock may show up from hand samples, but it may not be possible to discriminate between the rock types seismically. Two examples may illustrate the point:

At drill hole PD 100, on Traverse D between intersections with M and N, the drilling shows gravel to 64 ft and decomposed bedrock to 81 ft (bottom). The seismic work shows 1000 ft/s to 6 ft, 3500 ft/s to 30 ft, 5500 ft/s to 103 ft, 10,000 ft/s to 240 ft, and 14,000 ft/s below this.

At drill hole PD 110, on Traverse 0 between intersections with D and E, the drilling shows hill wash, sandy clay loam to 4 ft, soft decomposed clayey rock to 29 ft, and soft to hard weathered rock to 36 ft. The seismic work shows 1000 ft/s to 7 ft, 3500 ft/s to 30 ft, 5400 ft/s to 90 ft, 8500 ft/s to 171 ft, and 15,000 ft/s below this.

Plates 2 to 6 give the seismic cross-sections with some of the relevant drill hole information. The sections are self-explanatory.

#### Contour plans

Plate 7 shows a contour plan of the top of the weathered bedrock. The deepest area, below the 700-ft contour line, apparently underlies the old river course. The plan indicates the maximum limit of excavation required for a dam foundation.

For an earth dam, a bedrock with a seismic velocity of about 7000 ft/s may be sufficient. Using an empirical formula (Wiebenga and Manganwidjoyo, 1960), the strength of a 7000-ft/s rock in terms of a standard compression test is about 13,000 - 35% lb/in². Plate 8 shows the contours of formations with a seismic velocity of greater than 6500 ft/s, say about 7000 ft/s. The control points for this contour plan were located at either end of the seismic spreads, about 500 ft apart along the traverses. On the cross-sections (Plates 2 to 6) this contoured surface is indicated by a dashed line.

Following the 800-ft contour in Plate 8, it can be observed that the elongated depression near the intersection of Traverses K and C co-incides with the present course of the Buffalo River, but the depressions near the intersections of Traverses A and N and Traverses D and N co-incide with older river courses. The subsurface island near the intersection of Traverses A and L possibly represents a river terrace.

#### Young's Modulus

Young's modulus may be estimated from an empirical formula (Wiebenga, 1958):

$$E = 68.9V^{2.34}$$

where E is Young Modulus in dynes/cm<sup>2</sup> and V the longitudinal seismic velocity in kilo-feet/s. For V = 7000 ft/s, E =  $6.3 \times 10^{10}$  dynes/cm<sup>2</sup> or E =  $0.9 \times 10^{6}$  lb/in<sup>2</sup>.

Young's Modulus and Poisson's ratio were also determined from longitudinal and vertically polarised transverse wave velocities on a 150-ft long spread near station A 65, close to the spillway:

Longitudinal velocity 7200 ft/s
Ratio longitudinal to traverse velocity 1.92
Poison's ratio 0.31
Mean density from cores 2.25 g/cm<sup>3</sup>
Young's modulus 1.1 x 10<sup>6</sup> lb/in<sup>2</sup>

Summarising, Young's modulus determined with dynamic methods is of the order of  $10^6\ lb/in^2$ .

#### Accuracy of seismic depth measurements

An estimate of the accuracy of seismic depth measurements is not easily made because very little relevant drill hole information can be used. In the present survey the difference between two depth estimates for the deepest refractor at the intersection of two traverses gave some information. Thirty seven pairs of values were obtained. The greatest difference from the mean was found to be 20%. The standard deviation for the percentage difference from the mean was 11%. These figures agree with the usual accuracy of a seismic engineering survey of this type.

#### 5. CONCLUSION

The results of the survey show that the depth of unweathered bedrock is up to 380 ft below ground level (near Station 00, Traverse 0), suggesting that the use of unweathered bedrock is impractical. However, a rock with a seismic velocity of about 7000 ft/s is probably sufficiently strong as a foundation rock for an earth dam. Plate 8 shows that the best location for such a dam is at the place where the present dam is located, upstream from the 800-ft contour line.

#### 6. REFERENCES 1963 Buffalo Dam regional geology, CORMACK, G. dam geology, geomorphology. Progress report No. 1. State Rivers and Water Supply Commission. Buffalo River, geology of dam 1964 CURREY, D.T. site. Memorandum Senior Designing Engineer. State Rivers and Water Supply Commission. 1966 Personal Communication. JACOBSON, G. Nillahcootie dam site geophysical 1966 KEVI, L. survey, Victoria 1965. Bur Min. Resour. Aust. Rec. 1966/39. 1961 Buffalo River No. 1 dam site SEDMIK, E.C.E. geophysical survey, Victoria 1961. Bur. Min. Resour. Aust. Rec. 1961/136. Exploration Geophysics applied WIEBENGA, W.A. 1958 to the Dolerites of Tasmania. Dolerite Symposium, Uni. of Tasmania.

1960

WIEBENGA, W.A. and

MANGANWIDJOYO,

Some correlations between rock

parameters, derived from

<u>Trans</u>. 217.

Wuerker's "Annotated tables of strength and elastic properties, 1956". A.I.M.E.

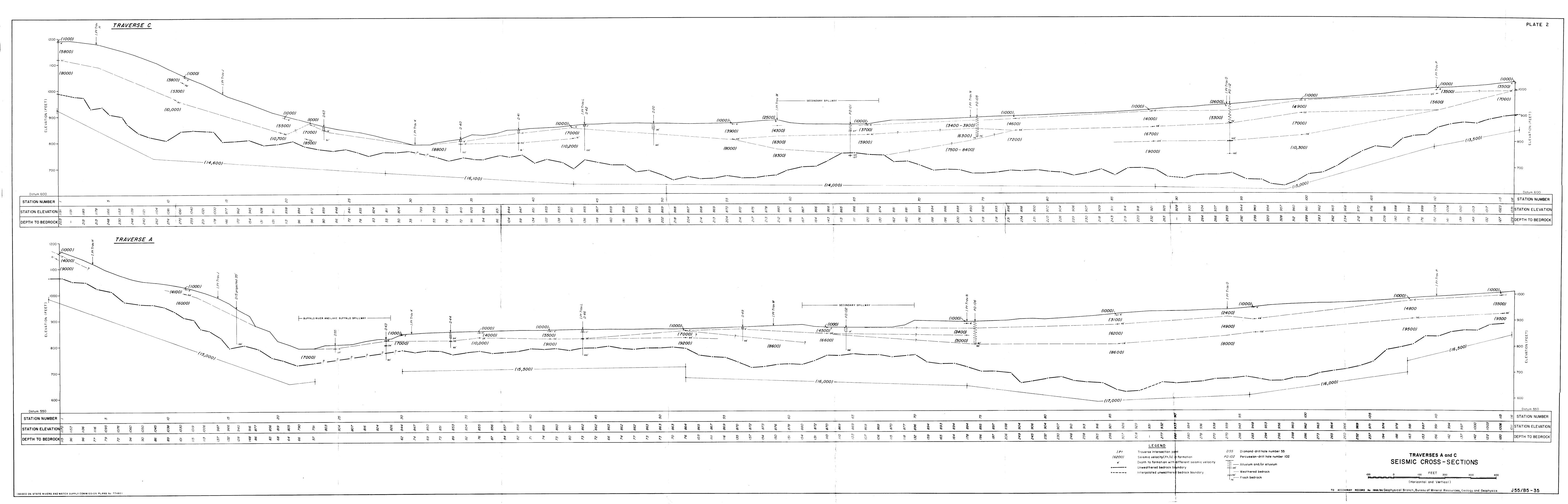
#### APPENDIX

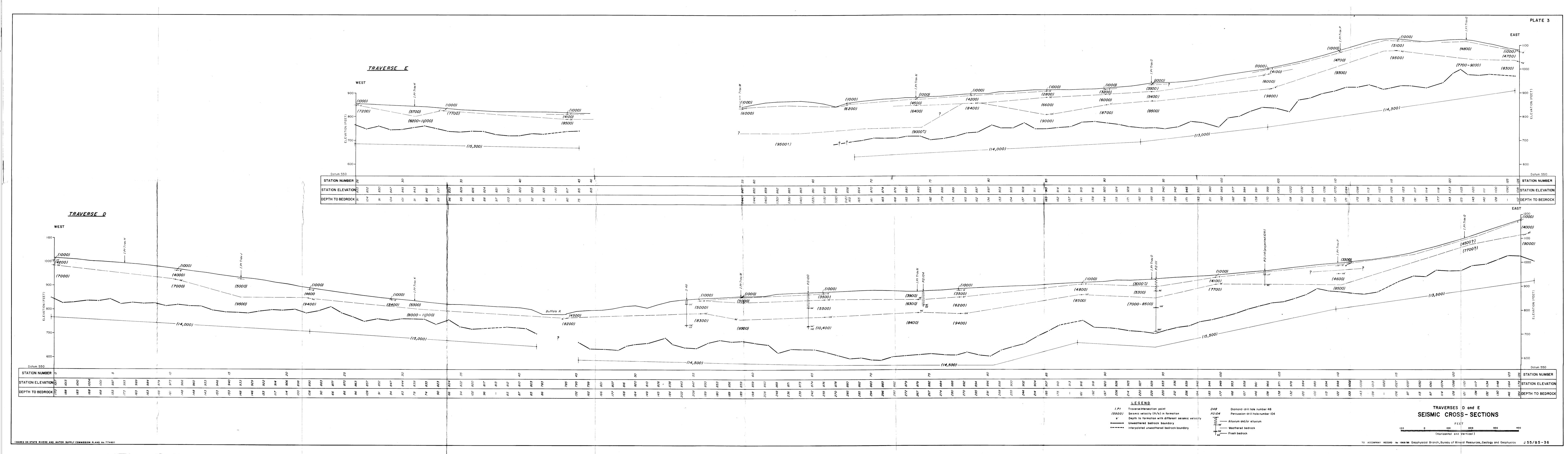
### Seismic velocities of core samples

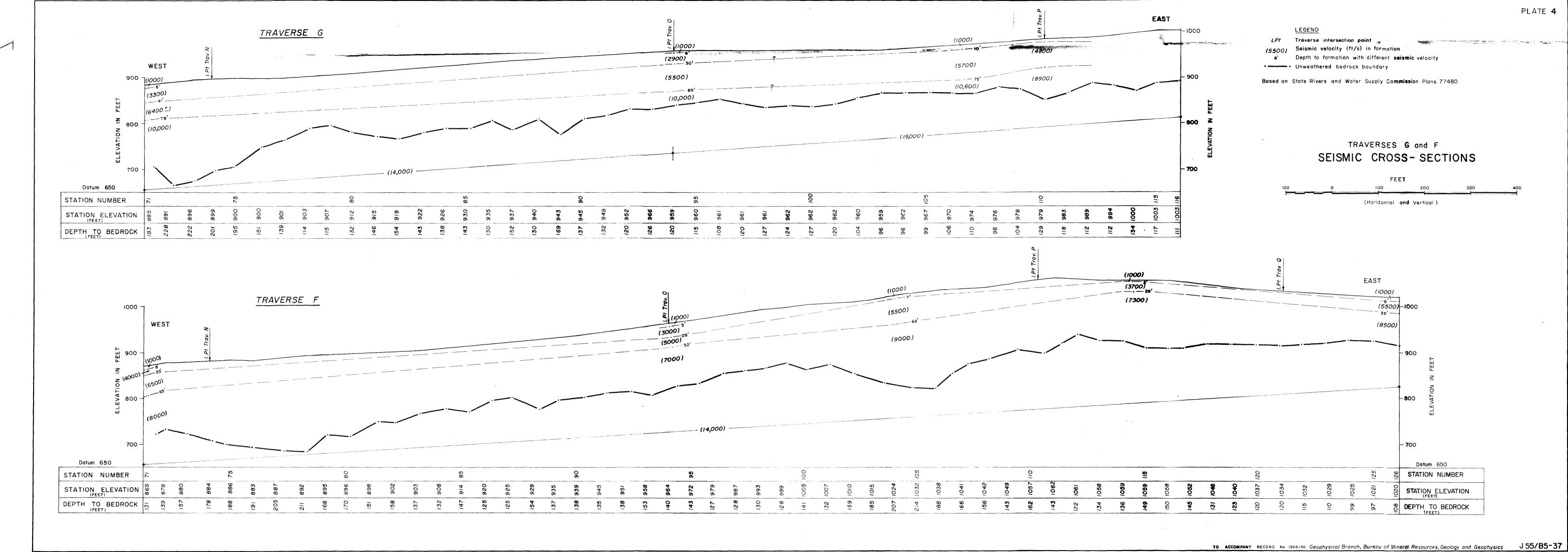
Waxed and unwaxed core samples from eight drill holes were received for velocity tests in the laboratory. Waxed cores were from PD 100, PD 101, and PD 103. Bulk velocities were measured but it was not possible to measure the bar velocity because the cores were too short or had been broken up, probably during transport. Because the poor condition of the cores and the inconclusive nature of the results, they are included only for completeness. The results are given in the following table:

Drill hole	Depth (feet)	Description	Density (gm/cm <sup>3</sup> )		Bulk velocity (ft/s x 103)	
			Dry	Wet	Dry	Wet
D31	20.0	Hard weathered siltstone	2.41	2.48	13.9	12.9
D31	23.7	Hard weathered sandstone	2.48	2.58	12.4	12.9
D42	52.0	Hard weathered sandstone	2.14	2.32	6.9	5.8
D42	99.0	Hard weathered sandstone	2.26	2.38	9.8	8.7
D20	67.0	Hard weathered sandstone	2.18	2.37	11.5	9.2
D20	79.0	Hard weathered sandstone	2.28	2.39	9.4	8.3
D57	68.0	Hard weathered siltstone	2.13	2.35	7.9	6.5
D46	80.2	Hard fresh sandstone	2.43	2.43	16.0	16.9
PD100	83.0	Hard weathered mudstone	2.15	2.36	5.2	5.2
PD100	102.0	Hard weathered mudstone	2.04	2.17	11.3	8.5
PD100	116.5	Hard weathered siltstone	2.11	2.33	9.2	8.5
PD100	141.0	Hard fresh sandstone	2.23	2.30	9.9	8.6
PD100	149.0	Hard fresh mudstone	2.22	2.38	5•5	8.3
PD100	151.0	Hard fresh siltstone	2.20	2.33	15.0	8.1
PD101	99•5	Hard weathered sandstone	2.47	2.52	7.2	6.0
PD101	115.0	Hard weathered sandstone	2.15	2.30	6.9	4.5
PD101	127.0	Hard fresh mudstone	2.22	2.28	10.0	9•5
PD103	49.0	Hard weathered mudstone	2.04	2.34	5.6	4.2
PD103	88.0	Hard weathered sandstone	2.52	2.59	6.8	66.1

BUFFALO R. VIC. 1966







166 May 241

