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BUREAU OF MINERAL RESOURCES, GEOLOGY AND GEOPHYSICS

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MURRUMBIDGEE PARK DRIVE, TUGGERANONG, A.C.T.
GEOLOGICAL INVESTIGATION



by

R.C.M. GOLDSMITH

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SUMMARY

The Bureau of Mineral Resources, Geology and Geophysics has examined the geology along the route of the proposed Murrumbidgee Park Drive. The route is 14 km long and proceeds south from Kambah Pool Road through Tuggeranong to Lanyon.

The geology of the area consists of Silurian acid igneous rocks unconformably overlying shale, sandstone, and porphyritic rhyolite. High-level gravel terraces and alluvial basins occur along the length of the broad Murrumbidgee valley.

Two bridge sites, the Point Hut water-feature embankment, and some major road-cuts were investigated. Depths to suitable foundation determined by diamond-drilling range from 0-8 m at Tuggeranong Creek bridge site, 0-1 m at Strangers Creek bridge site, and 3-7 m at the Point Hut water-feature embankment. Stability problems are not expected at these sites, but a proposed 10 m deep cut south of Tuggeranong Creek is unfavourably oriented with respect to the jointing and a 45° batter is recommended.

INTRODUCTION

The Murrumbidgee Park Drive is to be constructed as the main arterial roadway through the centre of Tuggeranong, A.C.T. (see location sketch, Plate 1). The project - designed by Maunsell Partners, consulting engineers for NCDC - requires the construction of 13.5 km of three-lane dual carriageways extending from the present Kambah Pool Road in the north along the Murrumbidgee valley to Lanyon in the south (Plate 1).

The Department of Construction (DC) has requested the Bureau of Mineral Resources (BMR) to provide information on the general geological features along the alignment, and on the expected subsurface conditions at the Tuggeranong Creek and Strangers Creek bridge sites and at the Point Hut water-feature site; the water-feature embankment will carry the Murrumbidgee Park Drive. A preliminary evaluation of the batter stability of the major cuts was also required.

Site investigations - between March and June 1976 - included a DC augering and back-hoe program, and a DC/BMR diamond-drilling program of four holes at Tuggeranong Creek bridge site, one hole at the Strangers Creek bridge site, and four holes at the Point Hut water-feature embankment site. Water-pressure testing was carried out in one hole to test the rock permeability in drillhole PH4. Logs of drill-hole core and the water-pressure test results are presented in Appendix 1.

GENERAL GEOLOGY

The general geology of the Tuggeranong area is shown in Plate 1, and has been described previously for the Lanyon trunk sewer (Goldsmith, 1975). The Tuggeranong valley is underlain by an Upper Silurian sequence of acid volcanics and sediments. Sandstone and siltstone were deposited in association with early volcanic activity, as agglomerate and tuffaceous sandstone are common throughout the sequence, especially in the Pine Island area, where there is evidence of a major Silurian centre of volcanic activity. These sediments, which crop out around Pine Island and to the south and west of Point Hut, have since been folded about an axis striking 140° .

Unconformably overlying the sediments are acid volcanics. Dark grey dacitic tuff crops out in the south of the area, including the Lanyon Hills and the area west of Point Hut. Pale and dark grey rhyodacite and dark grey dacitic tuff crops out in the south of the area, including the Lanyon Hills and the area west of Point Hut. Pale and dark grey rhyodacite and dark grey dacitic tuff extend from Point Hut northwards to Kambah and Weston Creek. A faulted wedge of reddish purple and green-grey rhyodacite crops out on topographically high points between Barneys Hill and Mount Taylor, directly to the north.

In more recent geological time the Murrumbidgee River has incised its channel into its broad older valley. High-level terrace gravels of probable Quaternary age are up to 30 m above the present river level. The geomorphology of this area is now controlled by entrenching of the Murrumbidgee River into the older broad valley: alluvium is accumulating in depressions in the broad valley, and upslope the alluvium grades to colluvium, which is thickest at the foot of the hills.

ENGINEERING GEOLOGY

EXCAVATION CONDITIONS

The geology, excavation conditions, and stability characteristics along the proposed route are summarized in Plate 2. In general, excavation of cuts deeper than 3-4 m will require some blasting, as these are in topographically higher areas with skeletal soils overlying rock at 2-4 m. Rock crops out in some of the locations of proposed deep cuts, and blasting will be necessary from the surface.

BATTER STABILITY

General

The batter stability of cuts over 3 m deep has been summarized in Plate 2. Generally, cuts of about 3 m in the acid volcanic rocks can be safely battered to 70-80°. Only the batters on cuts deeper than 3 m and on those along hillslopes adjacent to the Murrumbidgee River are likely to fail unless specific precautions are taken. On these slopes, which have experienced a release of stress by unloading during valley formation, joints are generally open - especially on the east bank, where they dip towards the river - and thereby contribute to failure in batters on the east-facing slopes.

After the stability is assessed during construction, lower batter angles should be selected for the west-facing side:

- a) to reduce the number of joints or joint intersections undercut by the slope, and
- b) to reduce the possibility of slumping in the colluvium and terrace gravels; cuts exposing these materials should be well drained.

Zones of potential instability identified during construction may require restraint by the setting of rock-bolts and wire mesh and/or the provision of drainage.

Major cuts

Stn 3700-3900 m

A 4.5-m cut at station 3700 to 3900 intersects the contact between the tuffaceous siltstone and sandstone and the dark grey dacitic tuff. This contact dips about 20° to the west, and is probably deeply weathered. Outcrop in the area is sparse and details of jointing are not known; however, a regional near-vertical joint set strikes 140° and is favourably oriented with respect to the cut.

Stn 10400-10500 m (Plate 3)

A deep section of cut is proposed between stations 10400-10500, where Murrumbidgee Park Drive will cross Lanyon trunk sewer (Goldsmith, 1975). Stations 375.4 to 491 of the buried pipeline will be within this road-cut. The roadway and associated cuts will be constructed first; then it is planned to excavate the pipeline trench (invert level 551 m RL) from the north, and approach as close as is feasible to the top of the batter slope of the road-cut (see section A-B in Plate 3). At this point trenching will cease, and a tunnel about 15 m long will be excavated beneath the batter slope to manhole (MH) 2.

This procedure will leave the batter slope for the roadway undisturbed, thus maintaining stability.

No rocks crop out over the immediate site, but dacite crops out on a hill to the northeast, where moderately to slightly weathered rock contains partly open joints with a rough surface. Two sets were observed: 90/070-080 and 90/165; shallow-dipping joints may have formed by unloading due to the downcutting of the Murrumbidgee River in its valley.

Drill hole LTS3 (Plate 3), near MH2, intersected moderately weathered dacitic tuff at 4 m depth, but a closely fractured zone between 6.5 m and 8.4 m comprises highly to extremely weathered rock. Hard and strong slightly weathered to fresh rock occurs from 8.4 m to the bottom of the hole at 14.05 m. Joints are moderately to widely spaced and many dip at 45° (probably to the west), parallel to veins of sericite and epidote (some weathered to clay).

Seismic traverse 3 (Plate 3) is one of a number of traverses completed along the line of the Lanyon trunk sewer (Koelle, 1975). Results indicate fresh rock at 8-9 m, which agrees with drilling results, and moderately weathered rock (seismic velocity 1500-1600 m/s) between 3.5 and 9.0m.

As a joint set dips 45° W, the west-facing batter for the road-cut should be no steeper than 45° . The east-facing batter can be cut to a steeper angle, about $60-70^{\circ}$, as the joints dip into the slope.

TUGGERANONG CREEK BRIDGE (Plate 4)

The proposed bridge consists of two carriageways spanning 90 m between abutments; each carriageway will be supported by two piers spaced 30 m apart. Green-grey dacitic tuff crops out on the steeper bluffs adjacent to Tuggeranong Creek, where it is moderately spaced. There are no dominant joint sets, but a general north-south strike is common; the dip ranges from 90° to 30° , with shallow joints dipping downslope.

Between the two bluffs, shown on the lower left and upper right of Plate 4, is a zone of closely fractured, highly weathered dacitic tuff which crops out adjacent to the southern bluff and on the bank of the creek in the north. Drill hole TCB4 also intersected this rock. This fractured zone appears to dip $40-50^{\circ}$ to the east.

Northbound carriageway

The bedrock surface dips generally west at 5° - from the ground surface at the east abutment to 7.6 m depth at the west abutment.

East Abutment. Nearby outcrops of moderately weathered dacite with closely to moderately spaced joints indicate that this abutment will have good foundations less than 2 m from the surface.

Pier 1. Drill hole TCB5, at the site of this pier, intersected moderately weathered dacite at 1 m depth. The rock mass contains joints with limonite and clay coatings and some clay seams 1 cm thick extending to 5.25 m depth. It would not be necessary to excavate deeper than 1-2 m for a stable foundation, unless a major zone of clay seams and closely spaced joints is encountered during construction.

Pier 2. This site is located in Tuggeranong Creek. Less than about 1 m of alluvium rests on extremely weathered rock with moderately weathered rock suitable for foundation estimated at 3-4 m. The rock in nearby drill hole TCB4 is closely fractured dacitic tuff, and any deeply weathered pockets should be cleaned out and grouted.

West Abutment. Drill hole TCB4 indicated that 4.5 m of silty sand and silty clay overlies highly weathered fractured dacitic tuff. At 7.6 m the rock is moderately weathered, but the rock mass is loose, with closely spaced joints and some clay; foundation for the abutment should be possible at 7.6 m

Southbound carriageway

East Abutment. Moderately weathered rock should be found at 5 m depth, but an airphoto-lineament representing a possible shear zone passes close to the northeast edge of the abutment, and competent rock may be deeper in this section.

Pier 3. Drill hole TCB7 encountered moderately to slightly weathered rock suitable for pier foundation at 5 m. The rock mass is hard and strong, and although the joints are weathered - especially vertical joints - no clay is present.

Pier 4. Competent rock suitable for foundation is estimated at 4-5 m depth underlying about 3-4 m of alluvium and slopewash. The rock at this site, as indicated by nearby outcrops, is hard and tight with moderately spaced joints.

West Abutment. Drill hole TCB6 indicates rock suitable for foundation at 4.3 m. Alluvium and cemented slopewash material overlies weathered rock at 4 m with slightly weathered hard and strong dacite at 4.3 m.

STRANGERS CREEK BRIDGE

This bridge with twin 35-m spans crosses Strangers Creek (Plate 5) 300 m east of Pine Island Reserve. Moderately weathered rock was reached at 1.5 m in drill hole SCB1. As rubbly outcrop is abundant over both abutment sites, neither foundation conditions nor stability are expected to pose any problems. Rock suitable for abutment foundations is expected at less than 2 m depth.

POINT HUT WATER FEATURE

This site, 550 m southeast of Point Hut Crossing, is for an earth embankment - 259 m long and up to 11 m high - that will impound a small lake to the east (Plate 6). The Murrumbidgee Park Drive occupies the top of the embankment between stations 4865 and 5135 m. Foundation conditions for the embankment were determined by four diamond-drill holes. One drill hole, PH4, was water-pressure tested.

The point where the embankment crosses the creek is the narrowest part of the water-catchment area of about 10 km², which extends back past Tuggeranong Hill and includes the Lanyon North Basin drainage area (Kellett, in prep.). A considerable volume of surface runoff will pass through the constriction at the embankment site.

Drill-hole logs indicate that 3.5-4.5 m of silty sandy alluvium overlies very hard slightly weathered to fresh dacite. Of the four drill holes on the site drill hole PH2 (on the upper slope of a low ridge) shows the greatest thickness of moderately weathered rock, with slightly weathered rock occurring at 10.5 m. Moderately weathered rock should be suitable for the embankment foundation.

Suitable foundation for the embankment is at 4-5 m in the centre of the valley, where sandy alluvium lies almost directly on competent rock. Downstream, at the end of the proposed spillway chute, drill hole PH3 intersected open-jointed weathered rock to a depth of 7.5 m, where the rock is slightly weathered; a steeply dipping fracture zone between 9.5 - 10.9 m will need grouting.

The section in Plate 6 summarizes the subsurface conditions. Alluvium and the highly weathered rock will have to be stripped to provide a suitable foundation for the dam.

Drill hole PH4 was water-pressure tested and losses were calculated at no greater than 1 lugeon (Appendix 1). However this was measured in the hole with the best quality rock found at the site; leakages would be greater through rock near drill hole PH3. When stripping is completed, possible leakage paths can be identified and mapped more easily, and fracture in the rock can be grout.

CONCLUSIONS AND RECOMMENDATIONS

1. No major stability problems are expected in road-cuts along the proposed route. Most batters can be cut to 60° or steeper, apart from the cut 10 m deep south of Tuggeranong Creek, where unfavourably oriented joints require a batter slope of 45°.

2. Most road-cuts will have to be partly excavated by blasting as they pass through topographically high rocky areas.
3. Depth to rock suitable for foundations at the Tuggeranong Creek bridge site varies between 1 and 8 m. For the Northbound Carriageway, depth to competent rock ranges from 1 m on the east abutment to 7.6 m on the west abutment; this bedrock surface dips 5° evenly to the west. Foundations for the southbound carriageway tend to be more uniform at 4-5 m depth.
4. The Strangers Creek bridge site has suitable foundations for the abutments at about 2 m from the surface.
5. The Point Hut water feature is sited in the narrow downstream end of a large alluvial basin. Alluvium is 3.5-4.5 m thick and depth to suitable foundation ranges from 4-7 m in the centre of the valley to 3 m on the upper flanks. Alluvium and highly weathered rock should be stripped to expose moderately weathered rock which is expected to provide a suitable foundation.
6. It is recommended that a geologist visit the sites investigated during construction to map exposures and advise on stability of the cuts and foundation conditions.

REFERENCES

- GOLDSMITH, R.C.M., 1975 - Lanyon trunk sewer, geological investigations, Tuggeranong, A.C.T. Bur. Miner. Resour. Aust. Rec. 1975/173 (unpubl.).
- KOELLE, A., 1975 - Lanyon Trunk Sewer, seismic survey. Central Testing Res. Labs. Tech. Rep. 156.

APPENDIX 1

GEOLOGICAL LOGS OF DRILL HOLES, AND RESULTS OF
WATER-PRESSURE TESTS

TCB 4
HOLE NO.

SHEET 1 OF 1

[illegible]

TCB 5
HOLE NO.

SHEET 1 OF 1

[illegible]

BUREAU OF MINERAL RESOURCES, GEOLOGY & GEOPHYSICS		PROJECT <u>MURRUMBIDGEE PARK DRIVE</u> LOCATION <u>TUGGERANONG CREEK BRIDGE SITE</u>				TCB 6 HOLE NO.	
GEOLOGICAL LOG OF DRILL HOLE		ANGLE FROM HORIZONTAL (θ) COORDINATES <u>E 203984.4N 589524.0</u>				DIRECTION R.L. OF COLLAR <u>543.6</u>	
						SHEET <u>1</u> OF <u>1</u>	

Rock Type and Degree of Weathering	Description Lithology, colour, strength, etc	Casing	Graphic Log	Lift and % core recovery	Depth and size of Core	Fracture Log	RQD	Defect Frequency Intercept Angle 0 30 60 80 90	Structures Joints, veins, seams, faults, etc	Water Level	Water Pressure Test Losses (Lugeons) *
CEMENTED SLOPEWASH	Dark grey brown, core can be broken by hand										
DACITE EW-HW	Broken rock, gray brown.										
DACITE SW	Pink and grey, porphyritic. Some pinkish-red Fe-oxide or jasper zones (hard). Rock is hard and strong + brittle, does not crumble. Texture irregular										
	End of Hole 7.0m										

Drill type MOLE

Feed CROWD PRESSURE

Core barrel type NMLC

Driller D of C (W. HART)

Commenced 13/4/76

Completed 13/4/76

Logged by R. GOLDSMITH

Vertical scale 1 cm = 1 m

Checked by

Notes

Fracture Log — Number of fractures per 25 cm of core. Zones of core loss blacked in.

Bedding and Joint Planes — Angles are measured relative to a plane normal to the core axis.

Defect Frequency — Number of natural defects (shears, joints, fractures) per 25 cm of core occurring at specified intercept angle range.

Water Level Measurements — Level when hole in progress at specified depth.
 Level in completed hole on specified date.

Water Pressure Tests

* Values in lugeons should be read in conjunction with computation sheets. Test sections are indicated by blacked in strips.

Core Photograph Negative No.

Depth (m)	Black & White	Colour

M(Pf)146

I55/A16/1671

SHEET 1 OF 1

Drill type <u>MOLE</u> Feed <u>CROWD PRESSURE</u> Core barrel type <u>NMLC</u> Driller <u>D & C (W.H.A.B.)</u> Commenced <u>8/4/76</u> Completed <u>8/4/76</u> Logged by <u>R.GOLDSMITH</u> Vertical scale <u>1 cm = 1 m</u> Checked by _____	<div style="text-align: center;">Notes</div> <p><i>Fracture Log — Number of fractures per 25 cm of core. Zones of core loss blacked in.</i></p> <p><i>Bedding and Joint Planes — Angles are measured relative to a plane normal to the core axis.</i></p> <p><i>Defect Frequency — Number of natural defects (shears, joints, fractures) per 25 cm of core occurring at specified intercept angle range.</i></p> <p><i>Water Level Measurements — ∇ Level when hole in progress at specified depth. \square Level in completed hole on specified date.</i></p>	<div style="text-align: center;">Water Pressure Tests</div> <p>* Values in lugeons should be read in conjunction with computation sheets. Test sections are indicated by blacked in strips.</p> <div style="text-align: right;">Core Photograph Negative No.</div> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="width: 33%;">Depth (m)</th> <th style="width: 33%;">Black & White</th> <th style="width: 33%;">Colour</th> </tr> </thead> <tbody> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> </tbody> </table> <div style="text-align: right; margin-top: 10px;"><i>M(Pf)/146</i></div>	Depth (m)	Black & White	Colour																																	
Depth (m)	Black & White	Colour																																				

LTS 3
HOLE NO. 3

SHEET 1 OF 1

ANGLE FROM HORIZONTAL (θ) - 90°
 COORDINATES E 204108 N 589367
 DIRECTION _____
 R.L. OF COLLAR 560.5 m

Drill type MOLE PIONEER	Notes	Water Pressure Tests
Feed CROWD PRESSURE	<i>Fracture Log — Number of fractures per 25 cm of core. Zones of core loss blacked in.</i>	* Values in lugeons should be read in conjunction with computation sheets. Test sections are indicated by blacked in strips.
Core barrel type ROCK ROLL — 0-2m, NMLC 2-14m.	<i>Bedding and Joint Planes — Angles are measured relative to a plane normal to the core axis.</i>	
Driller D. O'P. C.	<i>Defect Frequency — Number of natural defects (shears, joints, fractures) per 25 cm of core occurring at specified intercept angle range.</i>	
Commenced 1/4/76	<i>Water Level Measurements — \overline{I} Level when hole in progress at specified depth. \underline{I} Level in completed hole on specified date.</i>	Core Photograph Negative No.
Completed 6/4/76	EW — Extremely weathered	Depth (m) Black & White Colour
Logged by R. GOLDSMITH	HW — Highly weathered	_____
Vertical scale 1cm = 1m	MW — Moderately weathered	_____
	SW — Slightly weathered	_____
	Fr — Fresh	_____
Checked by _____		_____
		<i>M(Pf)146</i>

GEOLOGICAL LOG OF DRILL HOLE

ANGLE FROM HORIZONTAL (θ) 90° DIRECTION _____
COORDINATES E 204 971.6 N 5874 97.4 R.L. OF COLLAR 55-7.4SHEET 1 OF 1

Rock Type and Degree of Weathering	Description Lithology, colour, strength, etc	Casing Graphic Log	Lift and % core recovery	Depth and size of Core	Fracture Log	RQD	Defect Frequency Intercept Angle	Structures Joints, veins, seams, faults, etc	Water Level	Water Pressure Test Losses (Lugeons) *
							0 30 60 80 90			
DACITE EW	Slopewash and residual MW dacite fragments									
MW	Brown-grey feldspars white (altered), rock is mod. soft.									
DACITE SW	Dark green grey, phono- crysts of quartz and plag 3-7mm in size. less common orth 2-3mm Some reddish groundmass in places. Brecciated, but welded, rock occurs at 4 m.									
Fresh stained	Numerous small veins of sericite									
	END OF HOLE 5-95									

Drill type MOLE
Feed CRAWD PRESSURE
Core barrel type NMLC
Driller D. P. C. (W. HART)
Commenced 26/5/76
Completed 27/5/76
Logged by R. GOLDSMITH
Vertical scale 1 cm = 1 m

Notes

Fracture Log — Number of fractures per 25 cm of core. Zones of core loss blocked in.
Bedding and Joint Planes — Angles are measured relative to a plane normal to the core axis.
Defect Frequency — Number of natural defects (shears, joints, fractures) per 25 cm of
core occurring at specified intercept angle range.
Water Level Measurements — — Level when hole in progress at specified depth.
— Level in completed hole on specified date.EW — Extremely weathered
MW — Moderately "
SW — slightly "

Water Pressure Tests

* Values in lugeons should be read
in conjunction with computation
sheets. Test sections are indicated
by blocked in strips.Core Photograph Negative No.
Depth (m) Black & White Colour
.....
.....
.....
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.....
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Checked by

M(Pf)146

I55/A16/1673

GEOLOGICAL LOG OF DRILL HOLE

ANGLE FROM HORIZONTAL (°) 90° DIRECTION
COORDINATES E 206231.6 N 584623.4 R.L. OF COLLAR 564.33SHEET 1 OF 1

Rock Type and Degree of Weathering	Description Lithology, colour, strength, etc	Casing Graphic Log	Lift and % core recovery	Depth and size of Core	Fracture Log	RQD	Defect Frequency Intercept Angle	Structures Joints, veins, seams, faults, etc	Water Level	Water Pressure Test Losses (Lugeons) #
							0 30 60 80 90			
ALLUVIUM	Black silty sand and gravel lenses, statified, some sub-rounded to angular fragments of rhyolite.			0 1 2 3 4 5 6 7 8 9 10 11 12						
GRAVEL + COLLUVIUM	Loose and broken sub-rounded to angular rock fragments.			15 40 60 100 100 100 100 100 100 100 100 100						
DACITE HW								10cm weath. jt. Clay Rock mass loose some tight sections, joints moderately closely spaced and open, some rough		
DACITE MW	Mid grey-brown, moderately hard and strong.							From 6m joints are moderately spaced, partially open but no clay is present. Jts gen. contain limonite + associated with sericite and calcite veins. Rock mass as a whole is tight and strong.		
DACITE SW	Dark grey-brown, porphyritic texture, phenocrysts of quartz and green altered plagioclase 2-5 mm in size, biotite is also present.							From 9.2m staining is absent. Joints tight and mod spaced. Some minor broken zones. Epidote and calcite veins. Very hard and strong rock mass.		
DACITE Fresh stained										
DACITE Fresh	Blue-grey, very hard and strong, slow drilling - 90cm in 2 1/2 hrs for last run. Porphyritic quartz and plag. up to 5mm.									
	End of Hole 11.95m.									

Drill type MOLE
Feed CROWD PRESSURE
Core barrel type MLC 23-
8.7m, NX 8.7m-12m
Driller DHC (W.HART)
Commenced 5/6/76
Completed 13/6/76
Logged by R. GOLDSMITH
Vertical scale 1cm = 1m

Checked by

Notes

Fracture Log - Number of fractures per 25 cm of core. Zones of core loss blocked in
Bedding and Joint Planes - Angles are measured relative to a plane normal to the core axis
Defect Frequency - Number of natural defects (shears, joints, fractures) per 25 cm of core occurring at specified intercept angle range.

Water Level Measurements - ▽ Level when hole in progress at specified depth.
▽ Level in completed hole on specified date.

HW - Highly weathered
MW - Moderately weathered
SW - Slightly weathered.

Water Pressure Tests

* Values in lugeons should be read in conjunction with computation sheets. Test sections are indicated by blocked in strips.

Core Photograph Negative No.

Depth (m)	Black & White	Colour

M(PF)146

155/A16/1674

PROJECT MURRUMBIDGEE PARK DRIVE
LOCATION POINT HUT NUTRIENT POND, SOUTHERN END
OF EMBANKMENT
ANGLE FROM HORIZONTAL (θ) 90° DIRECTION
COORDINATES E 206281.6 N 584542.1 R.L. OF COLLAR 569.04HOLE NO. PH2

GEOLOGICAL LOG OF DRILL HOLE

SHEET 1 OF 1

Rock Type and Degree of Weathering	Description Lithology, colour, strength, etc	Casing Graphic Log	Lift and % core recovery	Depth and size of core	Fracture Log	RQD	Defect Frequency Intercept Angle	Structures Joints, veins, seams, faults, etc	Water Level	Water Pressure Test Losses (Lugeons) *
							0 30 60 80 90			
DACITE EW	Mid brown, weak - can be broken by hand. Crushed core has a clayey sand texture			30	1	0		Rock decomposed but structure preserved in core, sand + clay infilling along joints. Very open + loose rock.		
DACITE EW-HW	Core more solid but still weak.			100	3	0				
DACITE MW	Grey brown, medium to coarse grained - porphyritic with phenocrysts of quartz and plagioclase. From 8m - 12m plagioclase is more dominant over quartz. Core is moderately hard and strong but weakened by joints			100	4	55	1 3 1 2	2cm epidote vein		
				100	5	60	2 1 1 3 2 1	clay jt (clay)		
				100	6	80	1 2 2 8 1	5cm fract. zone		
				100	7	55	2 2	fract. zone		
				100	8	60	3 1 5 3 2 1	1cm epidote vein		
				100	9	60	3 2 1 1 2	fract + weath joint		
				100	10	30	2 2 4 1 3 1	open joint 1cm epidote vein		
				100	11	50	1 3 2	fractured + weathered zone		
DACITE SW	Dark grey, porphyritic			100	12	85	2 1 1	Hard and strong, but joints partially open and limonite stained, no clay.		
	END OF HOLE 12.0m.									

Drill type MOLE
Feed CROWD PRESSURE
Core barrel type NMLC
Driller DHC (W HART)
Commenced 1/6/76
Completed 4/6/76
Logged by R. GOLDSMITH
Vertical scale 1cm = 1m
Checked by

Notes

Fracture Log - Number of fractures per 25 cm of core. Zones of core loss blacked in.
Bedding and Joint Planes - Angles are measured relative to a plane normal to the core axis.
Defect Frequency - Number of natural defects (shears, joints, fractures) per 25 cm of core occurring at specified intercept angle range.
Water Level Measurements - ✓ Level when hole in progress at specified depth.
✓ Level in completed hole on specified date.EW - Extremely weathered
HW - Highly
MW - Moderately
SW - Slightly

Water Pressure Tests

* Values in lugeons should be read in conjunction with computation sheets. Test sections are indicated by blacked in strips.

Core Photograph Negative No.

Depth (m)	Black & White	Colour

M(PF)146

155/A16/1675

GEOLOGICAL LOG OF DRILL HOLE

ANGLE FROM HORIZONTAL (θ) 90° DIRECTION
COORDINATES E206220.5N 584642.8 R.L. OF COLLAR 562.97SHEET 1 OF 1

Rock Type and Degree of Weathering	Description Lithology, colour, strength, etc	Casing Graphic Log	Lift and % core recovery	Depth and size of Core	Fracture Log	RQD	Defect Frequency Intercept Angle	Structures Joints, veins, seams, faults, etc	Water Level	Water Pressure Test Losses (Lugeons) *
							0 30 60 90			
SAND AND SILTY ALLUVIUM										
DACITIC TUFF EW	Completely decomposed sandy-gravel texture, clay matrix. Some compact clayey core, (brown and grey clay)		100					No structure visible		
DACITIC TUFF HW - MW			100							
DACITIC TUFF SW (MW - HW zones)	Blue-grey, hard and strong rock, some feldspar discoloration and Feox on joints very few veins		50							
DACITIC TUFF MW	grey-brown, stained. Mod. hard and strong but friable on joints		100					Rock mass is loose joints open and weathered, crumbly zones, closely spaced joints, dip 0-30° + 90°.		
SW	No stains		100							
DACITIC TUFF Fresh	Blue-grey porphyritic minor veining of cc and ep. (along joint planes) some slicken-sides have replaced veins One core length from 13.83 - 14.7. No stains		100					9.5-10.9 Broken and fractured zone, rock strong between joints, but is fragmented in joints - washed out by NX core (blocked off NMLC).		
			100					From 10.9m to BOM the rock mass is tight and joints are tight and planar (mod. to widely spaced)		
			100					crushed jt. 0.5m - 60° core breaks up a bit when boxed, but are parallel to veins.		
	END OF HOLE 14.7m									

Drill type FOX 108LS -22'
Feed GRAVITY FEED
Core barrel type NMLC
3-5-10-3, NX 10-3-14.7
Driller D & C (W. HART)
Commenced 21/6/76
Completed 25/6/76
Logged by R. GOLDSMITH
Vertical scale 1 cm = 1m

Notes

Fracture Log - Number of fractures per 25 cm of core. Zones of core loss blacked in.
Bedding and Joint Planes - Angles are measured relative to a plane normal to the core axis.
Defect Frequency - Number of natural defects (shears, joints, fractures) per 25 cm of core occurring at specified intercept angle range.

Water Level Measurements - ✓ Level when hole in progress at specified depth.
✓ Level in completed hole on specified date.

EW - Extremely weathered
HW - Highly "
MW - Moderately "
SW - Slightly "

Water Pressure Tests

* Values in lugeons should be read in conjunction with computation sheets. Test sections are indicated by blacked in strips.

Core Photograph Negative No.

Depth (m) Black & White Colour

M(Pf)146

I55/A16/1676

GEOLOGICAL LOG OF DRILL HOLE

Rock Type and Degree of Weathering	Description Lithology, colour, strength, etc	Casing Graphic Log	Lift and % core recovery	Depth and size of core	Fracture Log	RQD	Defect Frequency				Structures Joints, veins, seams, faults, etc	Water Level	Water Pressure Test Losses (Lugeons) *		
							Intercept Angle								
							0	30	60	90					
ALLUVIUM	Silty sand and gravel. Stratified.			ROCK	ROLL	1									
						2									
						3									
						4									
				30											
EW, HW, MW.	Brown - to green gray, mod soft.			DACITE SW	100	5	10	4	1						
DACITE SW						5	4	2							
						80	4	3	1					5 cm thick crushed joint	
						100	6	10	2					Broken core	
						100	7		1					Joints weathered but gen. tight, horizontal. Some 60-70°	
						100	8		1						
						80	3	2	1						From 6.3m the rock mass is very hard and tight. Very few joints (some machine breaks). Some localized broken zones which probably contribute to water loss (minor). They are not weathered. Joints are calcite and epidote lined, no discoloration or weathering.
						100	9	2	1						
						100	10		1						
						80	2	1	1						
						100	12								
						100	13		1						
						100	14		2						
						95									
						100	15								
	END OF HOLE 16.0m							16							

1 LUGEON

0 LUGEON

Drill type FOX 108LS-22'

Feed GRAVITY

Core barrel type NMLC

Driller D. C. (W. HART)

Commenced 28/6/76

Completed 9/7/76

Logged by R. GOLDSMITH

Vertical scale 1 cm = 1 m.

Checked by

Notes

Fracture Log - Number of fractures per 25 cm of core. Zones of core loss blacked in.

Bedding and Joint Planes - Angles are measured relative to a plane normal to the core axis.

Defect Frequency - Number of natural defects (shears, joints, fractures) per 25 cm of core occurring at specified intercept angle range.

Water Level Measurements - ∇ Level when hole in progress at specified depth. ∇ Level in completed hole on specified date.

Water Pressure Tests

* Values in lugeons should be read in conjunction with computation sheets. Test sections are indicated by blacked in strips.

Core Photograph Negative No.

Depth (m) Black & White Colour

M(Pf)146

155/A16/1677

WATER PRESSURE TESTS
REDUCTION OF FIELD RESULTSPROJECT POINT HUT WATER FEATUREFEATURE EMBANKMENT SITEANGLE FROM HORIZONTAL (°) -90DIRECTION ---R.L. OF COLLAR 565.29SIZE OF HOLE NMLCLOCATION POINT HUT A.C.T.PACKER TYPE HYDRAULICDRILL LOG REF. ---HOLE NO. PH4SHEET 1 OF 1

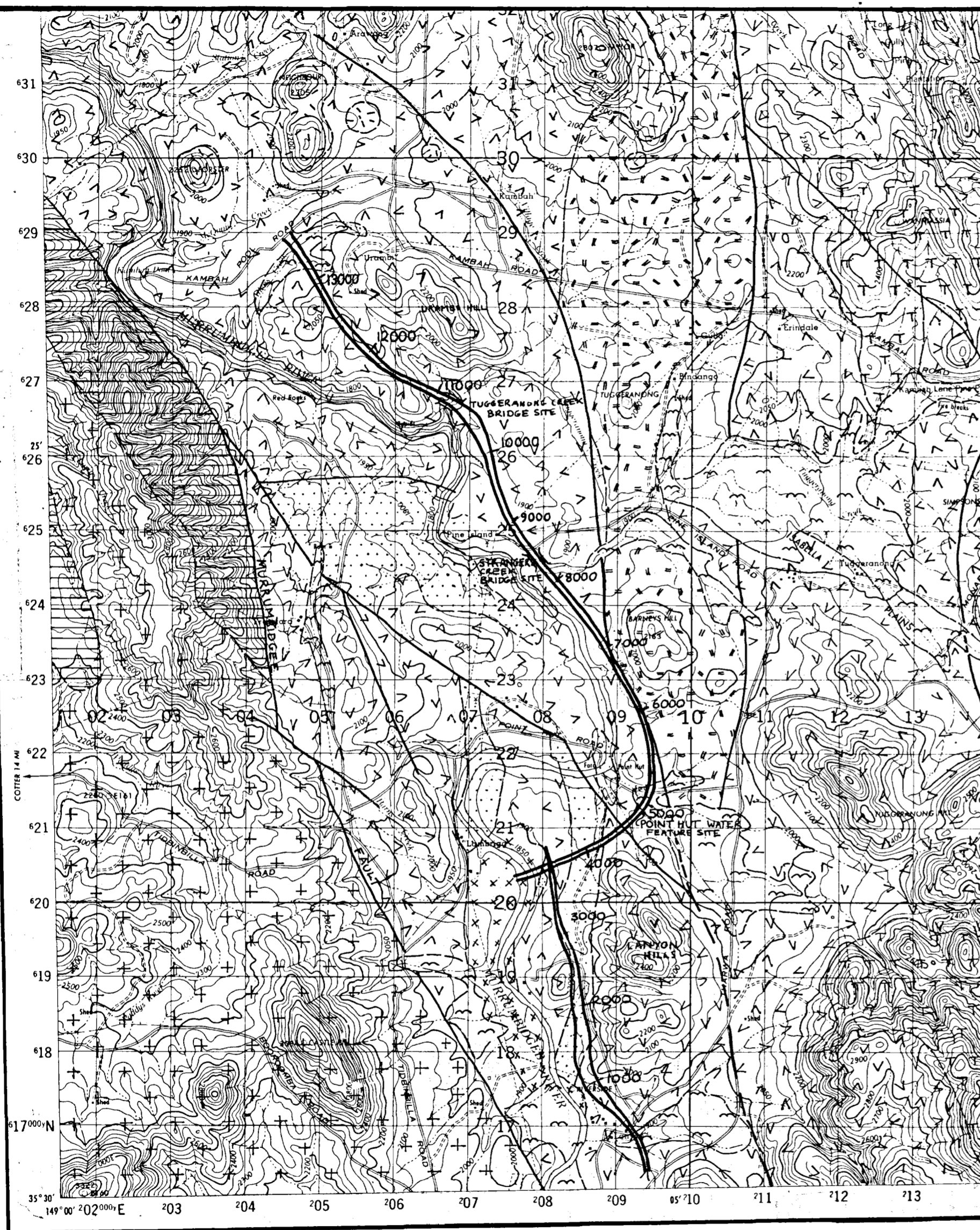
Date	Section Tested		Time of Test (min)	Gauge Pressure (kPa)	Water Meter Reading		Water Loss (l)	Leakage Rate (l/m)	Length of Test Section (m)	Conversion Factor or (1/8 m of NMLC)	Slope Depth to Standing Water (m)	Slope Ht. Gauge to Collar (m)	Length & Size of Supply Line (m)	Water Column Pressure (kPa)	Friction Losses		Effective Test Pressure (kPa)	Water Loss l/min/m	Water Pressure Test Loss in l/min/m		Remarks
	From (m)	To (m)			Start (l)	Finish (l)									Supply Line (kPa)	Packer (kPa)			0.5	1.0	
7/7/76	6.0	12.1	5	80	0	0	0	0	6.1	1	2.3	0	6.0	22.5	---	---	102.5	0			1 LUGEN
"	"	"	"	80	0	0	0	0	"	"	"	"	"	"			102.5	0			
"	"	"	"	140	0	3.5	3.5	0.7	"	"	"	"	"	"			162.5	0.11			
"	"	"	"	140	0	0.5	0.5	0.1	"	"	"	"	"	"			162.5	0.02			
"	"	"	"	140	0	0	0	0	"	"	"	"	"	"			162.5	0			
"	"	"	"	200	0	5	5	1	"	"	"	"	"	"			222.5	0.16			
"	"	"	"	200	0	4.5	4.5	0.9	"	"	"	"	"	"			222.5	0.15			
"	"	"	"	140	0	0	0	0	"	"	"	"	"	"			162.5	0			
"	"	"	"	140	0	0	0	0	"	"	"	"	"	"			162.5	0			
"	"	"	"	80	0	0	0	0	"	"	"	"	"	"			102.5	0			
"	"	"	"	80	0	0	0	0	"	"	"	"	"	"			102.5	0			
8/7/76	10.0	16.0	5	140	0	0	0	0	6.0	"	2.15	"	12.0	21.1			161.1	0			0 LUGEN
"	"	"	"	140	0	0	0	0	"	"	"	"	"	"			161.1	0			
"	"	"	"	200	0	0	0	0	"	"	"	"	"	"			221.1	0			
"	"	"	"	200	0	0	0	0	"	"	"	"	"	"			221.1	0			
"	"	"	"	400	0	0.5	0.5	0.1	"	"	"	"	"	"			421.1	0.02			
"	"	"	"	400	0	0	0	0	"	"	"	"	"	"			421.1	0			
"	"	"	"	200	0	0	0	0	"	"	"	"	"	"			221.1	0			
"	"	"	"	200	0	0	0	0	"	"	"	"	"	"			221.1	0			
"	"	"	"	140	0	0	0	0	"	"	"	"	"	"			161.1	0			
"	"	"	"	140	0	0	0	0	"	"	"	"	"	"			161.1	0			
"	"	"	"	200	0	0	0	0	"	"	"	"	"	"			221.1	0			
"	"	"	"	200	0	0	0	0	"	"	"	"	"	"			221.1	0			
"	"	"	"	400	0	0	0	0	"	"	"	"	"	"			421.1	0			
"	"	"	"	400	0	0	0	0	"	"	"	"	"	"			421.1	0			

* Values are read from appropriate correction graphs

+ If $l \leq a$, $p = 9.8 \sin \theta (l+m)$; if $l > a$, $p = 9.8 \sin \theta n$ W.P.T. Vertical scale 1 cm = 0.5 l/min/mCalculated by R.C. Goldsmith
Checked by ---

M(PF) 147

155/A16/1678



MURRUMBIDGEE PARK DRIVE General Geology, Tuggeranong, A.C.T.

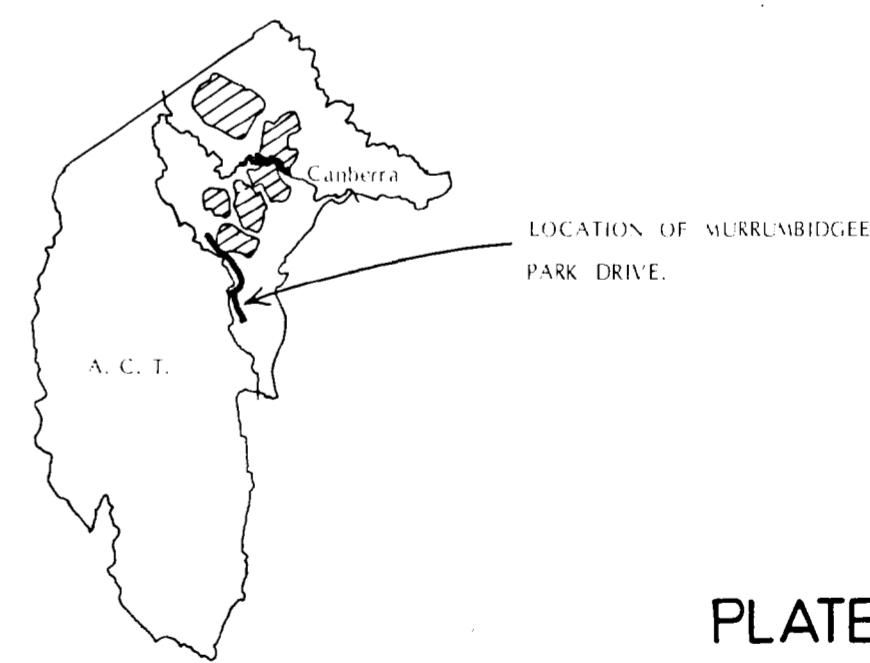
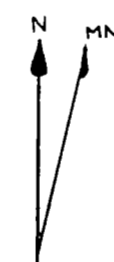
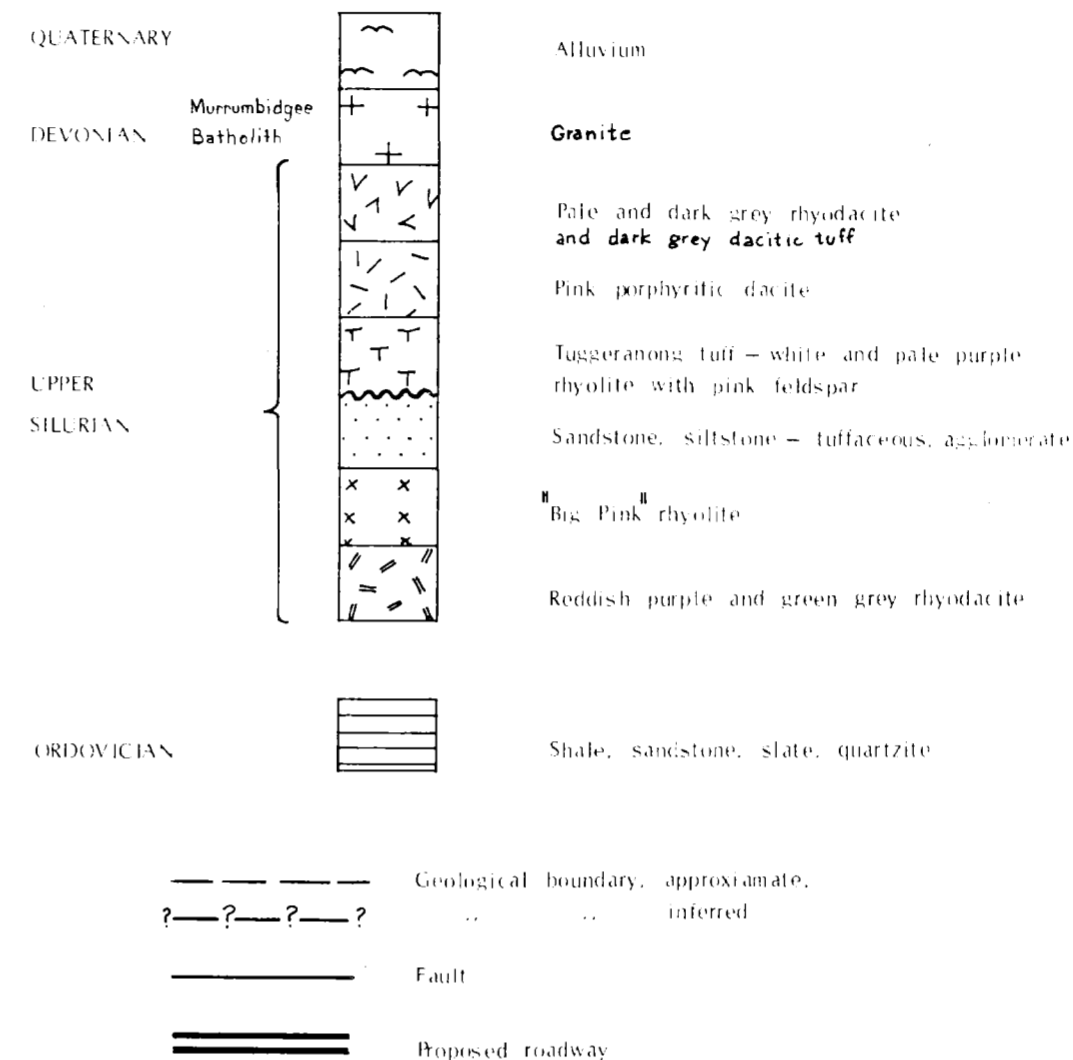
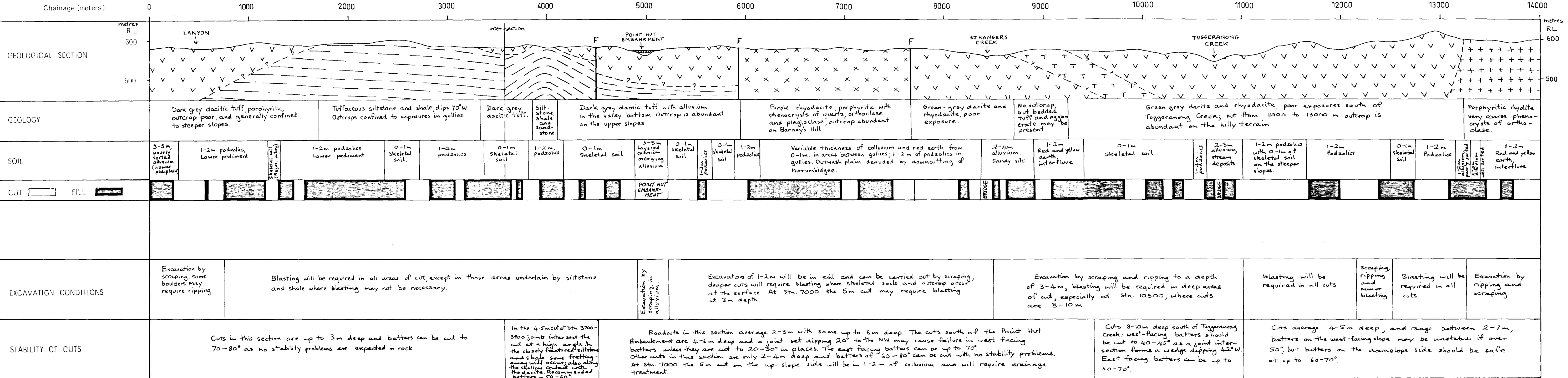


PLATE 1

AMENDMENTS				SCALE		COMMONWEALTH OF AUSTRALIA BUREAU OF MINERAL RESOURCES CANBERRA, A.C.T.	
No.	Description	Author	Checked	1:50 000		TITLE GENERAL GEOLOGY TUGGERANONG A.C.T.	
A1				Base map/survey 1:50 000 SHEET 8727-III		PROJECT MURRUMBIDGEE PARK DRIVE	
A2				Geology by Henderson, Goldsmith, Gardner, Compiled and checked by Jackson, Rossiter, Saltet, et. al.		To accompany Record 1976/106	
A3				R.G. GOLDSMITH Project geologist		Checked and approved D.C. PORCELL Senior geologist	
A4				E.G. WILSON Supervising geologist		Drawn by T.H.	
A5						Drawing No. 155/A16/1662	

SOUTH

NORTH

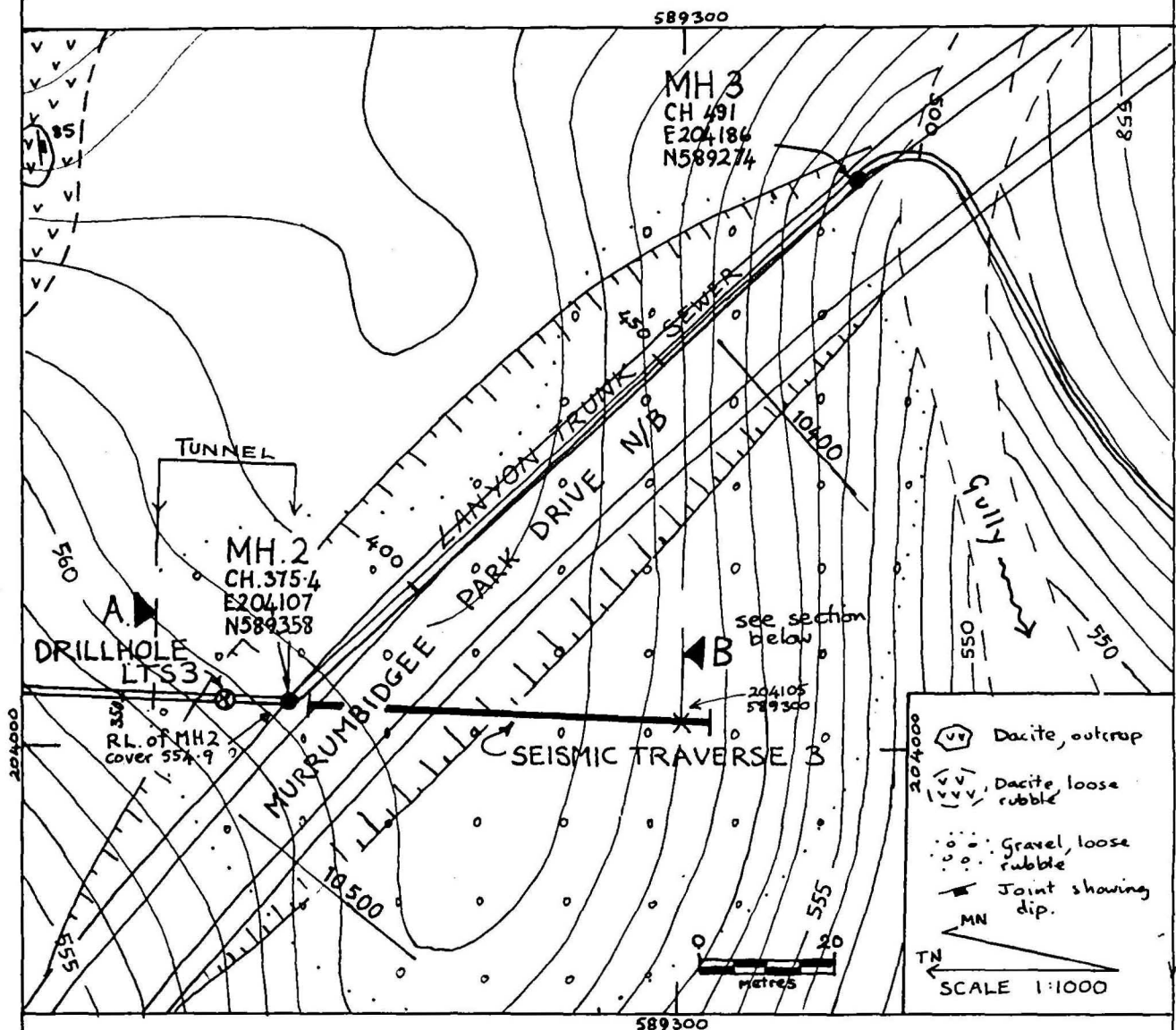


Record 1976/106

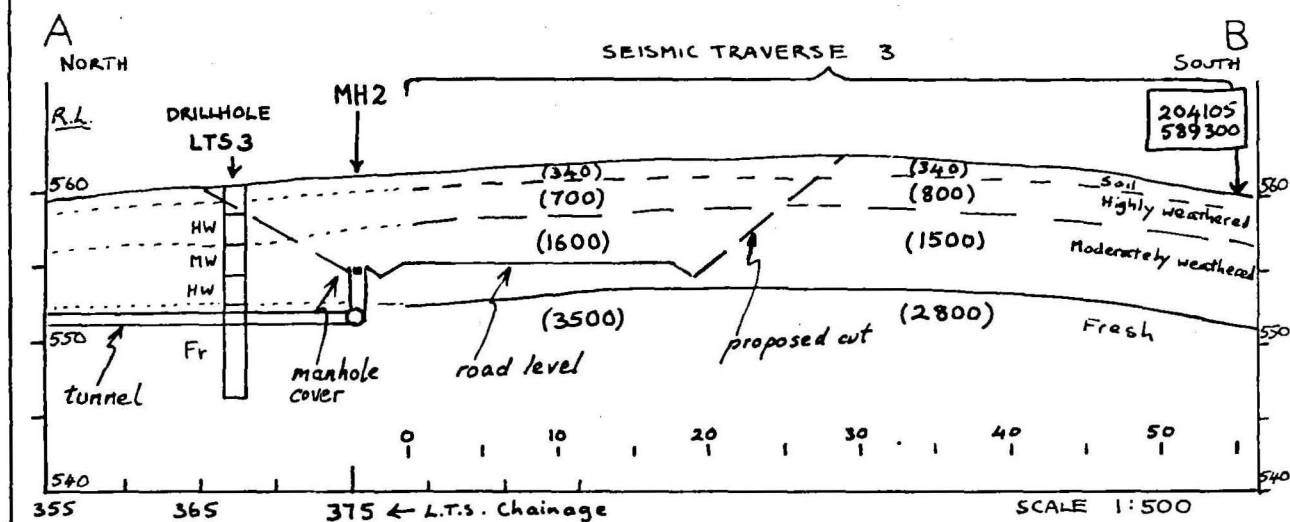
MURRUMBIDGEE PARK DRIVE
INTERPRETATIVE GEOLOGICAL SECTION ALONG PROPOSED CENTRELINE.

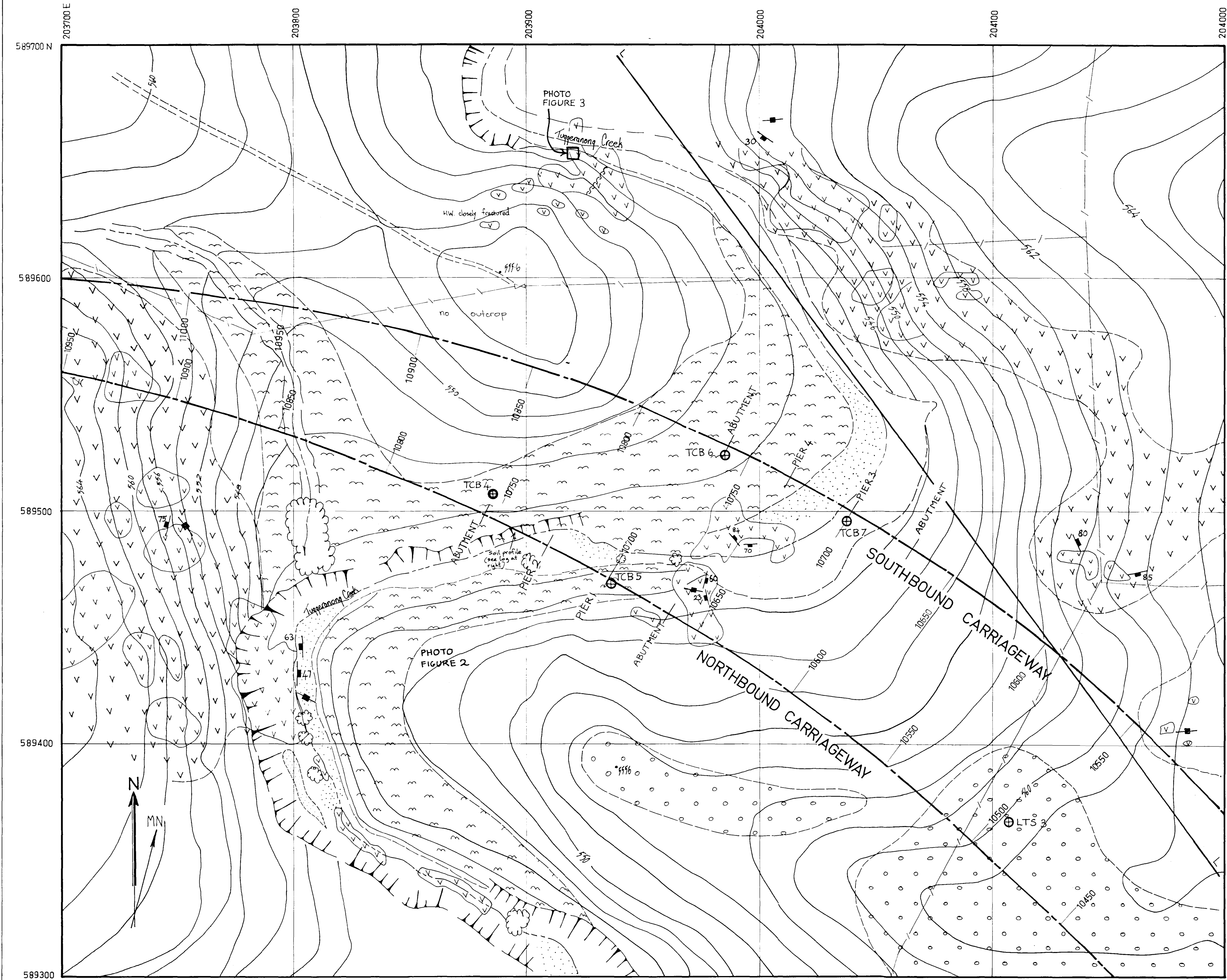
PLATE 2

MURRUMBIDGEE PARK DRIVE/LANYON TRUNK SEWER
DEEP ROADCUT SECTION (CH. 10400 - 10500 m)
SECTION OF PIPELINE ROUTE (CH. 350 - 550)
MAP OF SITE



SECTION A-B SHOWING PROPOSED WORKS





MURRUMBIDGEE PARK DRIVE

GEOLOGY OF TUGGERANONG CREEK BRIDGE SITE

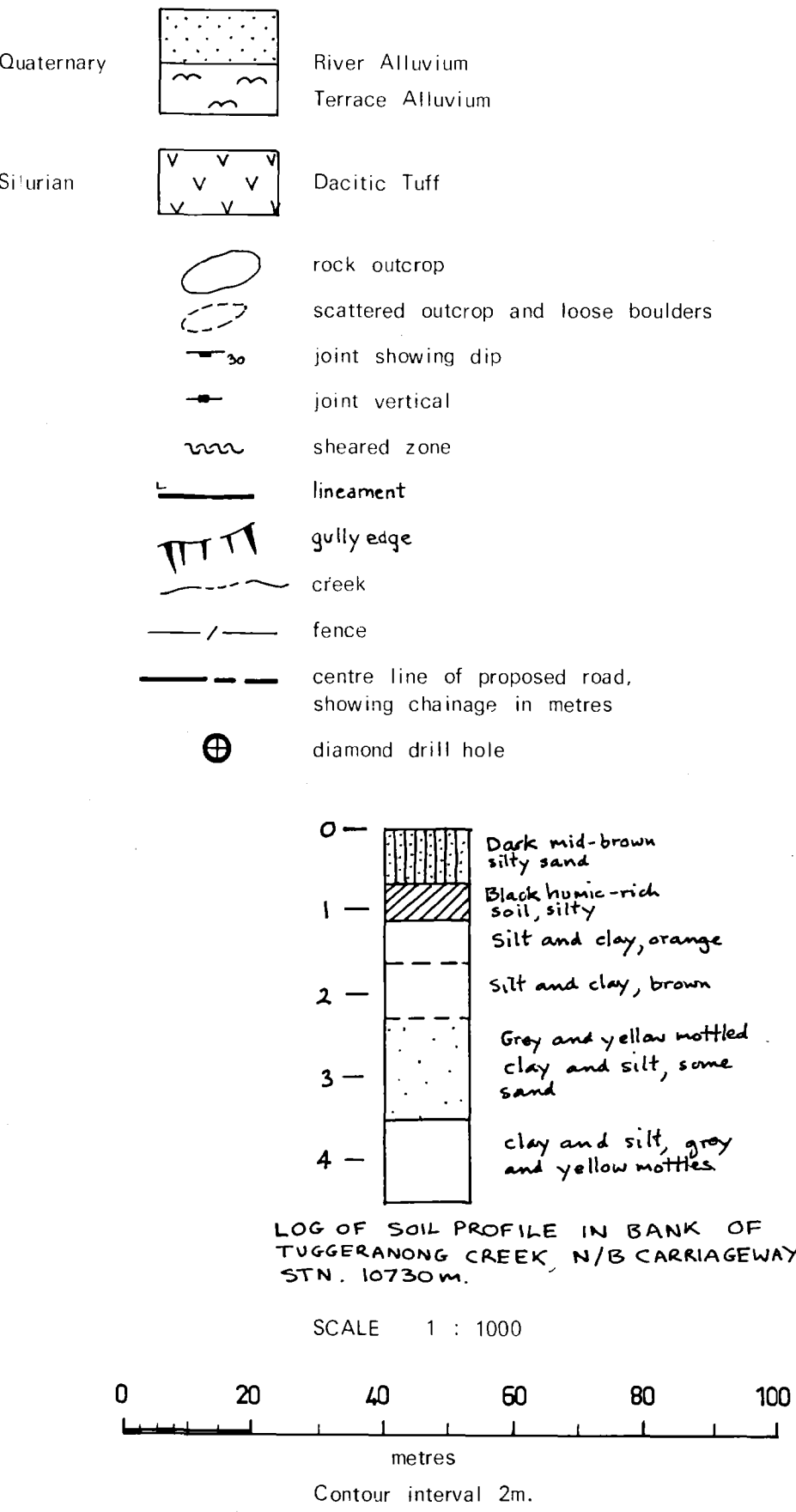


PLATE 4

AMENDMENTS				SCALE (see above)		COMMONWEALTH OF AUSTRALIA BUREAU OF MINERAL RESOURCES CANBERRA, A C T	
No.	Description	Author	Checked			TITLE	
A1				Base map/survey		GEOLOGY OF TUGGERANONG CREEK BRIDGE SITE	
A2				Geology by R. GOLDSMITH			
A3				Compiled and checked	Checked and approved	PROJECT MURRUMBIDGEE PARK DRIVE	
A4				R.C.M.G. Project geologist	D.C. PURCELL Senior geologist		
A5				E.G. WILSON Supervising geologist		To accompany Record 1976/106	Drawn by A.K. Drawing No. 155/A14/1665

MURRUMBIDGEE PARK DRIVE

GEOLOGY OF STRANGERS CREEK BRIDGE SITE

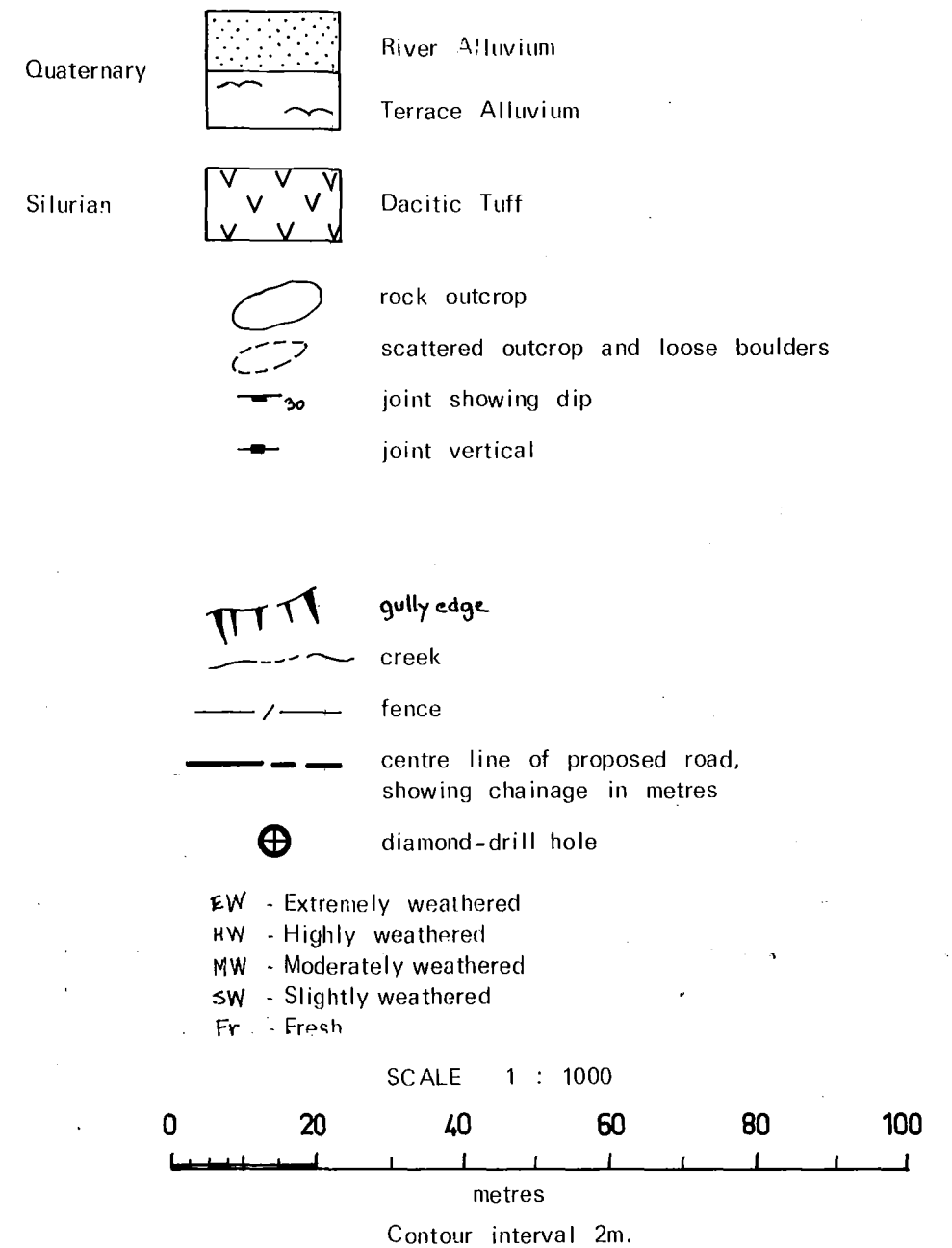
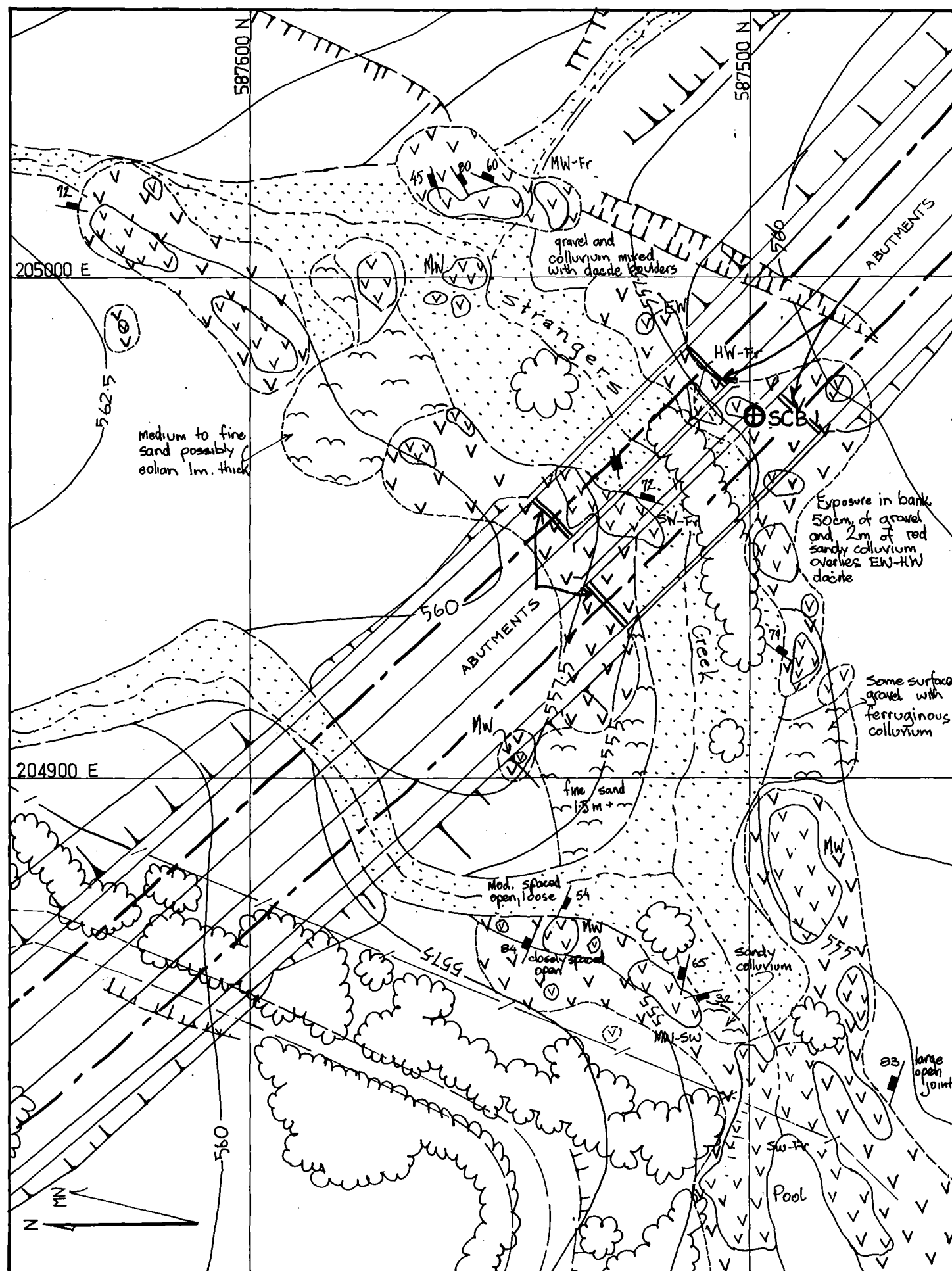


PLATE 5

