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PROGRESS REPORT ON HYDROGEOLOGICAL  
STUDIES IN THE MAGELA AND COOPER  
CREEK CATCHMENTS, ALLIGATOR RIVERS  
REGION, NT, 1979



by

J.R. Kellett, W.R. Evans, B. Jones  
and D.B. Guy

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## SUMMARY

The Bureau of Mineral Resources is undertaking an investigation of the hydrogeology and hydrochemistry of the surficial materials in the Magela and Cooper Creek catchments of the Alligator Rivers Region, NT. This study is part of a program being undertaken by the Office of the Supervising Scientist for the recognition and monitoring of environmental change which may occur as a consequence of the mining of uranium in the catchments. This report is concerned with the progress of the study in 1979; it is mainly a record of the work undertaken and of the results to hand at the end of 1979.

The main aim of the investigation is to establish a network of monitoring bores in the two catchments for the purpose of gathering baseline data on groundwater chemistry and hydraulics of the surficial cover to the bedrock. The data will be used by others as input to computer models of the two catchments.

Fifteen holes were drilled under contract in the Cooper Creek catchment, and twelve holes were drilled in the southern part of the Magela Creek catchment; all but one hole were completed with stainless steel screens and unplasticised PVC casing. Most of the bores have been developed by flushing and pumping and are ready for aquifer tests and water sampling, and the first set of water samples was collected and sent for analysis.

Drilling progress was slow and can be attributed to the poor serviceability of the drill rigs and delays in the supply of materials and equipment; only 27 of the 55 bores planned for the year were completed.

The coordinates of bores and height above sea level were determined for bores in the Magela Creek catchment only.

Infiltration tests were carried out by both the ring infiltrometer and the auger hole permeameter methods on most of the surficial lithologies.

### Cooper Creek catchment

In the Cooper Creek catchment the gentle slopes of partly dissected lateritised Proterozoic metasediments and metavolcanics have a cover of clays developed on lateritic detritus. Slopewash sands surround outliers of massive Kombolgie Formation (Carpentarian), and clays and earths in the drainage depressions are derived from alluvium; quartzose sands occupy stream channels and adjacent alluvial terraces.

Water in some bores is odorous and brackish-tasting, and a restriction of the drainage of groundwater from the catchment seems likely. As the quartz sand thicknesses in Tin Camp Creek basin decrease downstream from 9 m to 2.4 m at the junction of Tin Camp Creek and Cooper Creek, drainage would be restricted if the decreasing thickness of sand is associated with a rise in the level of the base of the sand; however, speculation should await the results of levelling in the catchment. The effect of the sewerage treatment plant on the quality of groundwater in the Tin Camp Creek basin during the dry season will be established as water quality information becomes available.

The highest yielding aquifers are the sands of the stream beds and adjacent terraces, where the thickness of sands ranges to over 9 m, with yields from bores ranging to 4.5 L/s, and a saturated hydraulic conductivity determined by infiltration ranging up to 30 mm/hr.

The red and yellow earths overlying the higher lateritised interfluvies of the catchment are highly permeable and gave infiltration capacities of up to 50 mm/hr. Lateritised bedrock gave a steady-state infiltration rate of 10 mm/hr that is attributed mainly to the transmission of water through quartzose sand infillings of irregular planes and tubular vughs in the disintegrated laterite.

Magela Creek catchment

The Tertiary landsurface in the catchment is defined by the top of the laterite which, in places, is lateritised detritus and elsewhere is lateritised Precambrian metamorphics and migmatite. Above the dissected laterite, the broad base of the valley is occupied by Quaternary channel deposits of sand and gravel; the sand and gravel has been dissected by the Magela and Gulungul Creeks and quartz sand comprises the present-day stream-channel deposits and alluvial terrace.

The thickness of Quaternary sand and gravel across Magela Creek ranges from 3 to 6 m overlying lateritised gravel to the west of the main stream channel, up to about 15 m to the east of the channel. The increased thickness to the east may be attributed to movement along the Magela Fault, after deposition of the Quaternary sands and gravel, with downthrow to the east. The thickness of Quaternary sand across Gulungul Creek does not exceed 5 m.

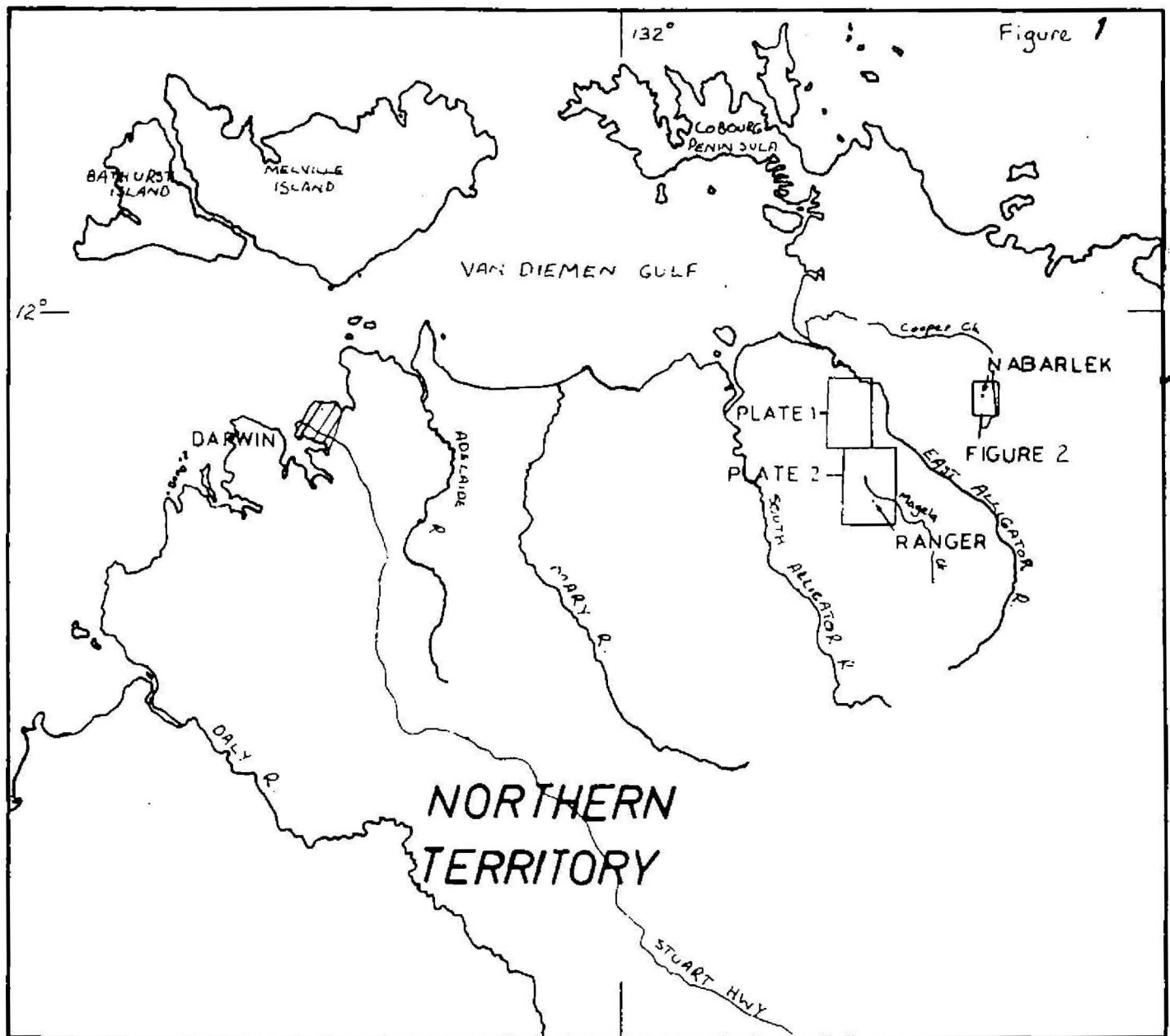
Information is not yet available on water quality from Magela Creek catchment; however, yields from bores range to 2 L/s on Gulungul Creek traverse and to 6.5 L/s on Magela No. 1 traverse. Saturated hydraulic conductivity in siliceous sands determined by infiltration tests ranged from about 14 to 30 mm/hr. Cracking clays in the lower Magela floodplain gave very high infiltration rates that were attributed to large shrinkage cracks, and these would ensure high infiltration rates early in the wet season.

Program for 1980

It is proposed to continue the study in 1980 with a large drilling program that will make up the shortfall in drilling in 1979; 72 holes are proposed for the Magela Creek catchment and six holes for the Cooper Creek catchment. Underflow studies in the stream bed of Magela Creek are planned and will be based on tests made on 18 piezometers in the stream bed sands. Four stratigraphic holes are proposed for drilling by the Bureau of Resources' drill rig in the northern Magela floodplain to link

4.

up with other stratigraphic holes associated with earlier geological studies. Aquifer testing, including water sampling and analysis, and surveying and levelling will continue in 1980, and as time permits a joint analysis will be made of structural domains of the Kombolgie Formation.



LOCALITY MAP, ALLIGATOR RIVERS REGION

## INTRODUCTION

### Background to the investigation

The Jabiru Workshop, held on 21-25 August 1978 and arranged by the Office of the Supervising Scientist (OSS), was attended by sixty-four representatives of the mining industry, government bodies, scientific disciplines and organisations, and environmental organisations, for the purpose of making recommendations for the detection and monitoring of environmental changes in the Magela and Cooper Creek catchments (Fig. 1) which may occur as a consequence of the mining of uranium in the two catchments.

Recommendations of the Groundwater Committee at the Workshop included proposals for groundwater monitoring in the vicinity of mine operations and for catchment studies and the development of models concerned with the dynamics of groundwater movement and the variables of groundwater chemistry.

A Hydrogeological Committee was convened by the OSS, and its function, as set out in a subsequent meeting, is to discuss and coordinate integrated hydrogeological programs in the Magela and Cooper Creek catchments, and to outline the contribution of the action agencies undertaking the various programs. The Committee first met on 9 April 1979 and monthly meetings were held throughout 1979; the meetings were attended by officers of OSS, and by representatives of the Water Division of the Northern Territory Department of Works and Transport (WD), the Geological Survey of the Northern Territory Department of Mines and Energy (NTGS), and the Bureau of Mineral Resources (BMR), under the chairmanship of WD officers. Representatives of the mining companies have also attended a number of meetings by invitation of the Committee.

In February 1979, the OSS requested BMR to provide hydrogeological advice and, where appropriate, to undertake hydrogeological field work.



Following approval for the mining of the uranium deposits at Jabiru and Nabarlek, a coordinated hydrogeological program was agreed upon for 1979 at the Hydrogeological Committee Meeting on 9 April 1979, and a description of the BMR contribution to the program is attached to this report as Appendix 1. It may be briefly summarised under the headings of the three projects of its contribution:

Hydrology of the alluvial aquifers,  
Surficial geology and infiltration  
capacities, and  
Joint analysis in the Kombolgie  
Formation.

Planning for the 1979 field season went ahead from this time; the main tasks were to arrange a drilling contract for investigation of the surficial geology and establishment of water bores and piezometers suitable for hydrogeological investigations, and to acquire suitable equipment for aquifer pumping tests and for infiltration studies. Estimates were prepared and submitted to the OSS. Members of the party with their equipment started moving into the field from Canberra on 29 June 1979 and the field party assembled in Darwin on 16 July and proceeded to Jabiru where they occupied accommodation arranged by the OSS. The members of the field party were:

J.R. Kellett, Party Leader (Geologist)  
W.R. Evans (Geologist)  
B. Jones (Technical Officer)  
D.B. Guy (Technical Assistant)  
D.S. Lamont (Field Hand) and  
F.J. Pritchard (Field Hand).

BMR provided salaried staff and the use of BMR vehicles and other equipment as its contribution to the project; the OSS funded the drilling contract, the operational expenses of the field party including the field hand wages, and the purchases of equipment specifically required for the project.

### Program for 1979

The most important task of the field party in 1979 was to set up a network of 40 monitoring bores in the southern Magela Creek catchment and 15 monitoring bores in the Cooper Creek catchment, for the purposes of gathering baseline data on groundwater chemistry and hydraulics of the surficial cover. This information is required to define the initial conditions in a predictive model to be developed by WD.

The establishment of the bore networks was dependent upon contract drilling which should have finished by the end of September 1979 in the initial timetable, and during the following month it was proposed to determine hydraulic conductivities and effective porosities of the alluvium by pump testing the bores, and to determine dispersion coefficients by dye tracing.

The program also included infiltration testing of each of the major soil groups of the Magela Creek catchment, particularly the soils of the flood plain.

If time permitted, it was also proposed to investigate groundwater storage and transmitting capacity of the Kombolgie Formation.

### CONTRACT DRILLING

#### Location of bores and their objectives

The proposed bores and those bores completed in 1979 in the Magela Creek catchment are shown in Plates 1 and 2, and those in the Cooper Creek catchment are shown in Figure 2.

In the Magela catchment the holes are set out in 9 traverses. Traverse 1 (Plate 2) intersects Magela Creek, its flood plain, and the higher lateritised landsurface southeast of the Ranger Project Area, and traverse 4 intersects Gulungul creek and its flood plain to the south of the Ranger Project Area. These two traverses are upstream from the Ranger Project Area and should define groundwater chemistry of the major surficial aquifers free from any possible pollution by mining at Ranger.

Traverses 2 and 3 lie to the north of the Ranger Project Area; traverse 2 is designed to provide early warning of possible groundwater contamination downstream from Ranger, and traverse 3 near Mudginberri Lagoon should contribute to the study of the dilution of polluted groundwater before reaching the permanent waterholes of the Magela Plain.

An additional part of the drilling program is to set shallow piezometers into the Magela creekbed so that the dynamics and groundwater chemistry of bed underflow during the dry season can be studied. Five shallow piezometers are planned for the Magela creekbed at its intersection with traverse 1, seven holes are planned for traverse 2, and six holes for traverse 3.

Traverse 5 across Hades Flat is designed to determine the characteristics of subsurface flow fed by the 7-J Creek catchment before mixing with waters of the Magela system.

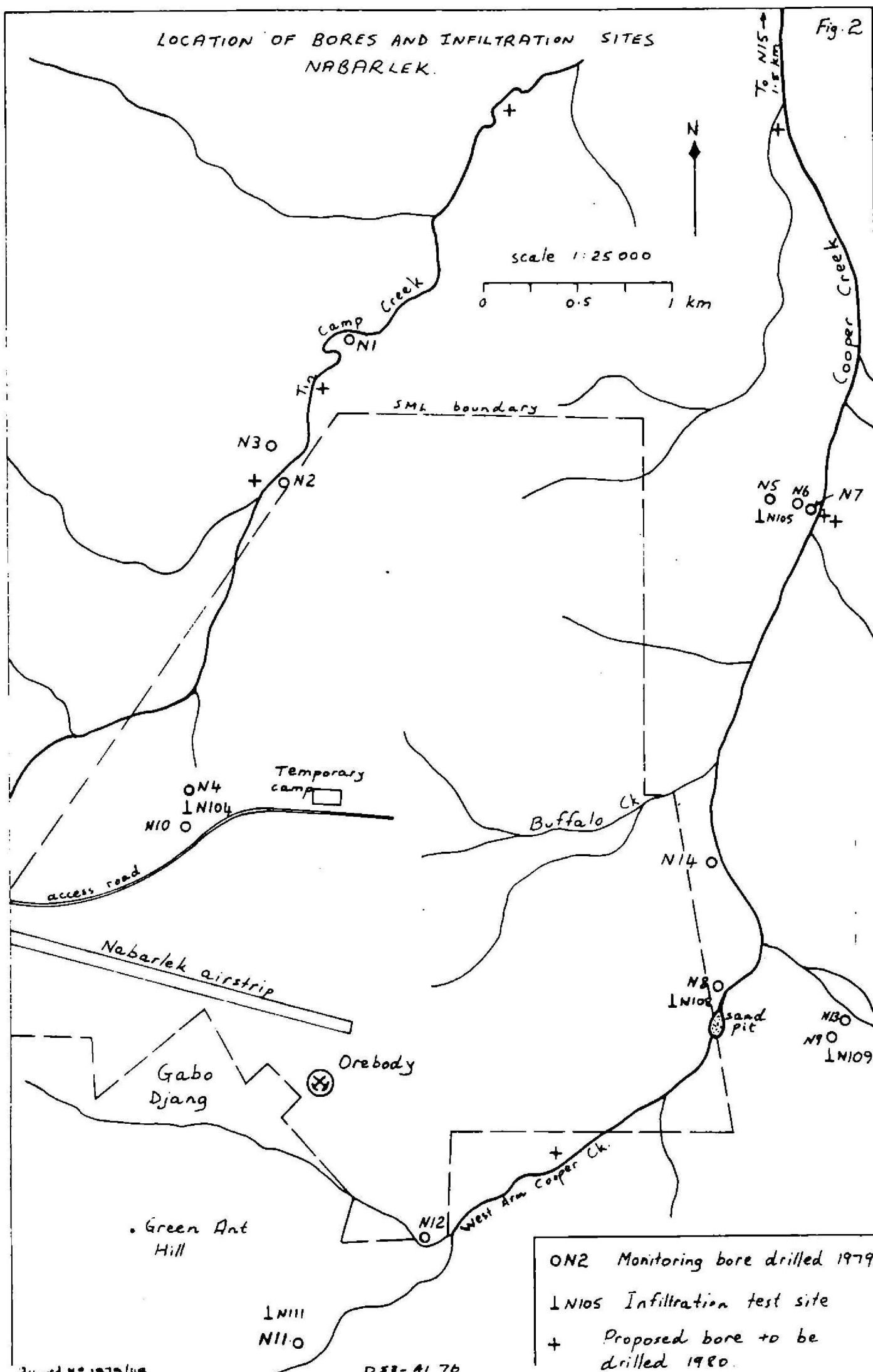
Traverse 6 near Ja Ja Billabong and other drillholes in the Jabiluka area, including those along the Oenpelli Road, will provide information on groundwater chemistry after exchange with the surface waters of the billabongs and will also give baseline data in the event of future mining activities in that area.

Traverses 7, 8, and 9 span the Magela Plain to the north of Jabiluka; they will monitor groundwater upstream of the wetlands that lie to the north, and should pollutants be identified, they will monitor the effect of remedial work that might be undertaken.

The Nabarlek holes in the Cooper Creek catchment (Fig. 2) are designed to ascertain baseline groundwater chemistry of the principal surficial aquifer in the area, namely the alluvium and terrace soils of Cooper Creek and Tin Camp Creek. From these holes it will be possible to calculate the stream underflow discharges during the dry season, which is the time when groundwater pollution would be most likely to occur. Possible pollutants would be detected well before they reach wet areas near the confluence of the two streams.

# LOCATION OF BORES AND INFILTRATION SITES NABARLEK.

Fig. 2



Borelogs of the holes put down in 1979 are shown in Appendix 2.

#### Calling of tenders

The OSS requested that BMR arrange a contract for drilling, and be responsible for the calling of tenders, awarding of the contract, and supervision of the drilling on behalf of the OSS. The Superintendent of Stores for the Department of National Development was requested on 5 June 1979 to arrange for the calling of tenders. The Purchasing Division of the Department of Administrative Services called tenders on 25 June, and tenders closed on 19 July. The contract was awarded on 29 August 1979 to F.A. Kelly Pty Ltd of Broadmeadow, NSW.

#### Drilling operations

Drilling commenced in the Magela catchment on 10 September with one rig. On 15 September when only one hole had been completed and another was part drilled, it was decided that the rigs should move to the Cooper catchment and remain there until all holes in that catchment were completed.

Drilling in the Cooper catchment commenced on 20 September with one rig; a second rig commenced drilling on 23 September. The 15 holes planned for the catchment were completed on 6 October, and both rigs returned to the Magela catchment.

One rig recommenced drilling hole M2 in the Magela catchment on 10 October, but the second rig did not become operational until 18 October.

The date for completion of the contract had initially been set at 19 October, later than originally planned because of the delay in the initial calling of tenders; however, because of the slow drilling progress, it was decided to continue drilling beyond that date. Drilling ceased on 26 October after the completion of only 12 of the 20 holes that were considered the minimum requirement in the Magela catchment for the 1979 field season.

Over the seven week period of the drilling contract, 42 working days, only 41 operational rig-days out of a possible 84 rig-days for the two rigs was achieved.

The members of the field party assisted the contractor beyond the normal limit of responsibility for supervision of the contract in an endeavour to speed up the drilling rate, generally in the tasks associated with the completion and development of the bores; the removal of mudcake in particular proved time-consuming because the contractor was obliged to use a highly viscous mudcake owing to his inability to drive casing with the rigs assigned to the project. A rig with a top drive is essential for this work and will be specified in future contracts of this nature.

Delays encountered during the drilling and completion of the bores can be attributed to the poor serviceability of the drilling plant and equipment, the lack of spare parts, and failure to provide reasonable quantities of screens and other materials at the drill sites when required. Despite the problems encountered, it should be stated that the relations between the members of the BMR field party and the drill crews were friendly and cooperative throughout the project.

#### Other factors contributing to the shortfall in drilling

Delay in the calling of tenders until 5 June, and a further delay in the final awarding of the contract, were due to the budgetary constraints on the OSS, and this probably was the main reason for attracting so few contractors to tender for the work. By the time tenders were called, those contractors who were prepared to operate in the area already had signed contracts for the season, and the limited time available for drilling was a further restriction.

Because of the late start and limited funding for the contract, the original requirement of 125 holes was reduced to 56 holes; a further reduction to 35 holes was made when it became clear that the required amount of drilling could not be carried out during the field season under this contract, and the holes planned for the lower Magela catchment were deferred until 1980.

### Drilling contract for 1980

It is proposed that tenders for a contract for drilling in the Magela and Cooper Creek catchments during the 1980 field season be called in early February 1980. The contract should be awarded by the end of March, and drilling should commence during the last week of May.

The amount of drilling that is recommended for 1980 has been set out in the section "Program for 1980" at the end of this report.

### SURFICIAL GEOLOGY

#### General

The oldest rocks of the area are a series of Archaean-Lower Proterozoic sedimentary formations deposited over 2000 million years ago. These sedimentary rocks were metamorphosed about 1800 million years ago to produce a variety of schistose, gneissic, and granitic rocks. The gneiss and granitic rocks have been mapped as the Nanambu migmatite complex in much of the Magela catchment, migmatite being rock formed by extreme metamorphism involving partial or complete melting of the original rock. A later sequence of Lower Proterozoic sediments was then deposited and these were also metamorphosed, the high-grade rocks being mapped as the Nimbuwah migmatite complex that is present in much of the Cooper Creek catchment.

Between 1800 and 1400 million years ago these formations were covered by Carpentarian deposits - the Kombolgie Formation which is predominantly sandstone with some interbedded volcanic members.

About 100 million years ago, sand and silt covered large parts of the area during the Cretaceous period, giving rise to the siltstone and sandstone of the Bathurst Island Formation.

The regional geomorphology and Cainozoic geology to the west of the East Alligator River have been described by Storey et al. (1969); the Cobourg Peninsula to the north has been investigated



by Hughes & Senior (1973) and Hughes (1978). These workers established that Miocene epeirogenic uplift and gentle tilting about an east-west axis close to the present coastline stripped the Cretaceous sediments of the Bathurst Island Formation from the area, and exhumed Carpentarian and Lower Proterozoic rocks. The closest known sediments of the Bathurst Island Formation are in the northern end of the Magela flood plain (Needham, 1976).

Extensive lateritisation occurred on the exposed surfaces of the upper Cooper Creek and Magela Creek catchments during a quiescent period in the late Miocene and Pliocene. Falling sea levels during the Pleistocene induced rejuvenation of Cooper Creek and Magela Creek, and much of the Tertiary laterite surface was removed. Several smaller scale climatically controlled erosional-depositional phases with minor movement along faults occurred intermittently throughout the Quaternary.

#### Nabarlek area

The surficial geology of the Nabarlek area is shown in Figure 3. Distribution of soil types is based on airphoto interpretation, on the logs of 15 shallow drill holes by BMR, and on drillers' logs from Australian Groundwater Consultants Pty Ltd (AGC). The Cooper catchment is currently being mapped by the Northern Territory Land Conservation Unit, and publication of their soils map at 1:25 000 scale is imminent (Wells, 1979).

Remnants of the Tertiary lateritised surface around Nabarlek are preserved in small footslopes around outliers of Kombolgie Formation (Fig. 3). The deposit above the northwest arm of Buffalo Creek is delineated on the basis of AGC drillers' logs which indicate 6 to 9 m of sand indurated by sesquioxide, overlying a strongly mottled clay and pallid zone. Lateritic remnants should also occur beneath footslopes of Kombolgie Formation to the west of Tin Camp Creek because of the similar geomorphic setting.

The oldest Quaternary soils are massive clays and earths developed on reworked laterite on the southern part of the interfluvium between Cooper Creek and Tin Camp Creek.





Quartzose alluvium



Slopewash sands



Clays and earths developed on immature alluvium



Polygenetic soils - layered clays developed on lateritic detritus



Laterites overlain by rock rubble



Rock outcrop



Figure 4. Disintegrated laterite showing sand infills in  
vughs. Location: Cutoff trench in Ranger No. 1  
Retention Pond. Negative No. M/2373/18

The next youngest unit is a solodic soil which occurs on the highest terrace of Cooper Creek and Buffalo Creek. Near hole N12, on the west arm of Cooper Creek, the clay is yellow-brown and blocky with abundant carbonate nodules in the B2 horizon; the hydromorphic equivalent is a dark grey cracking clay which is exposed in the eastern bank of the Cooper Creek sand pit. The Buffalo Creek clay is more strongly structured, which reflects its predominantly dolerite provenance.

Weakly indurated sheetwash and alluvial sands are either contemporaneous with or younger than the solodic soils described above. These soils were derived mainly from reworking of Kombolgie Sandstone and consist of poorly sorted quartz grains cemented together by clay. The plasma colour grades from pale grey or white in the southern part of the area to pronounced yellow and grey mottling towards the confluence of Tin Camp Creek and Cooper Creek.

The youngest soil is the uncemented quartzose alluvium in low terraces and beds of all major streams. Median grain size is about 0.3 mm and sorting ranges from moderate to poor.

#### Magela Creek catchment

The top metre of the surficial cover in the catchment has been mapped at 1:25 000 scale by the Northern Territory Land Conservation Unit.

At the Ranger mine site, tailings dam, and retention pond No. 1 a yellow earth up to 2 m thick overlies truncated lateritised alluvium/colluvium on bedrock. The Tertiary landsurface is defined by the top of the lateritised zone, whether it is lateritised detritus or lateritised bedrock. The laterite has disintegrated, with quartzose sand infilling irregular planes and tubular vughs (Fig. 4).

Traverse 1 (Plate 2) extends across the laterite pavement to the Magela flood plain, and a cross-section is shown in Figure 5. Holes M1, M2, and M3 intersected a buried lateritised

surface developed on immature gravel and sand, which may represent slopewash and fluviatile facies of sediments of the Bathurst Island Formation.

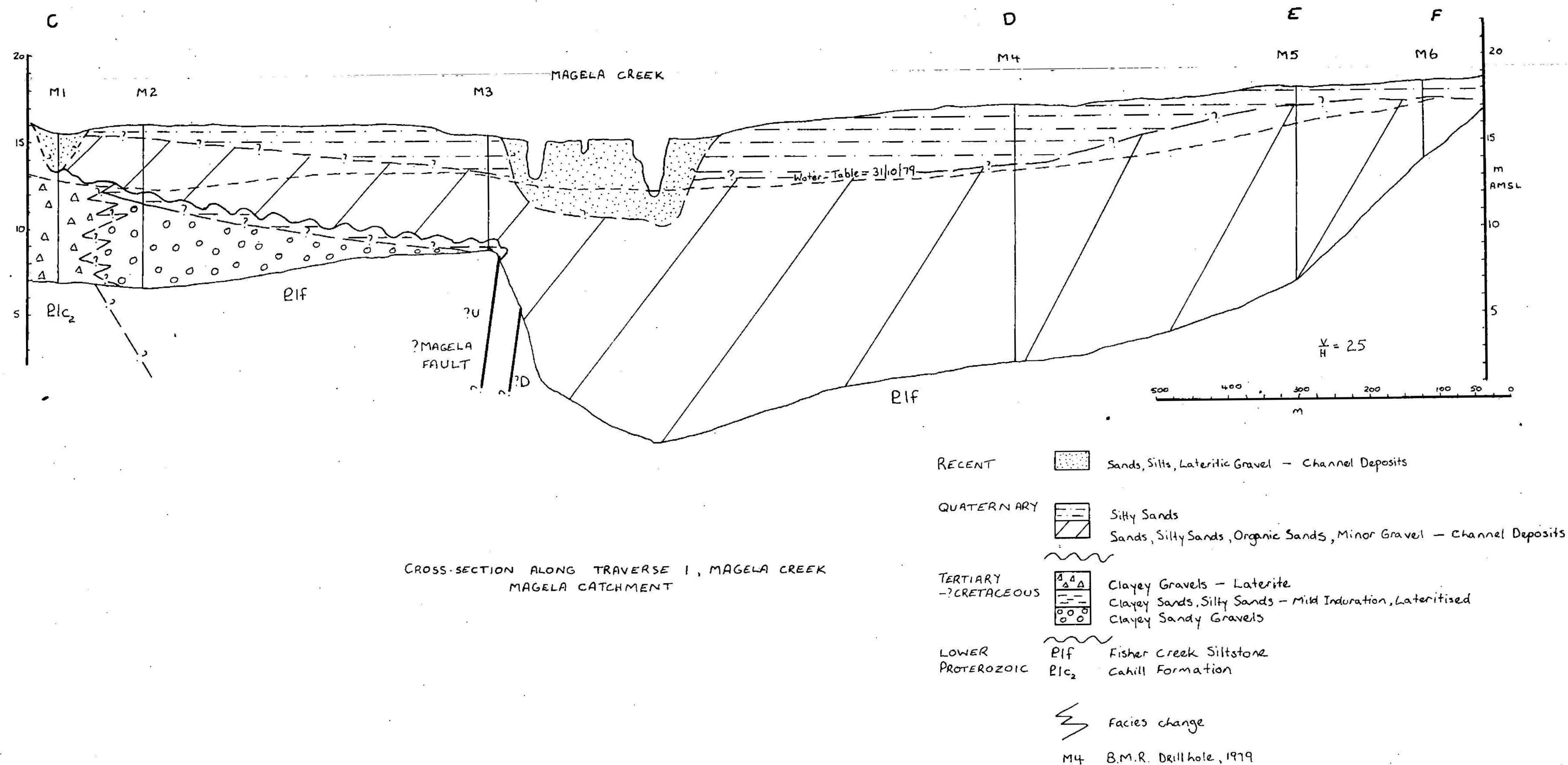
In the southern part of the catchment, the course of Magela Creek is controlled by the Magela Fault, which appears to have undergone Cainozoic movement. Upthrown sediments to the southwest of the fault were bevelled and lateritised; downthrown sediments to the northeast of the fault were extensively channelled, particularly near the fault alignment. Holes M4, M5, or M6 did not recover any remnants of lateritised material and are considered to be on the downthrown side of the fault; however, the core losses in these holes were such that lateritised material could have been present. An extensive lateritic pavement is known to occur to the northeast of hole M6. A buried fault scarp is inferred further downstream on Magela Creek where Ranger bores bottomed on bedrock at 2 m on the upthrown side and at 60 m on the downthrown side (Sarmed, pers. comm.).

Quaternary deposits of channel sands and overbank deposits are up to 14 m thick at Traverse 1 and are known to attain thicknesses of 60 to 70 m on the Magela flood plain south of Mudginberri.

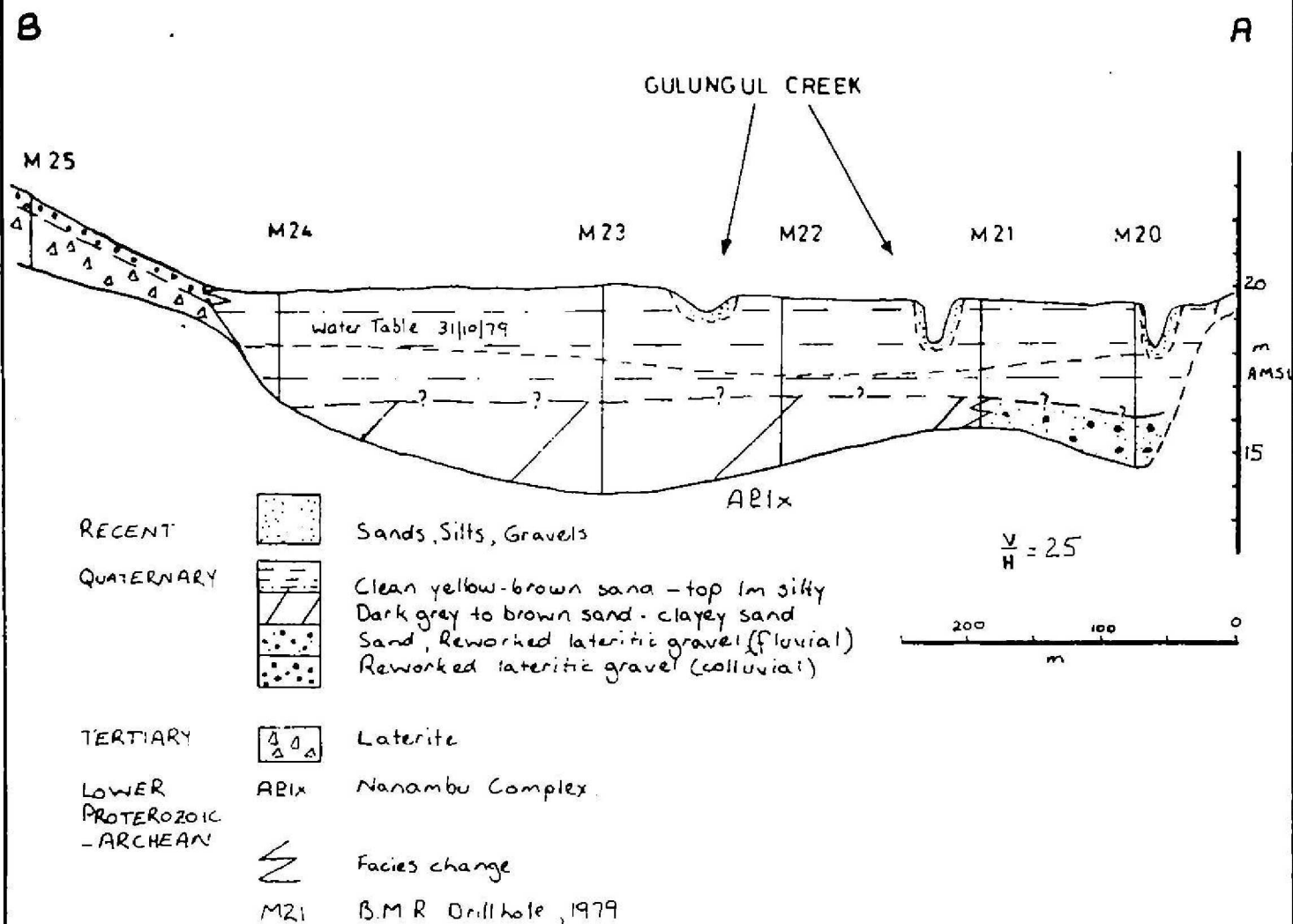
A silty sand layer with minimal pedogenetic organisation blankets the whole flood plain. This layer is incised and filled with alluvium of the present Magela Creek and abandoned channel deposits (refer to hole M1).

Traverse 4 (Fig. 6) indicates that Gulungul Creek is similarly superimposed onto a buried early Quaternary braided stream system with two discrete palaeo-channels, one filled with fine silty sand and the other containing fine sand and lateritic gravel; both are covered by a similar silty sand layer with minimal pedogenetic organisation.

Figure 5



CROSS-SECTION ALONG TRAVERSE 1, MAGELA CREEK  
MAGELA CATCHMENT



CROSS-SECTION ALONG TRAVERSE 4, GULUNGUL CREEK AREA  
MAGELA CATCHMENT



HYDROGEOLOGY OF THE SURFICIAL COVERNabarlek area

Casing schedules of the fifteen boreholes are included in Appendix 3. Boreholes N2, N3, N6, N7, N9, N11, and N13 were developed over the two weeks following drilling and are considered to be suitable for water quality monitoring. All other holes were either dry or of low seepage and therefore it was not possible to properly flush the aquifers, but the gravel pack was cleaned by forcing water through the screens and up through the gravels.

Table 1 shows steady yields of the bores, ascertained after 2 to 3 hours' pumping during the final bore development program.

The Recent alluvial terraces of Tin Camp Creek and Cooper Creek form the highest-yielding superficial aquifer. Chemical analyses of groundwater from the Nabarlek bores are not yet available, but water from bores N11 and N12 (Cooper Creek) was odorous and foul-tasting. This is attributed to the very low seepage rates through the sub-sola in this area. Groundwater from N2 and N3 (Tin Camp Creek) was slightly odorous and tasted brackish.

Sand thicknesses in Tin Camp Creek decrease from over 9 m at N3 to 5.5 m at N1, which is 700 m downstream, and at the junction with Cooper Creek the thickness of sand is only 2.4 m. If the reduction in thickness of sand downstream is due to a rise in the level of the base of the aquifer, groundwater will be ponded in Tin Camp basin sediments, and groundwater movement in the latter part of the dry season is likely to be very slow. In prolonged dry seasons, it is reasonable to expect groundwater movement to cease. Such stagnation points should be predicted by the model being developed by the Northern Territory Water Division. The Tin Camp Creek area is sensitive because it lies downslope from the Nabarlek sewerage effluent pond. Seepage from the pond into the sands would be maximised during the dry season because this is the time of the maximum potential difference in

TABLE 1STATUS OF NABARLEK BORES

Bore No	Interval screened (m)	Aquifer	Yield	Status
N1	1.5 - 5.5	Medium sand	Seepage	Flushing required before chemical sampling
N2*	2.9 - 7.0	Sand, gravel	4.5 L/s	Passed for sampling
N3*	3.7 - 9.3	Sand, gravel	1.5 L/s	Passed for sampling
N4	2.4 - 5.0	Lateritic gravel	Seepage	Further flushing required
N5	0.9 - 2.9	Slopewash	Dry	Flushing required
N6	1.7 - 3.7	Fine sand	2 L/s	Passed for sampling
N7	1.1 - 3.1	Medium sand	1 L/s	Passed for sampling
N8	1.0 - 3.0	Clayey gravel	Dry	Flushing required
N9	5.8 - 9.8	Weathered dolerite	0.5 L/s	Passed for sampling <sup>x</sup>
N10	3.5 - 5.5	Lateritic gravel	Seepage	Flushing required
N11**	2.1 - 6.1	Lateritic gravel	0.05 L/s	Passed for sampling
N12**	2.0 - 4.0	Lateritic gravel	Dry	Flushing required
N13	0.9 - 4.9	Gravel	Less than 0.05 L/s	Passed for sampling
N14	0.0 - 2.0	Medium sand	Dry	Flushing required
N15	0.5 - 2.5	Fine sand	Seepage	Flushing required

x Brass footvalve wedged on welding slag at join of screens at 7.8 m

\* Slightly odorous, brackish

\*\* Strongly odorous, brackish



water-levels between the pond and groundwater, and pollutants would be concentrated in the alluvium. Pollutants would be diluted with aquifer recharge and flushing during the following wet season.

Although yields in the alluvium of Cooper Creek are relatively high, groundwater movements during the dry season are very slow because of the downstream drainage constriction and the very low hydraulic gradient. Assuming a hydraulic conductivity of about 10 m per day, it is estimated that the subsurface discharge across Cooper Creek at holes N6 and N7 was no greater than 20 m<sup>3</sup>/day during September-October 1979.

#### Magela Creek catchment

Casing schedules of eleven boreholes are included in Appendix 3. Boreholes M3, M4, M20, and M22 are considered to be developed sufficiently for regular water quality monitoring. Boreholes M1, M6, and M23 require a small amount of flushing before they are suitable for sampling. Boreholes M2, M5, M21, and M24 still require further development.

Steady yields of the bores, ascertained after 2 to 3 hours' pumping during final bore development, are shown in Table 2.

Boreholes M1 to M6 were drilled on traverse 1 (Plate 2) and M21-M25 were drilled on traverse 4 across Gulungul Creek (Plate 2).

Along traverse 1 the highest yielding surficial aquifers are in the Quaternary alluvium of the Magela flood plain. Yields greater than 6.5 L/s were sustained in M4 on the flood plain 600 m northeast of the present Magela streambed. Lateritised gravels on the southwest side of Magela Creek are low producers.

At Gulungul Creek a yield of 2 L/s was obtained from Quaternary sand and reworked lateritic gravel near the present eastern arm of the creek. No water was found on the broad lateritised plain to the west, but laterite elsewhere at Ranger is known to contain significant amounts of groundwater. During construction

TABLE 2

STATUS OF MAGELA BORES

Bore No	Interval screened (m)	Aquifer	Yield	Status
M1	6.9 - 8.9	Lateritic gravel	Seepage	Requires minor flushing
M2	1.4 - 9.4	Sand and gravel	Seepage	Requires major development
M3	2.1 - 6.5	Medium to coarse sand	3 L/s	Passed for sampling
M4	3.0 - 15.0	Medium to fine sand	More than 6.5 L/s	Passed for sampling
M5	1.5 - 11.5	Fine sand with basal gravel	Seepage	Requires major development
M6	2.5 - 4.5	Fine sand with basal gravel	Seepage	Requires minor flushing
M20	2.4 - 5.0	Sand, lateritic gravel	2 L/s	Passed for sampling
M21	1.8 - 3.0	Sand, lateritic gravel	Seepage	Requires major development
M22	4.1 - 6.5	Fine sand	0.5 L/s	Passed for sampling
M23	2.6 - 6.6	Fine sand	Seepage	Requires minor flushing
M24	1.3 - 3.3	Coarse sand with basal gravel	Seepage	Requires major development
M25	Not screened	Lateritic gravel	Dry	Not assessed

of the cut-off trench for Ranger No. 1 retention pond, the laterite exposed in the trench walls took at least a week to drain. All the groundwater was contained within the sand infilling of vughs.

Dry season groundwater discharge through the Magela and Gulungul flood plains cannot yet be estimated, but the results given above indicate that the flood-plain sediments constitute a major aquifer of the hydrological system, with the greatest flow subparallel to the present streams.

#### Water quality sampling

A groundwater quality sampling run was done on the fully developed bores in late October 1979. Bores M3, M4, M20, M22 (Magela) and N2, N3, N6, N7, N9, and N11 (Nabarlek) were pumped for a minimum of 20 minutes and three samples from each bore were sent to AMDEL for chemical analysis. Borehole N13 was not sampled because it was dry. No results have been received at the time of writing this report.

Samples were contained in acid washed plastic bottles which had been thoroughly rinsed at each bore. A list of elements requested for analysis is given in Table 3.

TABLE 3  
WATER QUALITY SAMPLING, OCTOBER 1979

Container	Sample pre-treatment	Parameters & elements requested for analysis
1 litre plastic bottle	Acid washed bottle	pH, elect. conductivity. Ca, Mg, Na, K, Cl, SO <sub>4</sub> , HCO <sub>3</sub> , NO <sub>3</sub> .
1 litre plastic bottle	Acid washed bottle	Al, Cd, Cu, Fe, Mn, Pb, Zn, Hg, As, P <sub>2</sub> O <sub>5</sub> , SiO <sub>2</sub> .
1 litre plastic bottle	Acid washed bottle	U, Rn, Ra.

As a result of discussions with AMDEL, new methods have been adopted for future sampling. Sampling for cations and anions remain unchanged. The sample for heavy metals is to be acidified and filtered. The sample for radon should only occupy

375 mL in a 500 mL glass bottle and be unacidified and unfiltered. The modified sampling procedures are set out in Table 4.

The next sampling run is set down for January 1980, with another run to follow during the initial groundwater recession about May 1980.

TABLE 4

FUTURE WATER QUALITY SAMPLING

Container	Sample pre-treatment	Parameters and elements requested for analysis
1 litre plastic bottle	Acid washed bottle	pH, elect. conductivity. Ca, Mg, Na, K, Cl, $\text{SO}_4$ , $\text{HCO}_3$ , $\text{NO}_3$ , $\text{SiO}_2$ , $\text{P}_{205}$ .
1 litre plastic bottle	Acid washed bottle Filtered and acidified with 5 mL 1:1 $\text{HNO}_3$	Al, Cd, Cu, Fe, Mn, Pb, Zn, Hg, As, U, Ra.
500 mL glass bottle with glass stopper	Acid washed bottle Fill only to 375 mL	Rn

INFILTRATIONGeneral

Infiltration tests were run at 13 sites in the Magela Creek catchment (Plates 1 and 2) and at five sites at Nabarlek. The Magela sites were chosen as representative of soil groups in the catchment mapped by the Northern Territory Land Conservation Unit. The Nabarlek sites were located next to BMR boreholes (Fig. 2).

Two methods of constant-head (ponding) infiltration testing were employed: the ring infiltrometer method and the auger hole permeameter method. The apparatus used was based on equipment designed by Dr T. Talsma, CSIRO Division of Forest Research.

### Ring infiltrometer

The equipment consists of a ring, a constant-head permeameter, and a stand. The rings are constructed of mild steel with a diameter of 304 mm and a height of 150 mm. There is a reinforcing ring on the upper edge and a bevelled edge to the other end.

The permeameter is used to monitor the flow of water required to maintain a constant head on the sample. The permeameter consists of two concentric perspex tubes, a perforated base plate, and a scale for measuring the head of water. The stand is of either wood or metal construction, and is covered by a permeable gauze to allow free flow of water out of the bottom of the sample.

Soil samples are obtained by digging a pit down to the top of the sample, and the exposed soil surface is carefully levelled with a blade. The ring is then driven into the soil until the upper edge is horizontal and approximately 20 mm above the sample surface. The ring containing the sample is excavated and the bottom of the sample is trimmed. The ring and sample are then placed on the stand and the permeameter is filled and placed on the ring. Testing is continued until the infiltration rate reaches steady state, which is generally attained within six hours of starting the test.

Tests were carried out for several different soil antecedent moisture contents (AMC), and up to four comparison runs were done simultaneously.

### Auger hole permeameter

In this method, water is infiltrated from a constant-head permeameter into an auger hole of slightly greater diameter. The apparatus consists of a constant-head permeameter and a set of adjustable legs; equipment and procedures are described by Talsma (1980).

The constant-head permeameter is the same type as that used with the ring infiltrometer, except that the base plate is replaced by adjustable legs, which are set so that the base of the concentric tubes is below the top of the desired soil horizon. The tubes are then filled with water and infiltration continues until steady state conditions are attained, usually within one to two hours in a borehole of 50 mm diameter.

### Results

Saturated hydraulic conductivities of soils from Nabarlek and the Magela Creek catchment are shown in Table 5.

The  $k_{SAT}$  values were computed from the recession curves of the highest AMC values. Ring tests showing the dependence of infiltration rate on AMC are shown in Figures 7 to 10. It was not possible to determine AMC values in the auger hole tests because the soils required wetting before an auger hole could be dug.

In the ring infiltrometer method, evaporation losses from the water surface inside the ring were measured at an average of 0.012 mm/hr for the duration of the tests, and the figures have been adjusted by that amount.

In the auger hole method, difficulties were experienced in the sands and earths with slumping from the walls of the hole. When this happened, the test was aborted because the boundary conditions had changed. This problem could be overcome in the future by the installation of a fine screen in the section of the auger hole being infiltrated.

Table 5 shows very high infiltration capacities of around 20 mm/hr in the sands and earths of the upper Magela flood plain, indicating that groundwater recharge from rainfall will be rapid. The red and yellow earths which cover the higher lateritised interfluvies of the upper catchment also gave infiltration capacities of up to 50 mm/hr indicating that significant interflow will occur on top of the laterite during the wet season (Mill).

FIGURE 7

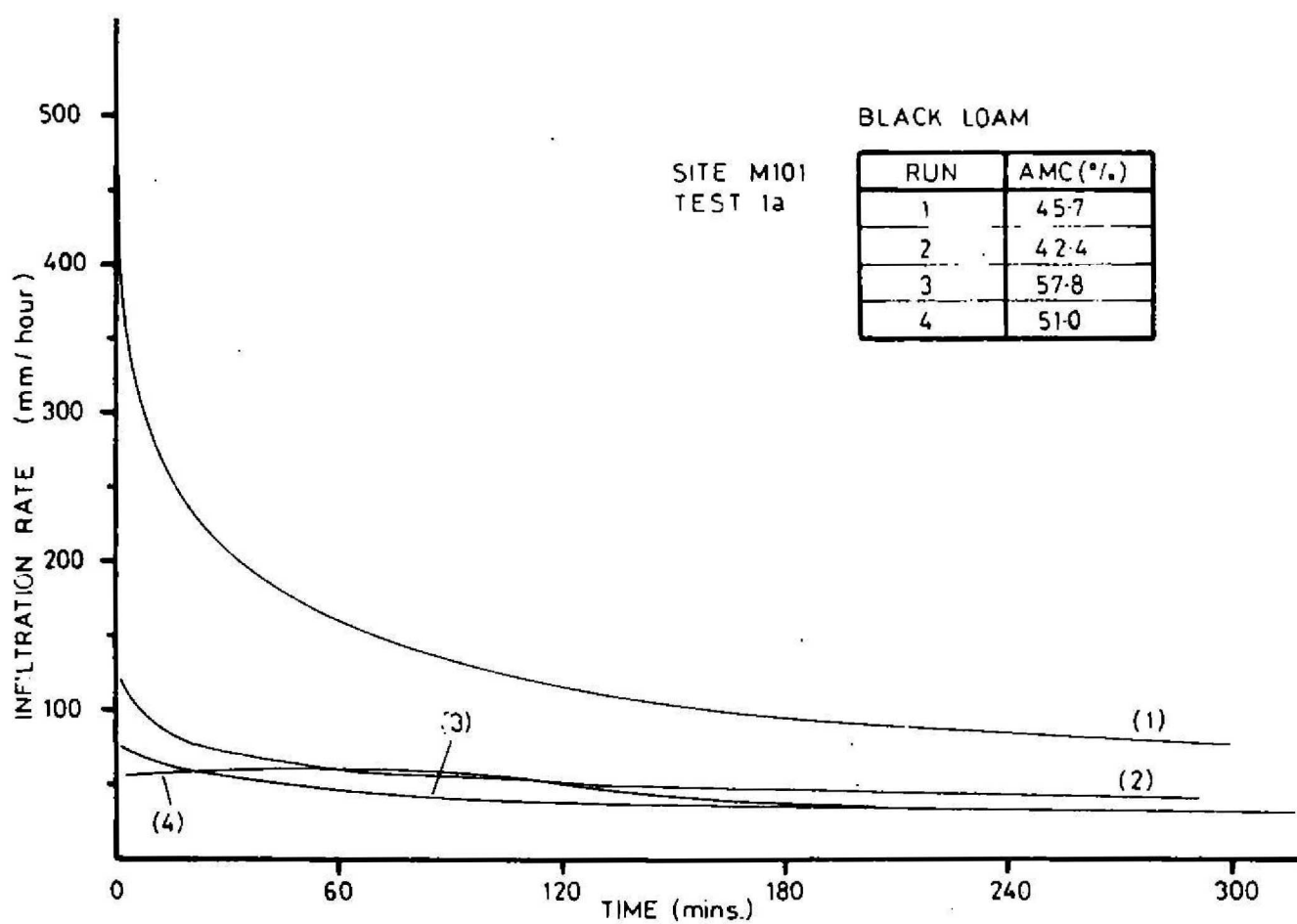
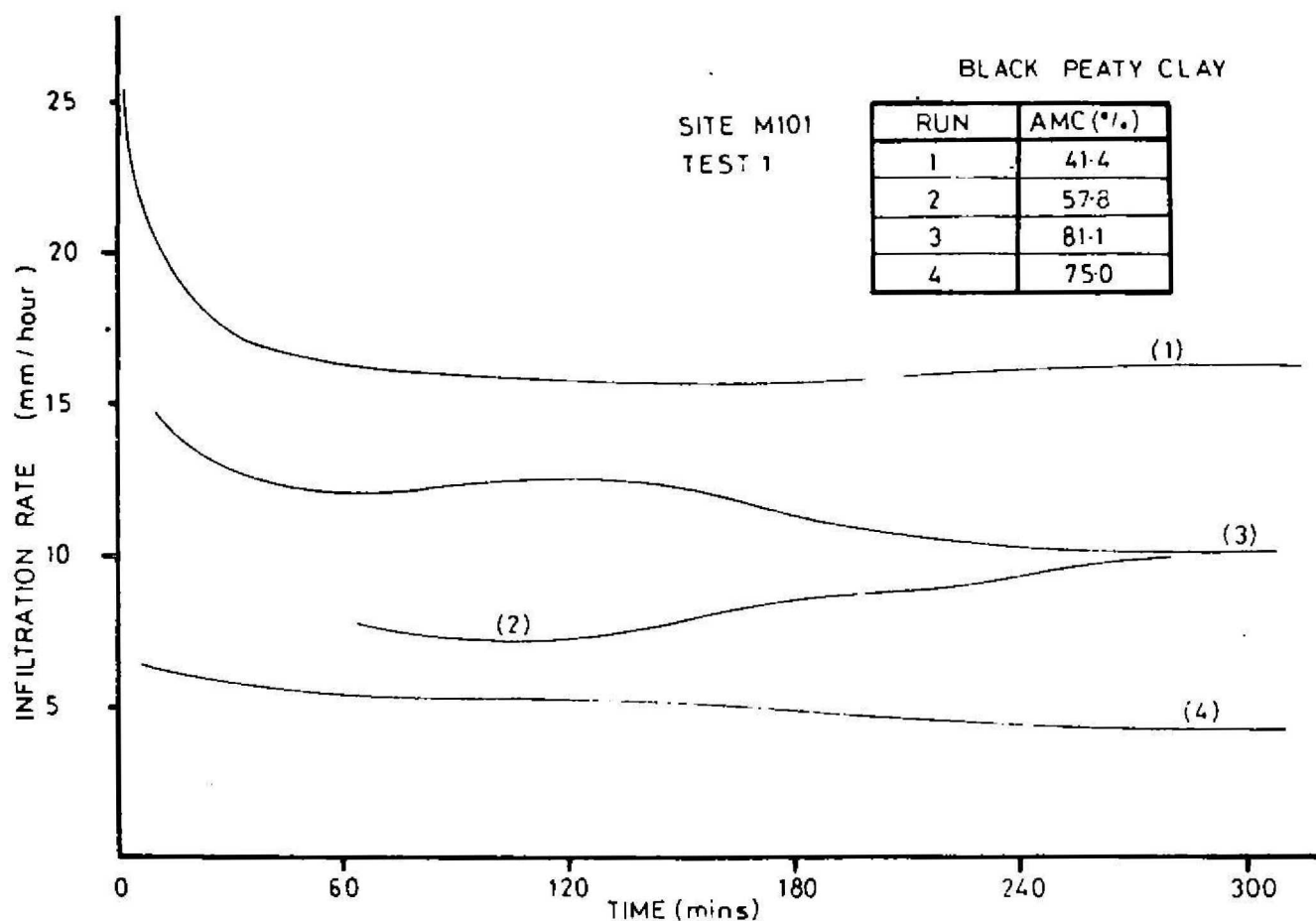


FIGURE 8

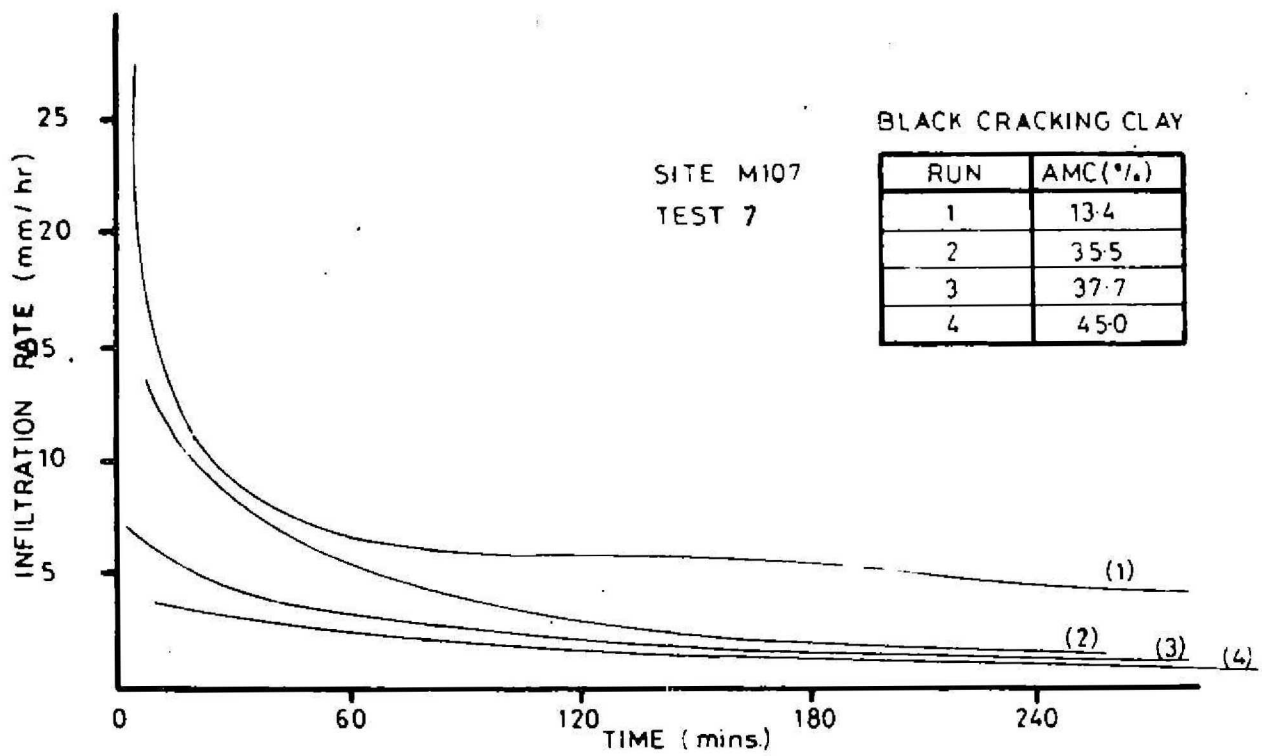
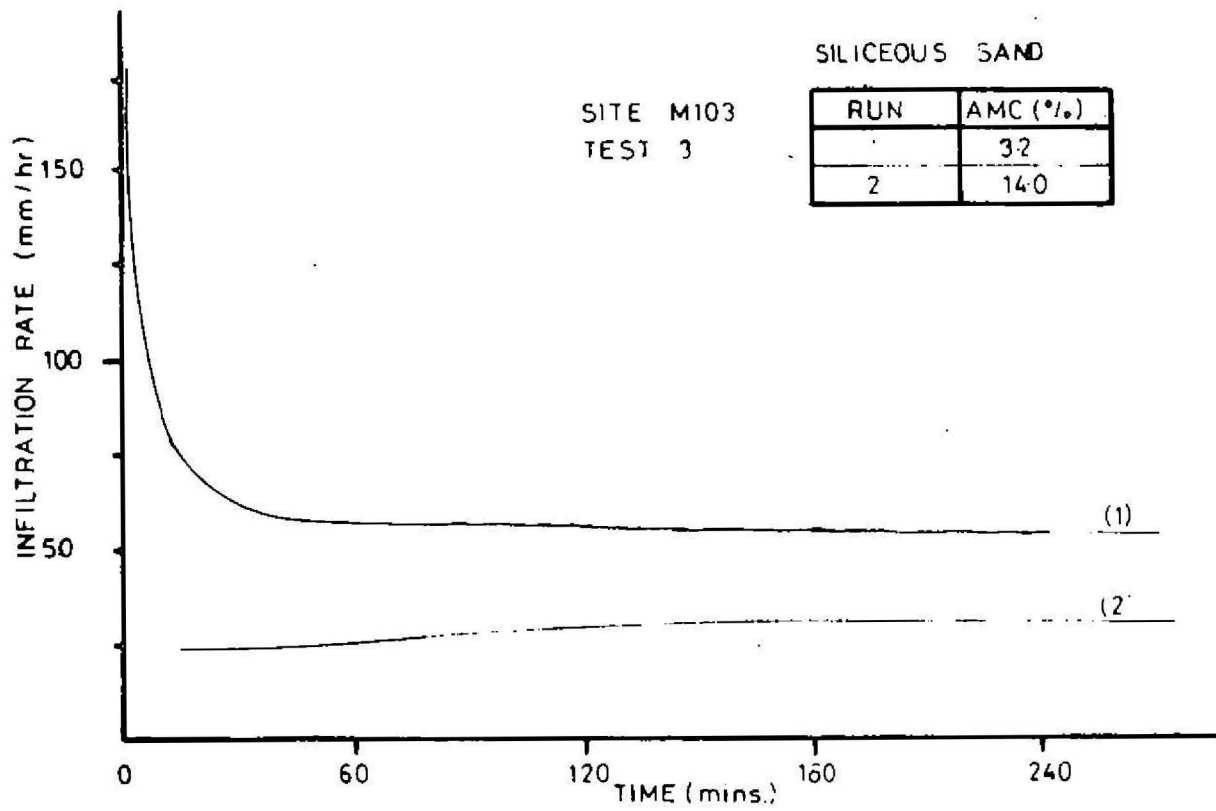


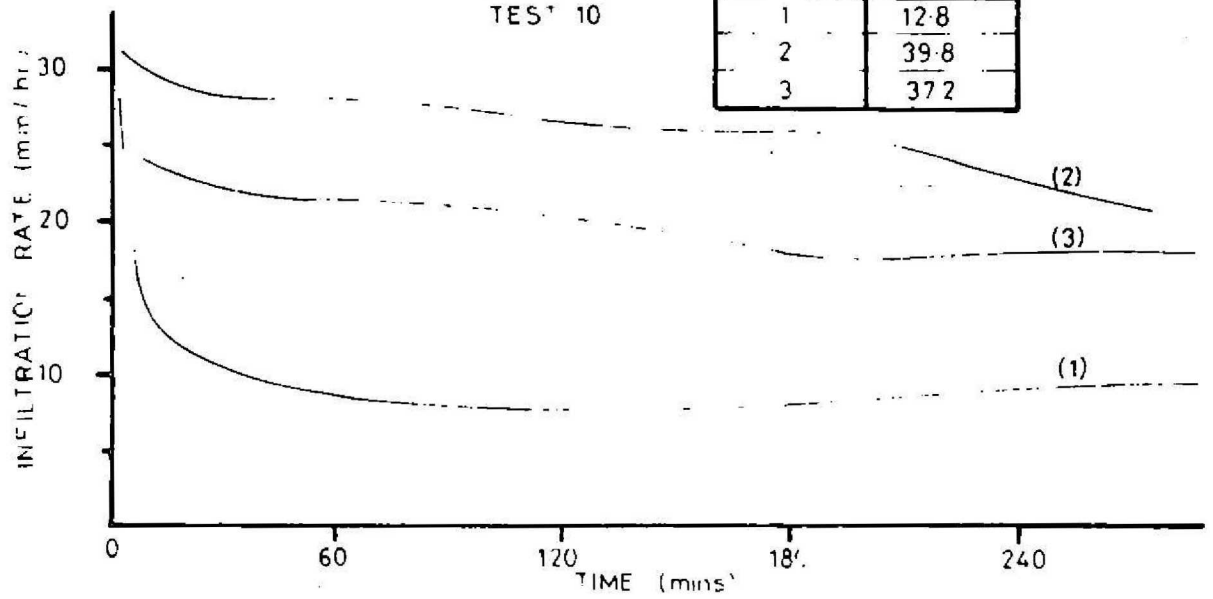


FIGURE 9

MASSIVE GLEYED CLAY

SITE M110  
TEST 10

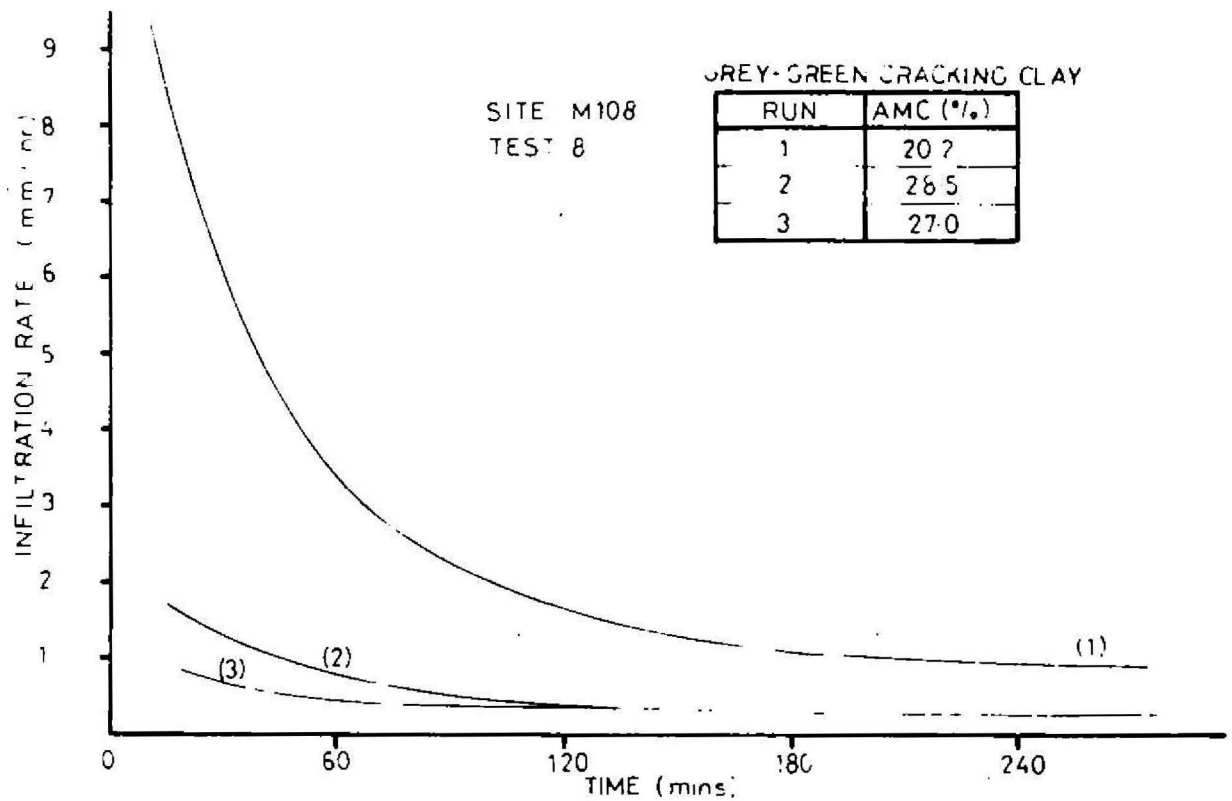
RUN	AMC (%)
1	12.8
2	39.8
3	37.2



GREY-GREEN CRACKING CLAY

SITE M108  
TEST 8

RUN	AMC (%)
1	20.7
2	28.5
3	27.0



MASSIVE RED GRAVELLY EARTH

SITE M109  
TEST 9

RUN	AMC (%)
1	6.4
2	22.5
3	20.5
4	20.1

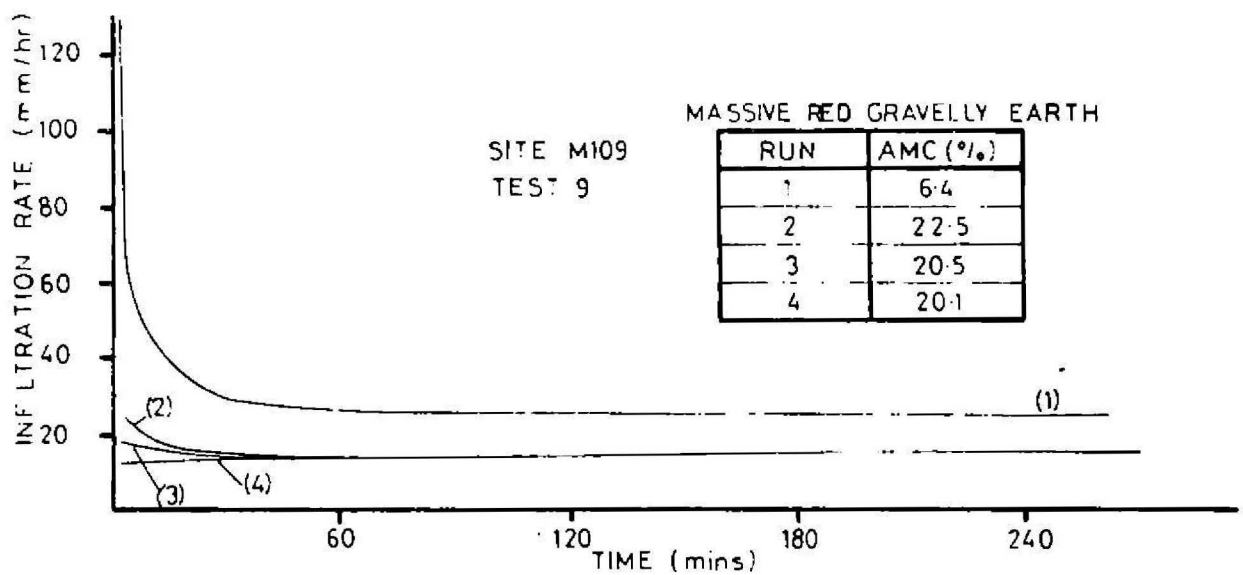
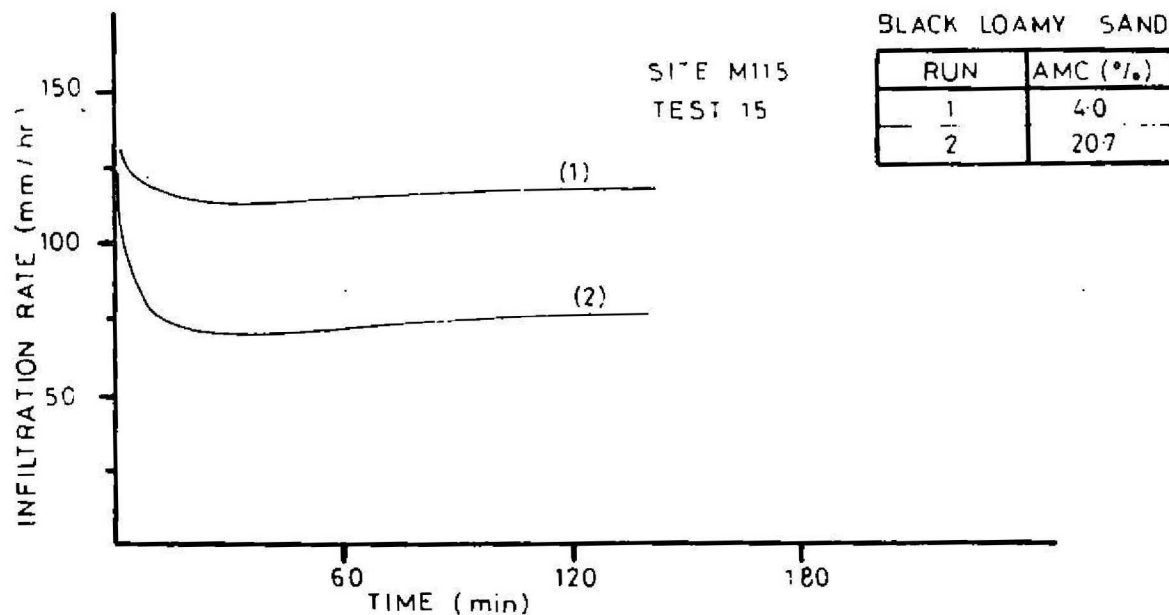
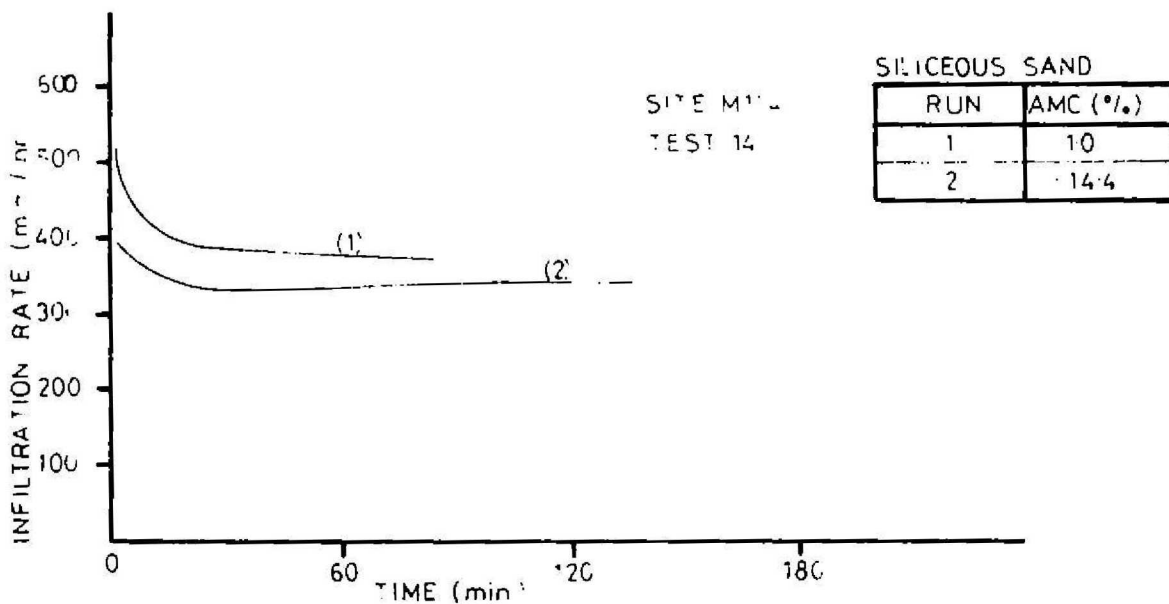
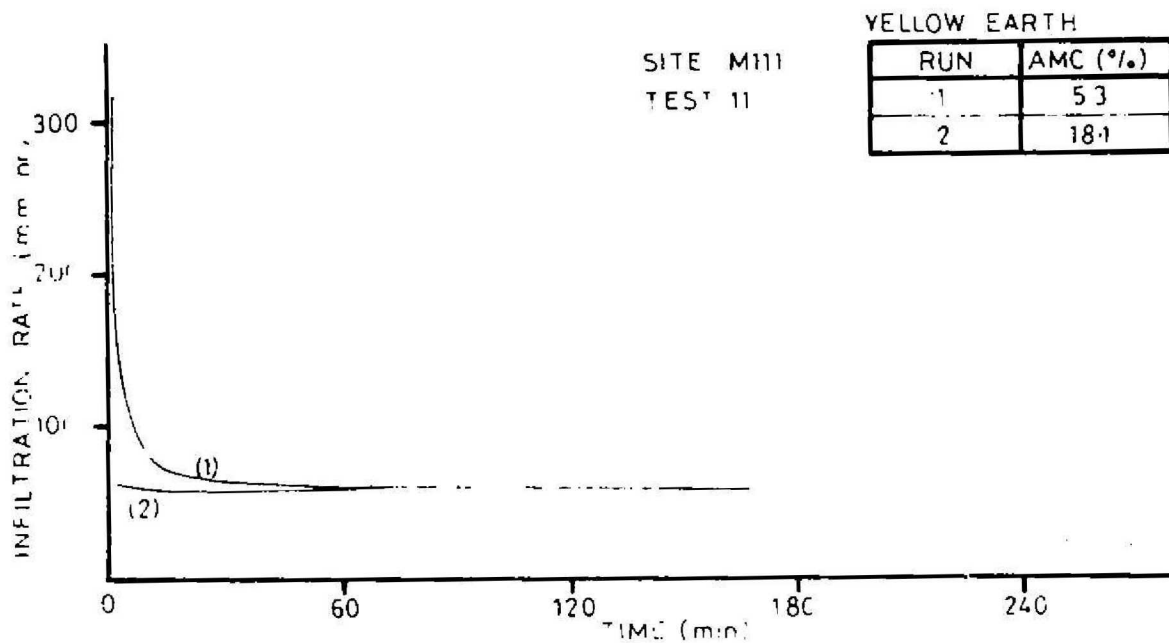


FIGURE 10



Lateritised bedrock (M104) gives a steady-state infiltration rate of around 10 mm/hr showing that hydraulic continuity exists within the vugh network, but admittance rates are very high and erratic until the vughs fill with water. The vesicular lateritised alluvium/colluvium was not tested because it proved impossible to obtain a ring sample or to put down an auger hole without disturbing the fabric of the sample.

The cracking clays around the billabongs of the lower Magela flood plain (M107) gave very high initial infiltration rates when the AMC was below the shrinkage limit. Open cracks up to 2 cm wide permit large volumes of water to gravitate through the soil before the clay absorbs water and swells to close off the transmission planes. Volume changes of up to 20 percent were observed in the clays after wetting at sites M101, M107, M108, and M110.

In some clay soils (M101, M107, M110) an apparent increase in infiltration rate was measured after 1 to 2 hours. This phenomenon is probably caused by delayed air expulsion and changes in atmospheric pressure during the tests.

The infiltration testing of the clays of the lower Magela flood plain (M101, M107, M108) indicated that the first rainstorm of the wet season will produce very little runoff. Thereafter the combined effects of higher soil moisture content and rising water-table will ensure ponding and runoff of most of the water during the rest of the wet season. The sandy soils of the upper Magela flood plain are capable of admitting large amounts of precipitation until the water-table rises to near the surface. Runoff from the upper flood plain in the early wet season should only occur from storms of intense rainfall.

#### SURVEYING AND LEVELLING OF BOREHOLES

Surveying and levelling of proposed boreholes was undertaken by the field party to establish grid references according to the Australian Geodetic Grid 1966, and heights above mean sea level were tied to the Australian Height Datum. Magela Creek was

TABLE 5

SATURATED HYDRAULIC CONDUCTIVITIES  
DETERMINED FROM INFILTRATION TESTS

Site No.	Test No.	NT Land Conserv. Soil Gp	Description of soil	Depth (m)	Type of test	$k_{SAT.}$ (mm/hr)
<u>MAGELA CREEK CATCHMENT</u>						
M101	1	7A3	Black peaty clay	0-0.15	Ring infiltro-meter	3.7
M101	1A	7A3	Black loam	0.3-0.45	Ring infiltro-meter	30.3
M103	3	5E	Siliceous sands	0-0.3	Ring infiltro-meter	31.5
M104	4	-	Lateritised weathered schist	0.93-1.52	Auger hole	12.1
M104	4A	-	Lateritised weathered schist	0.70-1.09	Auger hole	5.8
M106	6	5E	Siliceous sands	0.52-1.02	Auger hole	14.6
M106	6A	5E	Siliceous sands	0.6-1.1	Auger hole	13.5
M107	7	7A3	Black cracking clay	0-0.3	Ring infiltro-meter	0.9
M108	8	9A	Grey-green cracking clay	0-0.3	Ring infiltro-meter	0.2
M109	9	3C2	Massive red gravelly earth	0-0.3	Ring infiltro-meter	18.6

TABLE 5 (cont.)

Site No.	Test No.	NT Land Conserv. Soil Gp	Description of soil	Depth (m)	Type of test	k <sub>SAT</sub> (mm/hr)
<u>MAGELA CREEK CATCHMENT (cont.)</u>						
M110	10	6B2	Massive gleyed clay	0-0.3	Ring infiltrometer	14.6
M111	11	4E	Yellow earth	0-0.3	Ring infiltrometer	56.0
M112	12	-	Mottled gravelly clay	1.28-1.78	Auger hole	1.7
M113	13	-	Medium yellow-brown sand	1.1-1.5	Auger hole	31.7
M114	14	5E	Siliceous sands	0-0.15	Ring infiltrometer	30.3
M115	15	5E	Black loamy sand	0-0.15	Ring infiltrometer	60.7
<u>NABARLEK CATCHMENT</u>						
N104	4	-	Grey earth	0.10-0.42	Auger hole	1.6
N104	4A	-	Mottled silty sand	0.6-0.9	Auger hole	11.4
N104	4B	-	Mottled sandy clay, dense	1.65-2.2	Auger hole	0.4
N105	5	-	Pale grey fine slopewash	0.3-0.96	Auger hole	4.9
N108	8	-	Mottled yellow gravelly sand	0.52-0.95	Auger hole	22.9

TABLE 5 (cont.)

Site No.	Test No.	NT Land Conserv. Soil Gp	Description of soil	Depth (m)	Type of test	k <sub>SAT</sub> (mm/hr)
<u>NABARLEK CATCHMENT (CONT.)</u>						
N108	8A	-	Mottled grey & red clayey sand	0.78-1.05	Auger hole	34.7
N108	8B	-	Massive sandy clay	1.1-1.5	Auger hole	0.3
N109	9	-	Reddish brown friable silty clay	0.25-0.58	Auger hole	3.5
N109	9A	-	Red clayey sand	0.20-0.65	Auger hole	12.0
N109	9B	-	Yellow brown silty clay	0.20-0.69	Auger hole	1.2
N111	11	-	Red brown clay	0.18-0.73	Auger hole	0.9
N111	11A	-	Mottled yellow-grey massive sandy clay with carbonate nodules	1.30-1.52	Auger hole	0.08

plane-tabled for a distance of 100 m upstream and downstream of its intersection with traverse 1. No surveying or levelling was done at Nabarlek.

Instruments used were a Wild "T2" theodolite for horizontal angle determinations and tacheometry, and a Hilger & Watts "Auto Set" dumpy level for height measurements. A Wild self-reducing alidade was used for plane-tabling.

Borehole traverses were tied to stations surveyed by mining companies or to aerial survey markers, and initial bearings for a reference line were taken by compass. Time did not permit ties for loop closure. Tie points are shown in Appendix 4.

Twenty boreholes were surveyed in four traverses and five sites were fixed individually. Coordinates of holes are shown in Appendix 4. Boreholes which were surveyed but not drilled during 1979 have been marked in the field by yellow star pickets with BMR numbers.

A block of 100 Water Division registered numbers has been reserved for BMR boreholes.

Surveying and levelling of all bores not done in 1979 will be undertaken during the 1980 field season. If time permits, the holes proposed for drilling during 1980 will be surveyed during the season.

#### CONCLUSIONS

The upper Magela Creek is entrenched in Quaternary alluvium which has been deposited on the downthrown side of the Magela Fault. The alluvium constitutes the major surficial aquifer in the area and attains thicknesses of up to 70 m upstream from Mudginberri. Steady-state yields greater than 6.5 L/s were obtained in the flood-plain sands, and much higher rates are expected in gravels of palaeochannel deposits.

Red and yellow earths which cover the higher lateritised interfluvies of the upper Magela catchment are highly permeable. Measured constant-head infiltration rates ranged from 10 mm/hr in the lower catenary position to above 50 mm/hr in well drained sandy soils, indicating that significant interflow will occur on top of the laterite during the wet season. Consequently, surface pollutants from the mining areas are likely to be mobilised and transported towards Magela Creek only during the wet season, which is also the time of maximum dilution.

Hydraulic continuity within the vugh network of the lateritised surface has been demonstrated by steady infiltration rates up to 10 mm/hr.

Initial infiltration rates in the cracking clays around the Magela billabong system are very high when the antecedent moisture content is low, but decrease to less than 1 mm/hr after 1 to 2 hours of wetting. The first rainstorms of the wet season will produce very little runoff on these clays, but runoff and ponding will take place in later storms because of higher initial soil moisture content and a rising water-table. Runoff from the sandy soils of the upper Magela flood plain will not occur until much later in the wet season.

At Nabarlek steady-state yields up to 4.5 L/s were obtained in the alluvium and lowest terraces of Tin Camp Creek and Cooper Creek. Clay soils developed on reworked laterite on the interfluvies are characterised by very low seepage rates through their sub-sola.

Stagnation points of very slow groundwater flow are likely in the sands of the basin of upper Tin Camp Creek in prolonged dry seasons. This area required careful management because it lies below the Nabarlek sewerage pond, from which seepage into the sands would be maximised during the dry season. Pollutants would build up in the alluvium of the basin during the dry season and dilution would occur during the following wet season.

Groundwater flow rates during the dry season in Cooper Creek alluvium are considered to be very low because drainage is thought to be very low. The maximum subsurface discharge was estimated at 20 m<sup>3</sup>/day during September-October 1979. Careful monitoring of groundwater quality in Cooper Creek is necessary during dry seasons because the only source of recharge to the surficial aquifers is from the fractured rock aquifers below.

#### PROGRAM FOR 1980

##### Drilling

It is proposed to install 72 monitoring bores in the Magela catchment as shown in Plates 1 and 2. This figure includes 28 bores which were planned for 1979, but which the contractor failed to drill, and a further 44 holes originally planned for the



1980 field season. Eighteen shallow piezometers for stream underflow studies are planned in the Magela streambed at traverses 1, 2, and 3.

Six bores are planned for Nabarlek and these are located in stream alluvium (Fig. 2).

It is anticipated that the Nabarlek bores and the initial 28 Magela bores would be put down before 30 June 1980 and the remainder should be completed by 30 September 1980. In addition BMR drilling section has programmed four stratigraphic holes to be put down during September-October to the south of Cannon Hill at no charge to the OSS. These holes will provide a link with the stratigraphy obtained from a number of holes previously drilled by BMR to the northwest of the Magela Creek catchment.

#### Aquifer testing

Pump testing of bores to determine aquifer parameters is programmed to start in June and will carry through until October. Dye tracing experiments are planned to be conducted concurrently with the pump testing. Dye injection is to be carried out at Nabarlek bores N11, N12, N5 to N7, N2, and N3, and in the shallow piezometers on Magela traverses 1, 2, and 3. Some dye tracing is also planned for bores near the billabongs to test groundwater/surface-water connection.

#### Joint analysis, Kombolgie Formation

Fractures in the Kombolgie Formation are to be analysed as time permits throughout the field season. Structural domains on the western side of the Brockman Massif and on the Jabiluka Outlier have been selected for analysis, and approval has been obtained to enter these areas.

#### Surveying and levelling

Surveying and levelling of bores will continue throughout the field season.

Duration of field season

In order to carry out the program it is essential that the party depart from Canberra in early May and that drilling start in the fourth week of May. Fieldwork should finish by the end of October and the party should arrive back in Canberra by mid November.

ACKNOWLEDGEMENTS

The party is indebted to Dr Tjeard Talsma, CSIRO, for his advice on infiltration testing.

Scientist from the Office of the Supervising Scientist, the NT Water Division, the NT Geological Survey, and the NT Land Conservation Unit were most helpful to the party both in the field and in Darwin.

Chris Haynes of the Australian National Parks and Wildlife Service, and David Rourke of the Northern Land Council assisted the party greatly by negotiating on our behalf with the traditional landowners of the area and by obtaining permission for drilling.

The mining companies cooperated fully with the party; in particular, the courtesy and assistance of Safar Sarmed (Ranger) and Les Hunter (QML) are gratefully acknowledged.

Clive Prichard of BMR provided the party with solid backing from Darwin and offered wise counsel, which was much appreciated, on a wide range of matters.

The Division of National Mapping's loan of surveying equipment to the party is gratefully acknowledged.

The contributions of the OSS in providing accommodation at Jabiru, and of QML in providing accommodation at Naborlek, are gratefully acknowledged.

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## Appendix 1

**Description of BMR Contribution to Alligator Rivers  
Hydrogeological Study: Approved by the  
Hydrogeological Committee, 9 April 1979**

## DESCRIPTION OF BMR CONTRIBUTION TO ALLIGATOR

### RIVERS. HYDROGEOLOGICAL STUDY 1979

#### AIMS

- (i) To investigate the hydrogeology, including hydro-chemistry, of the surficial (generally unsaturated) cover in the Magela Creek and Coopers Creek catchments.
- (ii) To determine the diffusivity and dispersion tensors of the surficial material as an input to the mass transport model to be developed by Water Division NT Department of Transport and Works.
- (iii) To determine infiltration capacities, zones of surface recharge and subsurface piping by buried alluvial aquifers, as an input to the source terms for the groundwater hydraulics model to be developed by Water Division.
- (iv) To participate in the Hydrogeological Committee. By this means and by general consultation to contribute to the overall Alligator Rivers Hydrogeological project study and ensure uniformity of data input.

#### PROJECT 1 Hydrology of Alluvial Aquifers.

- (a) Distribution and morphology of aquifers, based on Survey of available data and airphoto interpretation, plus field mapping (see below). To be confirmed by drilling. Any geophysical input will be by others.
- (b) Drilling and equipping of Piezometers. Estimated requirements is 100 piezometers for Magela Catchment and 25 for Cooper Creek Catchment (some variations will be necessary in the light of field experience). Thin ( $\frac{1}{2}$ " ) pvc tubes and tensiometers will be used for aquifers of low K; pumping bores cased with 3" - 4" pvc tubing will be used for aquifers of high K.
- (c) Determination of (T & S (confined) ) by pump testing  
(K & M (unconfined)

PROJECT cont.

- (d) Regular chemical sampling throughout the field season arrangements are to be made with Water Division (or Geological Survey in Project Areas?) for sampling to be done during the Wet Season. Samples are to be despatched initially to AMDL in Adelaide, for determination but may subsequently be determined in Jabiru Laboratory by Water Division Staff. Uniform sampling procedures and analytical techniques are to be agreed on by WD, GS and BMR.
- (e) Production of potentiometric contour map for surficial sediments for Magela and Cooper catchments. Limited additional work will be done as required in areas of surface recharge into streams, piping etc.
- (f) Tracer studies to determine travel times (dyes only).
- (f) Statistical analysis of geochemical trends.

2. Surficial Geology and Infiltration Capacities.

- (a) Map and determine the characteristics of the surficial (post - Proterozoic) sediments and sites of the Magela and Cooper catchments below the escarpment.
- (b) Produce map(s), at 1:25 000 scale, showing the distribution and thickness of the units referred to in 2(a).
- (c) Infiltration testing of all the mapping units identified by 2(a). In view of the limited field season it will be possible to determine kSAT only. It is proposed to use
  - (i) Flooding - type constant head
  - (ii) Downhole injection
- (d) Mineralogical investigation of clays, using: X-Ray diffraction (representative samples to be determined by BMR, routine samples by AMDL).

## VEHICLES

- 2 x 3 ton International trucks (one only if based at Jabiru)
  - 1 from Canberra, 1 from Brisbane
- 2 x L.W.B. Landrovers - 1 from Canberra, 1 from Darwin
- 1 x S.W.B. Landrover - from Canberra
- 1 Laboratory caravan from Canberra
- 1 Water tanker - from Darwin
- 1 Pump trailer unit - from Canberra if available

(Note: These vehicles and equipment can be provided by BMR but reimbursement of hire charges, repairs and maintenance and fuel will be sought from Office of the Supervising Scientist)

## EQUIPMENT (other than vehicle and accommodation equipment)

- \* Indicates that item so marked has to be acquired by purchase or by loan from other organisations
- + Indicates that item so marked will need to be purchased and constructed by BMR before field season
- \*
  - \* 2 Low capacity pumps
  - \* 1 High capacity pump
  - \* Pressure transducer(s) to record instantaneous variation in water level
  - \* Tensiometers (80) to determine progress of wetting front through surficial cover
  - \* 3" to 4" pvc pipe (to be installed by driller, perforated by BMR). Screens
  - + Chemical grouts, ceramic pots, galvanized pipe, 1/2" pvc pipe, glues and cement (to be installed by BMR or by driller under BMR supervision)
  - \* Electronic recorders (2) for water level measurements in thin piezometers
  - \* Salinity meter
  - + Infiltrometers and downhole injectors (to be constructed by BMR)
  - \* Dyes and recorder(s)
- Portable laboratory, standard field and laboratory equipment. Consumables to be paid for by OSS.

PROJECT cont.

- (d) Scanning electron microscopy (BMR)  
Cation exchange capacity (CSIRO or AMDL)
- (e) Determination of diffusivity and dispersion  
tensors by slug injection.
- (f) Sediment samples - collection, preparation and  
testing by BMR.

3. Joint Analysis in Kombolgie Sandstone - field and  
office studies

STAFF

J.R. Kellett, Party Leader.

Office studies, equipment preparation May - June. Field work  
June - September Office Studies, laboratory studies,  
Petrography and report writing November - December (continuing 1980)

W.R. Evans.

Office studies, equipment preparation May - June. Field work  
June - October. Office studies part-time November - December  
(continuing 1980)

Technical Officer, Grade 1.

Equipment preparation May - June. Field work July - October.  
Sample preparation and testing November - December.

Technical Assistant, Grade 2.

As for TO: May - December.

Field Hand(s).

- 1 if party based at Jabiru (some further assistant may  
be required at times) June - October.
- 2 if party has its own field camp.

(Services of mechanic may be required from time to time to service  
equipment and vehicles).



#### EQUIPMENT cont.

- \* Automatic bore recorders (6)

Note Bore completion, security and identification facilities to be supplied and installed by drilling contractor.

#### AIR PHOTOGRAPHY AND BASE MAPS

One set of colour photographs (to be supplied by OSS)  
Base maps, preferably at 1:25 000 scale over entire area; larger scale maps as available in areas adjacent to areas of special interest. Infra-red photography.

#### ACCOMMODATION

Require facilities for 6 + 3 (visitors)  
Messing at Jabiru and Nabarlek (if available), as appropriate to field program, preferred with facilities for field camping or job (bedroll, cooking facilities only).  
Alternative is base camp. Equipment for base camps and fly camps to be provided by BMR.

#### CONTRACT SERVICES

Drilling (125 holes, average depth 10 m) Undisturbed sampling.  
Airlifting may be required in high transmissivity alluvial aquifers. Large diameter perforated pvc casing (and/or screens) to be installed by contractor. Wells to be completed and secured by contractor.

Water sample testing: determinations by AMDEL, subsequently by Water Division.

Petrography: thin sections prepared by AMDEL or BMR  
period contractors, descriptions by BMR.

X-Ray Diffraction determination of routine samples by AMDL.

Cation exchange capacity of soils. Determination of samples collected by BMR by CSIRO or AMDL.

Note All external services to be charged to Office of the Supervising Scientist. Sediment samples to be prepared and tested by BMR.

## Appendix 2

### Barélogs of Holes Completed in 1979

**PRINTED**

~~HEADING OF TITLE SHEET~~

~~NAME OF PROJECT~~

~~NAME OF SITE~~

GEOLOGICAL LOG OF BORE.

BMR

ALLIGATOR  
RIVERS.

MZ

LOCATION OF BORE: MAGELA CK

R.N.:

MAP NAME: CAHILL 1:100 000

I.N.:

5472

GRID REFERENCE: 766595

PROJECT:

DOWNHOLE LOGGING  
GAMMA RAY LOGGING  
BY NTGS

DRILLING METHOD:  
Dry plug - 0.00 to 5.30  
Rotary - 5.30 to 10.07

HOLE COMMENCED: 10/10/79  
HOLE COMPLETED: 11/10/79

JOB NUMBER:

DEPTH (m)		GRAPHIC LOG	LITHOLOGY	AQUIFERS YIELD SWL	ADDITIONAL INFORMATION
LIFT AND CORE RECOVERY	To				
	(m)		Light brown to grey silt to silty sand.		
	0.7		Mottled yellow-brown		
	1		White to light grey sand		
	2				
			Purple-grey medium sand		
	3		Grey sand		
	3.2		Mottled yellow grey sand		
	3.8			▼ 3.72 8/10/79	
	4		Mottled yellow-red sand		Angular quartz ? latents
			Grey sand - no gravel		
			Some brown and yellow mottles		
	4.8		Yellow mottling intensifies		
	5				
			Yellow sand		
	5.6				
	5.9	Core loss			
	6		minor small angular gravel		
	6.5			Minor	
	7		Yellow-grey silty sand with angular quartz pebbles. Some red mottles	superficial basal gravel	
	8				
	9				
	9.56				
	10.07		E.W. Bedrock Brown schist		
		EOH			
			Record 1979/88		

DEPTH (m)		GRAPHIC LOG	LITHOLOGY	AQUIFERS YIELD SWL	ADDITIONAL INFORMATION
LIFT AND CORE RECOVERY	To				
	(m)		Grey brown silty sand - organic		
	0.3		Light grey silty sand		
	0.6				
	1		Yellow-brown mottled silty sand. Sand slightly more coarse than above		
	2				
	2.3	core loss		2.25 3110	
	3			Aquifer	
	3.2		White to light grey coarse sand - some fines	1.21 32/sec	Slightly indurated
	3.8				
	4	core loss			
	4.1		Grades to yellow and grey mottled sand.		Indurated
	5		Minor angular qtz pebbles		
			Yellow mottles dominate	Aquifer	
	6		Intense red and yellow mottles		
	6.25	core loss			
			large gravel at base		
	7		E-W Bedrock Schist. Same weathering pattern as above		
	7.60				
		EOH			
<div style="border: 2px solid black; padding: 5px; display: inline-block;"> <b>PRELIMINARY, UNEDITED</b> </div>					
Record 1979/80					

~~Department of the Interior~~

~~Mineral Resources~~

~~Geological Survey~~

GEOLOGICAL LOG OF BORE. M4

BMR ALLIGATOR RIVERS

LOCATION OF BORE: MAGELA CK.

MAP NAME: CAHILL 1:100 000

5472 773966

GRID REFERENCE:

R.N.:

I.N.:

PROJECT:

DOWNHOLE LOGGING  
GAMMA RAY LOGGING  
BY NTGS

DRILLING METHOD:  
Dry plug c to 5.75  
Rotary 5.75 to 16.50

HOLE COMMENCED: 26/10/79

HOLE COMPLETED: 26/10/79

JOB NUMBER

DEPTH (m)		GRAPHIC LOG	LITHOLOGY	AQUIFERS YIELD SWL	ADDITIONAL INFORMATION
LIFT AND CORE RECOVERY	To				
	(m)		Grey silty sand		
	0.5				
	1		Grey to yellow grey silty sand		
	2				
	3		Some yellow brown mottles		
	3.5			YIELD 26 3.45	
	4		Red-brown mottled sand	▼ 3.50 3.110	
	5				
	5.5		Unconsolidated clean sand. Brown to grey	Aquifer to bedrock	
	6				
	7	Cone loss			
	8				
	9				
	10				

PRELIMINARY, UNEDITED

Record 1979/88

Record 1979/88



~~Geological Log of Bore~~

~~Geological Log of Bore~~  
GEOLOGICAL LOG OF BORE.

BMR  
M5

ALLIGATOR  
RIVERS

LOCATION OF BORE: MAGELA CK

MAP NAME: CAHILL 1:100 000  
5472

GRID REFERENCE: 779596

R.N.:

I.N.:

PROJECT:

DOWNHOLE LOGGING  
GAMMA RAY LOGGING  
BY NTGS.

DRILLING METHOD:  
Dry plug 0.00 to 2.70  
Rotary 2.70 to 12.10

HOLE COMMENCED: 25/10/79

HOLE COMPLETED: 25/10/79

JOB NUMBER:

DEPTH (m)		GRAPHIC LOG	LITHOLOGY	AQUIFERS YIELD SWL	ADDITIONAL INFORMATION
LIFT AND FROM CORE RECOVERY	TO				
	0.10		Dark Brown silty sand. Loose white to yellow sand		Yellow brown mottles
1	1.20				
2	2.43		loose dark grey fine sand. Saturated	▼ 2.04 31/10	
3		Core loss		Seepage only.	
4			Fine sand in cuttings	Continuous Aquifer to Bedrock	
5					
6					
7					
7.70			organic pebbly silty sand.		Dark grey to black
8			Dense olive grey clays. Pedal. Fe stained cutans		
8.70			Gravel at base		
9		Core loss	Gravel layer hit during reaming		
10					

PRELIMINARY, UNEDITED

Record 1979/88



BMR ALLIGATOR  
M5 (cont.)

**PROJECT :**

**HOLE COMPLETED :**

[illegible]

DRILL TYPE: GEMCO

~~Geological Log of Bore~~

~~Geological Log of Bore~~  
BMR  
Geological Log of Bore.

ALLKATOR  
RIVERS  
M20

LOCATION OF BORE: MAGEZA CK.

MAP NAME: CAHILL 100,000 100,000  
5472

GRID REFERENCE: 705593

R.N.:

I.N.:

PROJECT:

DOWNHOLE LOGGING  
GAMMA RAY LOGGING  
BY NTGS

DRILLING METHOD:  
Dry plug to 3.20  
Rotary 3.20 to 9.97

HOLE COMMENCED: 16/10/79

HOLE COMPLETED: 17/10/79

JOB NUMBER:

DEPTH (m)		GRAPHIC LOG	LITHOLOGY	AQUIFERS YIELD SWL	ADDITIONAL INFORMATION
LIFT AND FROM CORE RECOVERY	To				
	(m)				
	.4		Grey hard silt		
	1		Yellow to light grey medium sand		
	2			▼ 1.44 5110 2.1/sec	
	3		Some layering evident beds 3 to 5 cm thick distinguished by Fe contents	Aquifer continues to bedrock	
	3.40	core loss		Main section	
	4		Dark brown gravelly sand Gravel is reworked laterite	at base	
	4.9 5		Bedrock Sequence of highly weathered gneiss and schist		
	6				
	7				
	8				
	9				
	9.97	EOH			

PRELIMINARY, UNEDITED

DRILLER: F.A. KELLY  
Record 1979/88

DRILL TYPE: GEMCO

Record: 1979-88

DRILL TYPE: GEMCO

~~HEAVYWEIGHT LOGGING~~

~~Heavy Gauge~~

~~Heavy Gauge~~

GEOLOGICAL LOG OF BORE.

BMR

ALLIGATOR  
RIVERS.

M23

LOCATION OF BORE: MAGELA CK R.N.:

MAP NAME: CAHILL 100000 Map 5472 I.N.:

GRID REFERENCE: 701593 PROJECT:

DOWNHOLE LOGGING  
GAMMA RAY LOGGING  
BY NTGS.

DRILLING METHOD:  
Dry plug to 4.25  
Rotary 4.25 to 8.22

HOLE COMMENCED: 12/10/79  
HOLE COMPLETED: 20/10/79

JOB NUMBER:

DEPTH (m)		GRAPHIC LOG	LITHOLOGY	AQUIFERS YIELD SWL	ADDITIONAL INFORMATION
LIFT AND FROM CORE RECOVERY.	To				
	(m)				
	1		Grey fine sand to silt		
	2		Yellow-brown clean medium sand		
	2.8			▼ 2.44 3/110	
	3		Light grey medium sand	Seepage only.	
	3.5				
	4		Dark grey <sup>sand</sup> grading to light grey clayey sand		
	4.61				
	5	CORE LOSS			
	5.97				
	6				
	6.2	CORE LOSS	white to light grey clean fine sand		
	6.5				
	7	CORE LOSS	FW 8.11		
	8				
	8.22	EDH			
	9				

PRELIMINARY UNEDITED

DRILLER: F.A. KELLY  
Record 1979/88

DRILL TYPE: GEMCO

GEOLOGICAL LOG OF BORE. M24

LOCATION OF BORE: MAGEZA CK

**R.N.:**

MRP NAME : CAHILL, BOB C. 1/27/24  
5472

**IN:**

GRID REFERENCE: 670 593

**PROJECT :**

## DOWNHOLE LOGGING

## GAMMA RAY LOGGING

B4 NTGS.

**DRILLING METHOD:**

Dry plug  
0.00 to 5.00

HOLE COMMENCED: 21/10/79

HOLE COMPLETED: 21/10/79

**Job Number :**

[illegible]

**PRELIMINARY, UNEDITED**

DRILLER: F. A. KELLY

DRILL TYPE: GEMCO

Record, 1979/88







~~Geological Log of Bore~~ **BMR ALLIGATOR RIVERS**

~~Geological Log of Bore~~ **NI**

**LOCATION OF BORE:** ~~MADEL~~ **WATER CK**

**MAP NAME:** ~~GENP ELL 1100 COR~~ **5573**

**GRID REFERENCE:** **175121**

**R.N.:**

**I.N.:**

**PROJECT:**

**DOWNHOLE LOGGING**

**DRILLING METHOD:**  
Dry plug 0 to 3.00  
Rotary 3.00 to 8.50

**HOLE COMMENCED:** 5/10/79

**HOLE COMPLETED:** 6/10/79

**JOB NUMBER:**

DEPTH (m)		GRAPHIC LOG	LITHOLOGY	AQUIFERS YIELD SWL	ADDITIONAL INFORMATION
LIFT AND FROM PRE RECOVERY	TO				
	(m)				
	1		Light grey to grey coarse sand highly porous.		
	2				
	3				
	4	CORE LOSS			
	5				
	5.36				
	5.86				
	5.80		A.W. Bedrock		
	6	CORE LOSS	Schist		
	7				
	8				
	8.50				
		EOH			

**PRELIMINARY, UNEDITED**

IN:

GEOLOGICAL LOG OF BORE. N2

GRID REFERENCE: 172415

PROJECT

## DOWNHOLE LOGGING

### DRILLING METHOD:

HOLE COMMENCED: 26.9.79

JOB NUMBER

N.A.

Rotary

HOLE COMPLETED: 26.9.79

[illegible]

~~Department of the Interior~~

~~Mineral Resources~~

~~Geological Survey~~

Geological Log of Bore. N3

BMR

ALLIGATOR  
RIVERS

LOCATION OF BORE: COOPER CK

MAP NAME: DEN DELIA 1501 N 0200  
5573

GRID REFERENCE: 171417

R.N.:

I.N.:

PROJECT:

DOWNHOLE LOGGING

DRILLING METHOD:

Dry plug 2-84

Rotary 2-84 to 9-27

HOLE COMMENCED: 27/9/79

HOLE COMPLETED: 5/10/79

JOB NUMBER:

DEPTH (m)		GRAPHIC LOG	LITHOLOGY	AQUIFERS YIELD SWL	ADDITIONAL INFORMATION
LIFT AND FROM CORE RECOVERY	To				
	(m)				
	1		Light grey to white silty sand. Highly porous Unconsolidated		
	2				
	2-84		silt content decreases		
	3	CORE LOSS			
	4				
	4-50				
	5		Contains pieces of M.W. Kombolgie sandstone - white to pink.		
	5-65				
	6	CORE LOSS			
	6-42				
	7		<b>PRELIMINARY, UNEDITED</b>		
	8		Kombolgie fragments slightly larger		
	9				
	9-27				
		EOH			

Record 1979/88

DRILLER:

DRILL TYPE:

151-4





Record 1979/88

~~Internal Circulation~~

~~CONFIDENTIAL - SECURITY~~

GEOLOGICAL LOG OF BORE. N7

BMR ALLKATOR  
RIVERS

LOCATION OF BORE: NABAR LEIK

MAP NAME: OENPELLI 1:100000  
5573

GRID REFERENCE: 199413

LRN:

IN :

PROJECT :

## DOWNHOLE LOGGING

**DRILLING METHOD:**

Dry plug 2-1.4

Rotary 1.4-3.58

HOLE COMMENCED: 24.9.79

HOLE COMPLETED: 24.9.79

JOB NUMBER

[illegible]

~~Water Resources~~

**BMR ALLIGATOR RIVERS**

~~Hydrogeological Survey~~  
GEOLOGICAL LOG OF BORE. **N8**

LOCATION OF BORE: **NABARLEK**

MAP NAME: **OENPELLI 1:100 000**  
**5573**

GRID REFERENCE: **194388**

R.N.:

I.N.:

PROJECT:

**DOWNHOLE LOGGING**

DRILLING METHOD:  
Dry plug 4.2m  
Rotary 4.20 to 700

HOLE COMMENCED: **20/9/79**

HOLE COMPLETED: **23/9/79**

JOB NUMBER:

DEPTH (m)		GRAPHIC LOG	LITHOLOGY	AQUIFERS YIELD SWL	ADDITIONAL INFORMATION
LIFT AND CORE RECOVERY	To				
	(m)				
	0.4		Light grey to yellow fine to medium sand		
			Mottled light grey to red brown clayey sand - some gravel		
	1		Mottled light grey to red brown silty clay.		
	1.6		Mottled light grey to yellow gravelly sand		
	2		Yellow brown to brown sandy gravel.		
	2.5				
	3		E.W. Bedrock. Light green to white mottled clay. Dolerite		
	4				
	5				
	6				
			M.W. Dolerite		
	7				
		ECH			
	8				
	9				
	10				

**PRELIMINARY, UNEDITED**



~~Geological Log of Bore~~

Geological Log of Bore.

**BMR ALLIGATOR RIVERS.**

**N9**

LOCATION OF BORE: **NABARLEK**  
MAP NAME: **OENPELLI 1:100 000**  
**5573**  
GRID REFERENCE: **200385**

R.N.:

I.N.:

PROJECT:

DOWNHOLE LOGGING

DRILLING METHOD:  
Dry plug 0-2.2  
Rotary 2.2-10.0

HOLE COMMENCED: 20.9.79  
HOLE COMPLETED: 22.9.79

JOB NUMBER

DEPTH (m)		GRAPHIC LOG	LITHOLOGY	AQUIFERS YIELD SWL	ADDITIONAL INFORMATION
LIFT AND CORE RECOVERY.	To				
	(m)				
	0.25		Yellow-Grey silty fine sand.		Slightly organic
	0.9		Yellow-Brown to Red-Brown mottled silty clay. Contains reworked lateritic gravel		Slightly lateritised at base
	1		Dense grey clay - some lateritic gravel in top 10cm.		
	2		Gravel replaced by ? secondary feldspar		
	3		Extremely weathered Dolomite		
	4				
	5				
	6				
	7				
	8				
	9				
	10	EOH.			

**PRELIMINARY, UNEDITED**

Record 1979/88

Record 1979/88

Record 1979/88

~~Walter G. Anderson~~

GEOLOGICAL LOG OF BORE. N12

BMR ALLIGATOR  
RIVERS

LOCATION OF BORE: NABARLEK

MAP NAME : OENPELL 1:100 000  
5573

GRID REFERENCE: 179375

**AN:**

*IN:*

PROJECT :

## DOWNHOLE LOGGING

DRILLING METHOD:  
Dry plug 0 - 1.8 m  
Rotary 1.8 - 4.0 m

HOLE COMMENCED : 25.9.79

HOLE COMPLETED: 25.9.79

JOA NUMBER:

[illegible]



BMR ALLIGATOR  
RIVERS.

N14

LOCATION OF BORE: LOOPER CK

1 KN:

MAP: OENDELL 1700000 SHEET  
5573

**IN :**

GRID REFERENCE: 193394

**PROJECT :**

## DOWNHOLE LOGGING

DRILLING METHOD:  
Dry plug 0.0+22.40

HOLE COMMENCED : 23/9/79

JOB NUMBER

HOLE COMPLETED: 23/9/79

[illegible]

Record	1979	88
--------	------	----

LOGGED BY :

DRILLER :

DRUG TYPE

ALLIGATOR  
RIVERS

GEOLOGICAL LOG OF BORE. N15

LOCATION OF BORE: COOPER CK

TRN:

MAR:OBENRELL 1'100000 SHEET

***IN.:***

5573

GRID REFERENCE: 193451

PROJECT :

## DOWNHOLE LOGGING

**DRILLING METHOD:**  
Dry piling to 2-25  
Rotary 2-25 to 4-08

HOLE COMMENCED: 6/10/79

**JOB NUMBER :**

HOLE COMPLETED: 6/10/79

[illegible]

Record 1979/88

LOGGED BY : RE.

DRILLER: F.A. KELLY

NDH, TVDF, GEMCO

### **Appendix 3**

#### **Casing Schedules of Bores Drilled in 1979**



# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : MAGELA CASING SCHEDULE

BORE No: M1

R.N.:

LOCATION (map + grid): CAHILL 1:100 000 SHEET 5472  
COORDS 766595

I.N.:

PREPARED BY: F.A. KELLY / BMR

MATERIAL TYPE: BLANK uPVC  
SCREENS stainless steel

RL COLLAR:

CASING TOP

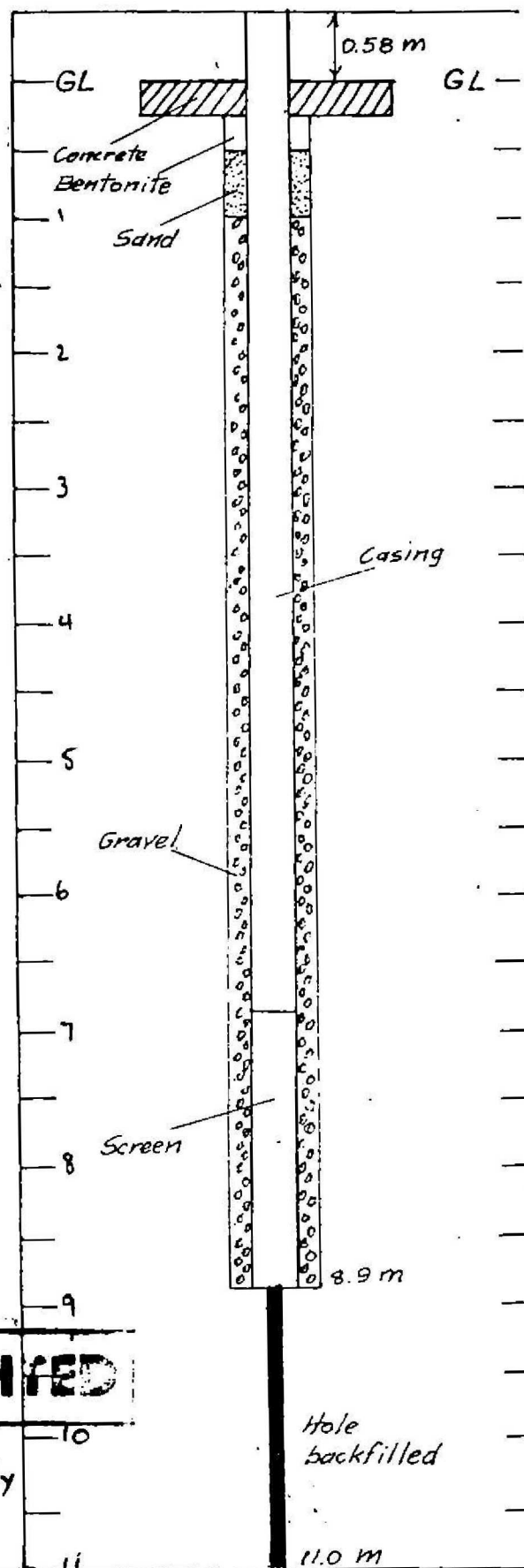
1 m above ground  
0.58 m above collar

DEPTH INTERVAL (m)	CASING DESCRIPTION
0.00 - 6.90	UPVC 140 mm diam
6.90 - 8.90	SCREENS: Stainless steel 1.2 m x 1.5 m in aperture 100 mm diam

\* Below ground level

GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
1.00 - 8.90	3/8" Gravel - crushed



**PRELIMINARY, UNEDITED**

Office use only

DATE RECEIVED:

CHECKED BY:  
Record 1979/89

Hole developed  
and capped. Not ready  
for sampling

Hole  
backfilled

# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : MAGELA CASING SCHEDULE

BORE No: M2

LOCATION (map + grid) : C. A. HILL 1:100,000 SHEET 5472  
COORDS 766595

R.N.:

I.N.:

PREPARED BY: KELLY / BMR

MATERIAL TYPE : BLANK UPVC  
SCREENS St. Steel

R.L. COLLAR

CASING TOP

0.64 m above ground  
0.64 m above collar

DEPTH INTERVAL * (m)	CASING DESCRIPTION
0 - 1.36	UPVC 140 mm diam
1.36 - 9.36	Screens: 100 mm diam 3 x 2 m x 1.5 mm aperture 1 x 0.6 m x 0.75 mm " " 2 x 0.6 m x 0.50 mm " "

\* Below ground level

GRAVEL PACK

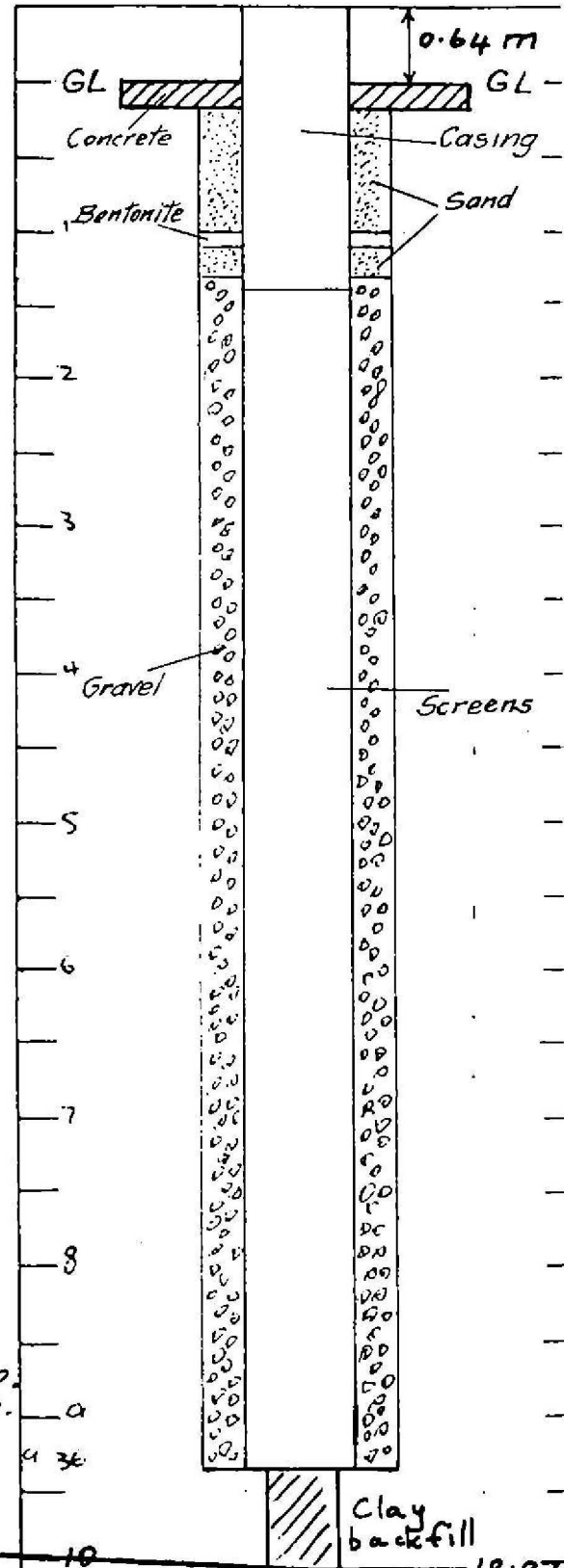
DEPTH INTERVAL * (m)	DESCRIPTION
1.30 - 9.36	3/8" crushed aggregate

status 1.11.79: Pumped intermittently since completion.  
Takes 2A hrs to recover - needs more pumping.

Record 1979/88

Office use only

DATE RECEIVED:  
CHECKED BY:



**PRELIMINARY, UNEDITED**

# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : MABELA CASING SCHEDULE

BORE No: M3

R.N.:

LOCATION (map + grid): CAHILL 1:100 000 SHEET 5472

I.N.:

PREPARED BY: KELLY / BMR COORDS 769596

MATERIAL TYPE : BLANK UPVC  
SCREENS St. Steel

R1 COLLAR:

CASING TOP: 0.4 m above ground  
0.40 m above collar

DEPTH INTERVAL <sup>*</sup> (m)	CASING DESCRIPTION
0 - 1.90 m	UPVC 140 mm diam
1.90 - 6.50	SCREENS: 100 mm diam 2 x 2m x 1.5 mm aperture 1 x 0.6m x 1.5 mm aperture

\* Below ground level

## GRAVEL PACK

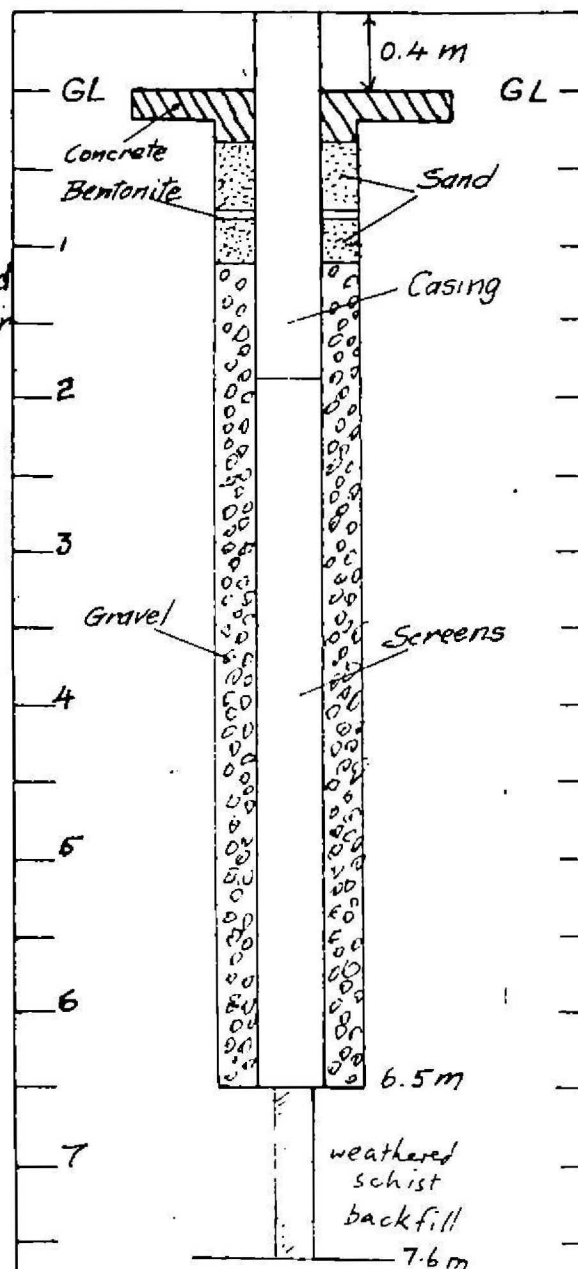
DEPTH INTERVAL <sup>*</sup> (m)	DESCRIPTION
1.10 - 6.50	3/8" Crushed aggregate

status: 13/12/79 pumped for 2 hrs 40 min. water clear.  
15/12/79 " " 40 min.  
16-18/12/79 pumped intermittently, ready for sampling.

Record 1979/88

Office use only

DATE RECEIVED:  
CHECKED BY:



**PRELIMINARY, UNEDITED**

DEPARTMENT OF TRANSPORT AND WORKS  
WATER DIVISION

PROJECT : BMR / MAGELA DRILLING CASING SCHEDULE

BORE No: M/4

R.N.:

LOCATION (map + grid): CAHILL 1100 CCC SHEET 5472  
COORDS 773966

IN:

PREPARED BY: F.A KELLY / BMR

MATERIAL TYPE : BLANK uPVC  
SCREENS stainless Steel

RL COLLAR:

CASING TOP: 0.30 m above ground  
0.50 m above collar

[illegible]

\* Below ground level

## GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
2.60 to 15.00	316" crushed aggregate

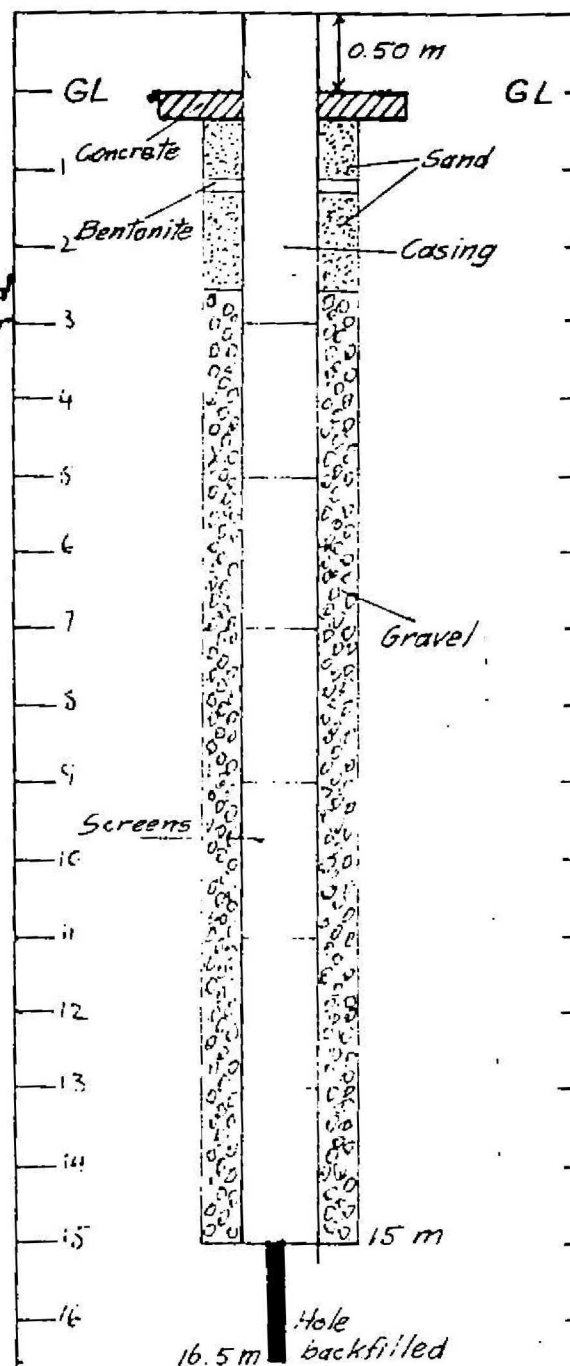
Date	Time	Remarks
20.10.79	11.00	Status: 20.10.79 pumped for 3 hrs - water clear, ready for sampling.

Office use only

DATE RECEIVED:

CHECKED BY :

Record 1979/88



**PRELIMINARY, UNEDITED**

# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

BORE No: M5

PROJECT : BMR | CASING SCHEDULE  
MAGELA DRILLING

R.N.:

LOCATION (map + grid): CAHILL 1:100 CCC SHEET 5472  
COORDS 779546

I.N.:

PREPARED BY: F.A. KELLY | BMR

MATERIAL TYPE: BLANK : PVC  
SCREENS Stainless Steel.

R/L COLLAR:

CASING TOP: 11 m above ground  
0.50 m above collar

DEPTH INTERVAL* (m)	CASING DESCRIPTION
0.00 to 1.50	PVC 140 mm diam.
1.50 to 11.50	Screens 100 mm diam 5 x 2 m x 1.5 mm aperture

\* Below ground level

## GRAVEL PACK

DEPTH INTERVAL* (m)	DESCRIPTION
1.30 to 11.50	3/8" crushed aggregate

Status: Pumped 19/10/79 20/10/79 - requires more pumping.  
seepage only entering hole.

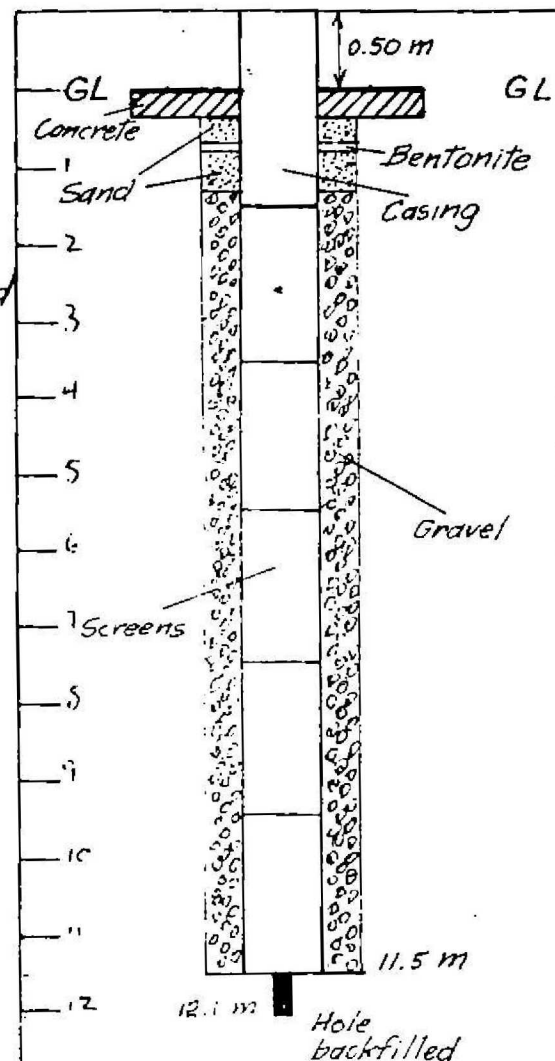
Office use only

DATE RECEIVED:

CHECKED BY:

Record 1379/88

**PRELIMINARY, UNEDITED**



DEPARTMENT OF TRANSPORT AND WORKS  
WATER DIVISION

PROJECT : BMR / MAGELA DRILLING CASING SCHEDULE

BORE No: M6

RN:

LOCATION (map + grid): CAHILL 1:100 000 SHEET 5472  
COORDS 779969

IN :

PREPARED BY: F.A. KELLY / BMR

MATERIAL TYPE : BLANK uPVC  
SCREENS Stainless Steel

RL COLLAR.

CASING TOP:

0.50 m above ground  
0.50 m above collar

[illegible]

\* below ground level

## GRAVEL PACK

DEPTH INTERVAL <sup>*</sup> (m)	DESCRIPTION
1.70 to 4.50	3/8" crushed aggregate

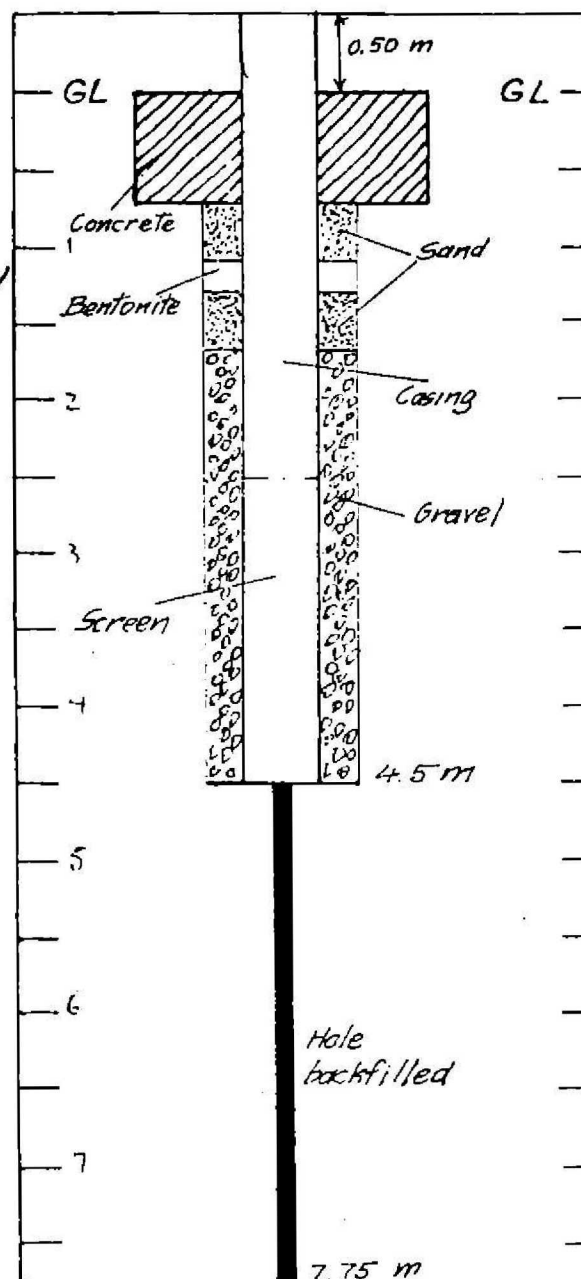
Status: 19-20/10/79 pumped. Not ready for sampling  
Seepage only at this stage.

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DEPARTMENT OF TRANSPORT AND WORKS  
WATER DIVISION

BORE No: M2C

PROJECT : BMR, CASING SCHEDULE  
MAGELA DRILLING

RN:

LOCATION (map + grid): CAHILL 1100000 SHEET 5472  
COORDS 705593

IN:

PREPARED BY: F. A. KELLY / BMR

MATERIAL TYPE : BLANK vPVC

SCREENS Stainless Steel.

RL COLLAR:

### CASING TOP:

0.60 m above collar  
0.60 m above ground

[illegible]

\* Below ground level

## GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
2.24 to 5.00	3/8" crushed aggregate

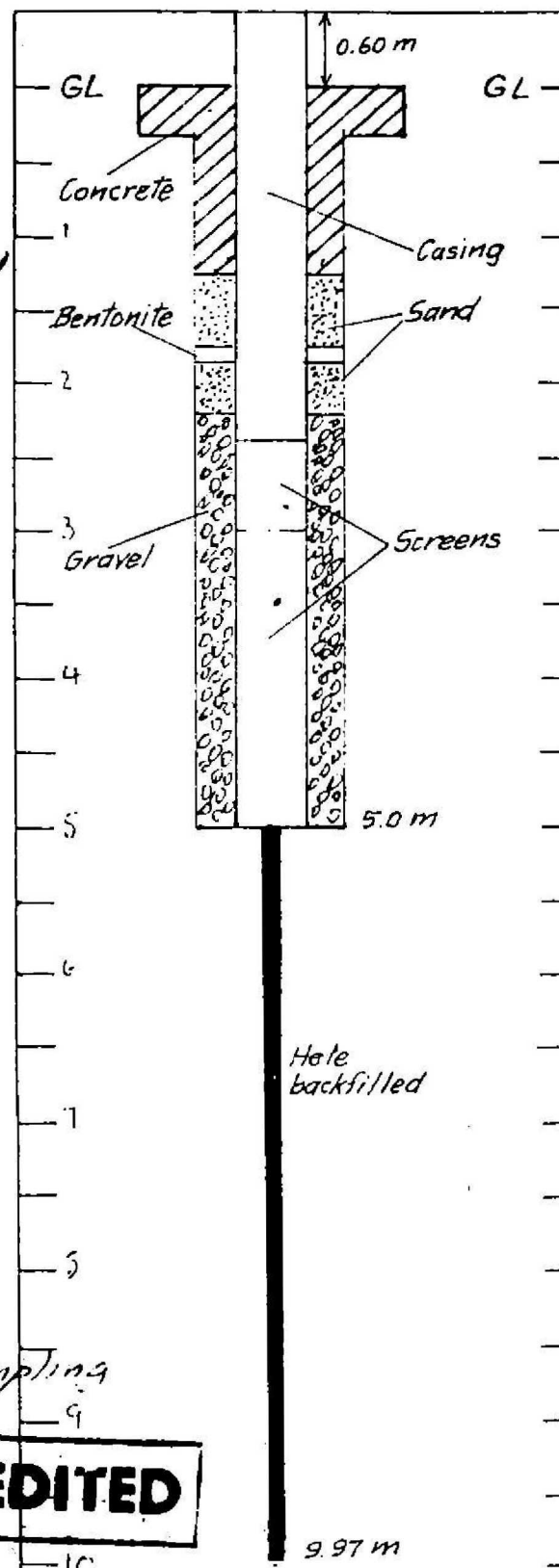
Status: 22/10/79 pumped ready for sampling

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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : BMR IMAGELA DRILLING

BORE No: M21

LOCATION (map + grid): CAHILL 1 100000 SHEET 5472  
COORDS 705593

R.N.:

IN:

PREPARED BY: F.A KELLY / BMR

MATERIAL TYPE : BLANK UPVC  
SCREENS Stainless Steel

RL COLLAR:

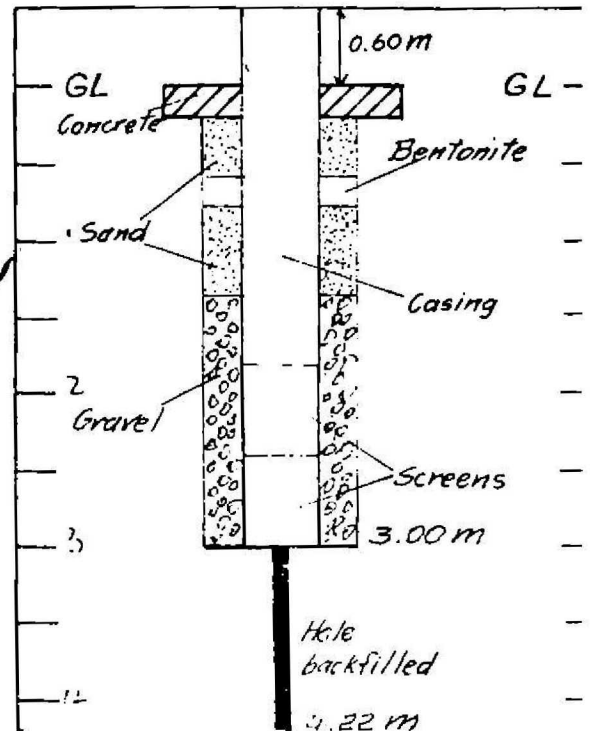
CASING TOP: 0.60 m above collar  
0.60 m above ground

DEPTH INTERVAL (m)	CASING DESCRIPTION
0.00 to 1.80	UPVC 140 mm dia.
1.80 to 3.00	SCREENS Stainless Steel 1.5 mm aperture 100 mm dia 2 x 0.6 m lengths

\* below ground level

## GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
1.40 to 3.00	3/4" crushed aggregate



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DEPARTMENT OF TRANSPORT AND WORKS  
WATER DIVISION

PROJECT : BMR/CASING SCHEDULE  
MAGELA DRILLING

BORE No: M22

R.N.:

LOCATION (map + grid): CAMILL 1:100,000 SHEET 5472

IN:

PREPARED BY: F.A. KELLY / BMR

MATERIAL TYPE : BLANK uPVC

SCREENS stainless steel

RL COLLAR;

CASING TOP: 0.40 m above ground  
0.50 m above collar

[illegible]

\* Below ground level

## GRAVEL PACK

DEPTH INTERVAL <sup>*</sup> (m)	DESCRIPTION
2.30 - 6.50	3/8" crushed aggregate

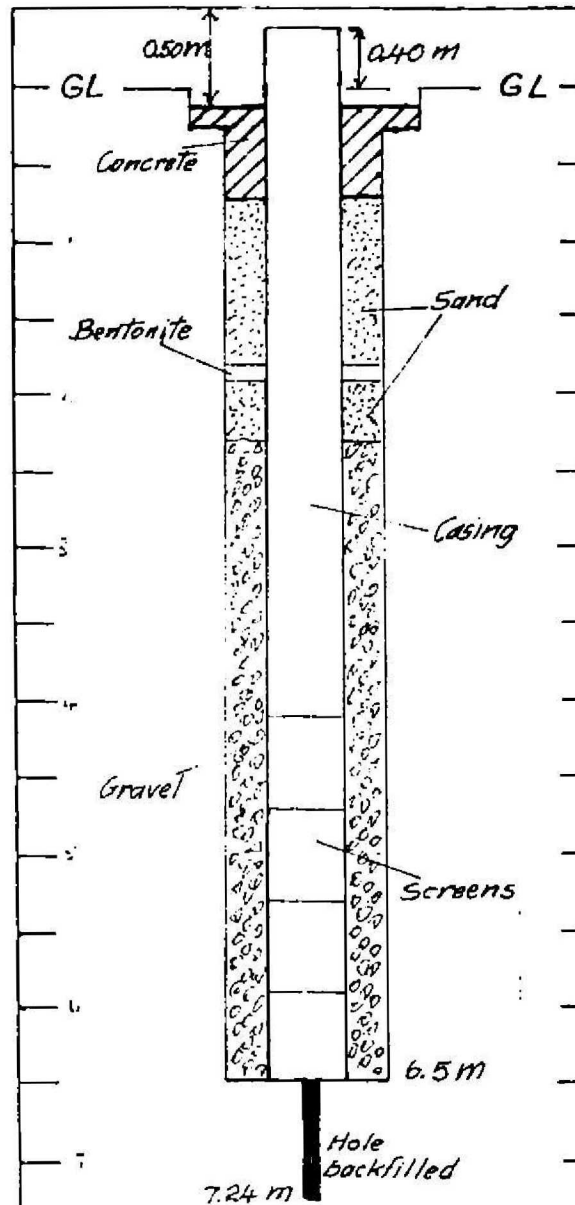
**PRELIMINARY, UNEDITED**

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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : BMR | MAGELA DRILLING

BORE No: M 23

LOCATION (map + grid) : CAPELL 1:100 000 SHEET 5472  
COORDS 701593

R.N.:

PREPARED BY: F.A. KELLY | BMR

I.N.:

MATERIAL TYPE : BLANK UPVC

SCREENS Stainless Steel

RL COLLAR:

CASING TOP:

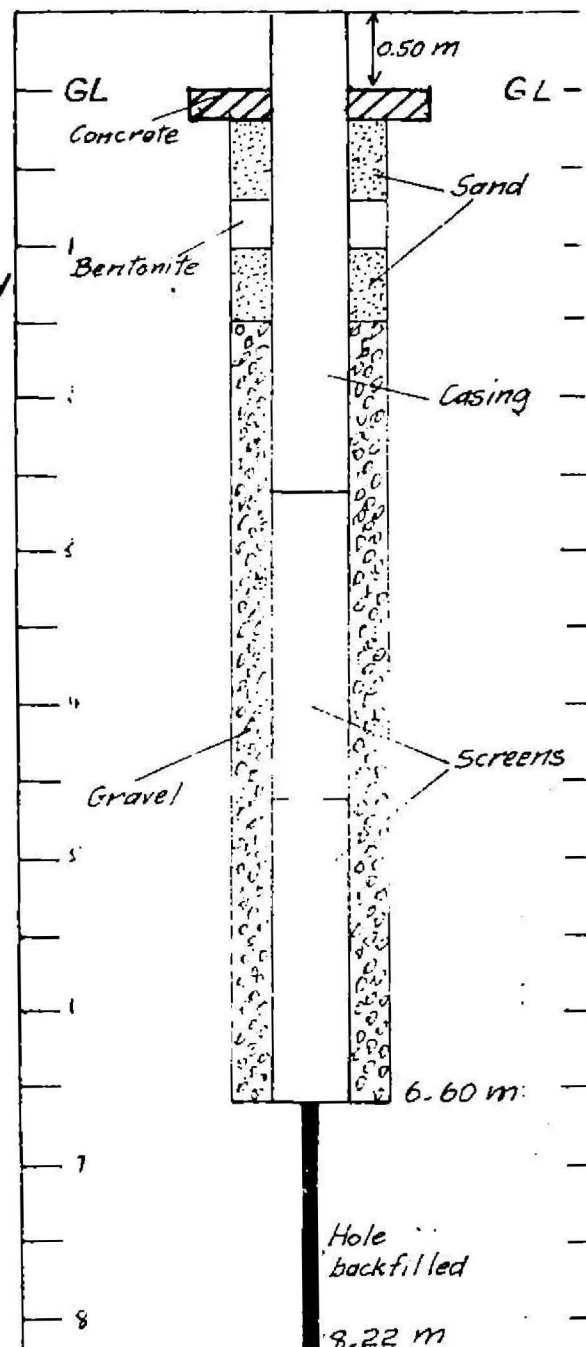
m above ground  
0.50 m above collar

DEPTH INTERVAL (m)	CASING DESCRIPTION
0.00 to 2.60	UPVC 140 mm dia
2.60 to 6.60	Screens 2 x 2 m x 1.5 m aperture 100 mm diam

\* Below ground level

GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
1.50 to 6.60	3/8" crushed aggregate



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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : BMR / CASING SCHEDULE  
MAGELA DRILLING

BORE No: M24

R.N.:

LOCATION (map + grid): LAHELL 1:100 000 SHEET 5472  
COORDS 670593

IN:

PREPARED BY: F.A. KELLY/BMR

MATERIAL TYPE: BLANK uPVC  
SCREENS: Seamless Steel

RL COLLAR:

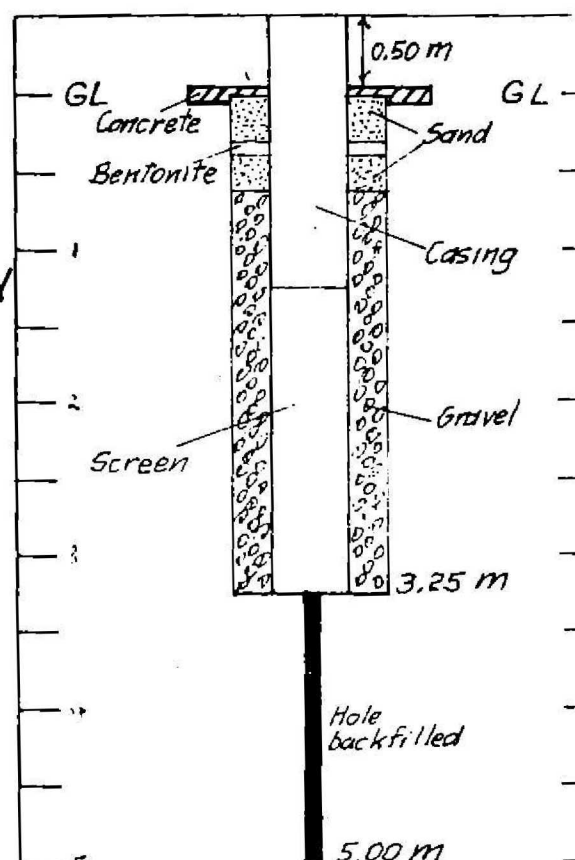
CASING TOP: m above ground  
0.30 m above collar

DEPTH INTERVAL (m)	CASING DESCRIPTION
0.00 to 1.25	uPVC 140 mm dia
1.25 to 3.25	Screens 1 x 2m x 15 mm aperture 100 mm dia

\* below ground level

## GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
0.63 - 3.25	3/8" crushed aggregate



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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : COOPER CASING SCHEDULE

BORE No: NI

R.N.:

LOCATION (map + grid) : DENDELI 1100000 SHEET 5573  
COORDS 175421

IN:

PREPARED BY: F.A. KELLY / BMR

MATERIAL TYPE : BLANK uPVC

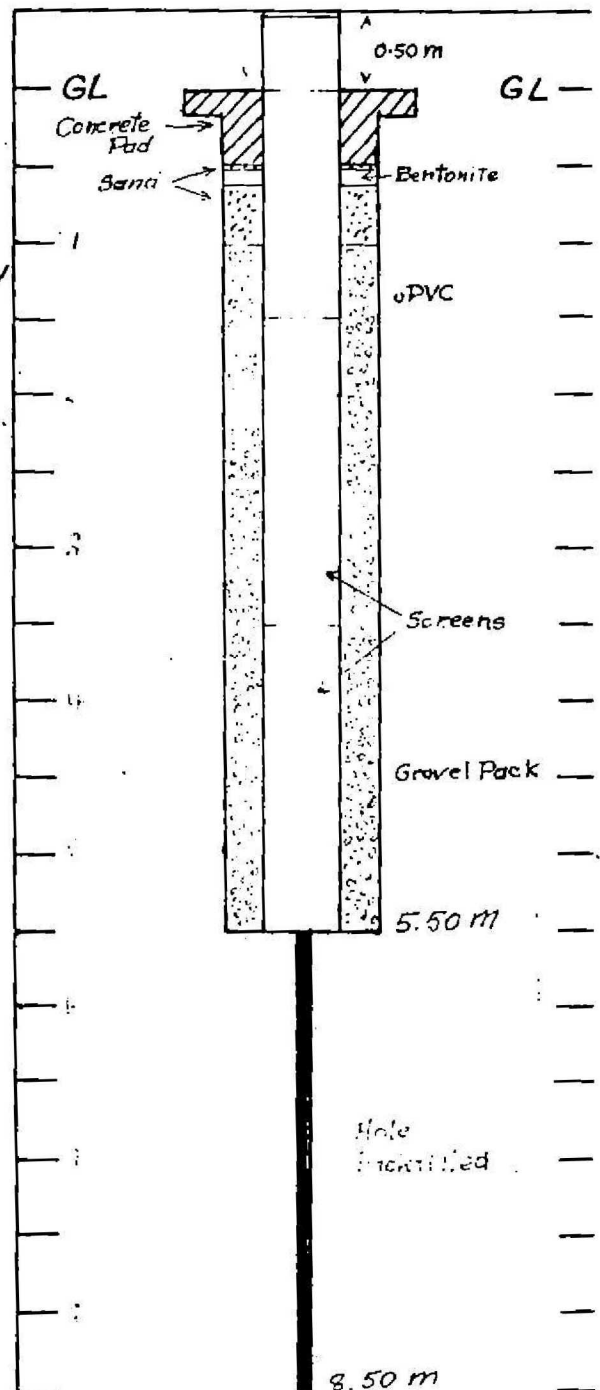
R/L COLLAR: SCREENS stainless steel

CASING TOP: m above ground  
0.50 m above collar

DEPTH INTERVAL (m)	CASING DESCRIPTION
0.00 TO 1.50	uPVC 140 mm dia
1.50 TO 5.50	SCREENS 2 x 2 m x 100 mm aperture 100 mm chain

## GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
1.0 TO 5.5	3/5" washed gravel



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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : *Cooper* CASING SCHEDULE

BORE No: *N 2.*

LOCATION (map + grid) : *OFIPELLI 1:100000 SHEET 5573*  
*Coords 172415*

R.N.:

PREPARED BY: *KELLY / B.M.R.*

IN:

MATERIAL TYPE : *BLANK upvc*  
*SCREENS St. Steel*

RL COLLAR:

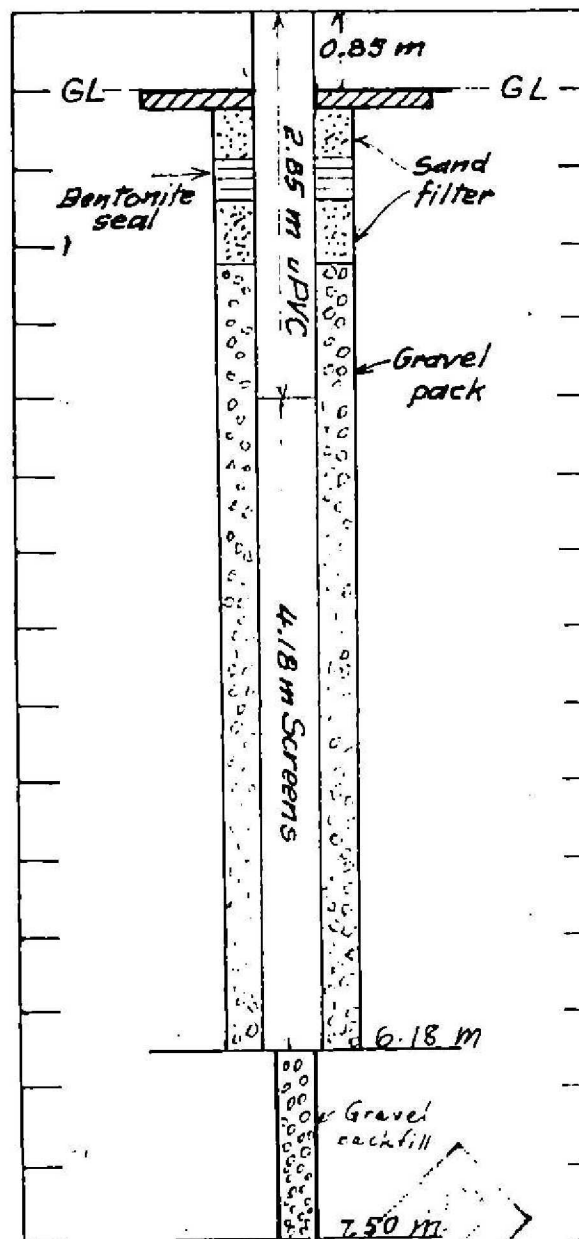
CASING TOP: *0.85 m above ground*  
*0.85 m above collar*

DEPTH INTERVAL (m) *	CASING DESCRIPTION
0 - 2.00	<i>upvc 140 mm diam</i>
2.00 - 6.18	<i>SCREENS</i> <i>2 x 2.00 x 1.5 mm aperture</i> <i>100 mm diam</i>

\* Below ground level

## GRAVEL PACK

DEPTH INTERVAL (m) *	DESCRIPTION
<i>1.12 - 6.18</i>	<i>3/8" crushed aggregate</i>



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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : COOPER CASING SCHEDULE

BORE No: N 3.

R.N.:

LOCATION (map + grid) : CENPELH 1100000 SHEET 5573  
CORDS 171417

I.N.:

PREPARED BY: F.A. KELLY / BMK

MATERIAL TYPE : BLANK uPVC

SCREENS : Stainless Steel

RL COLLAR :

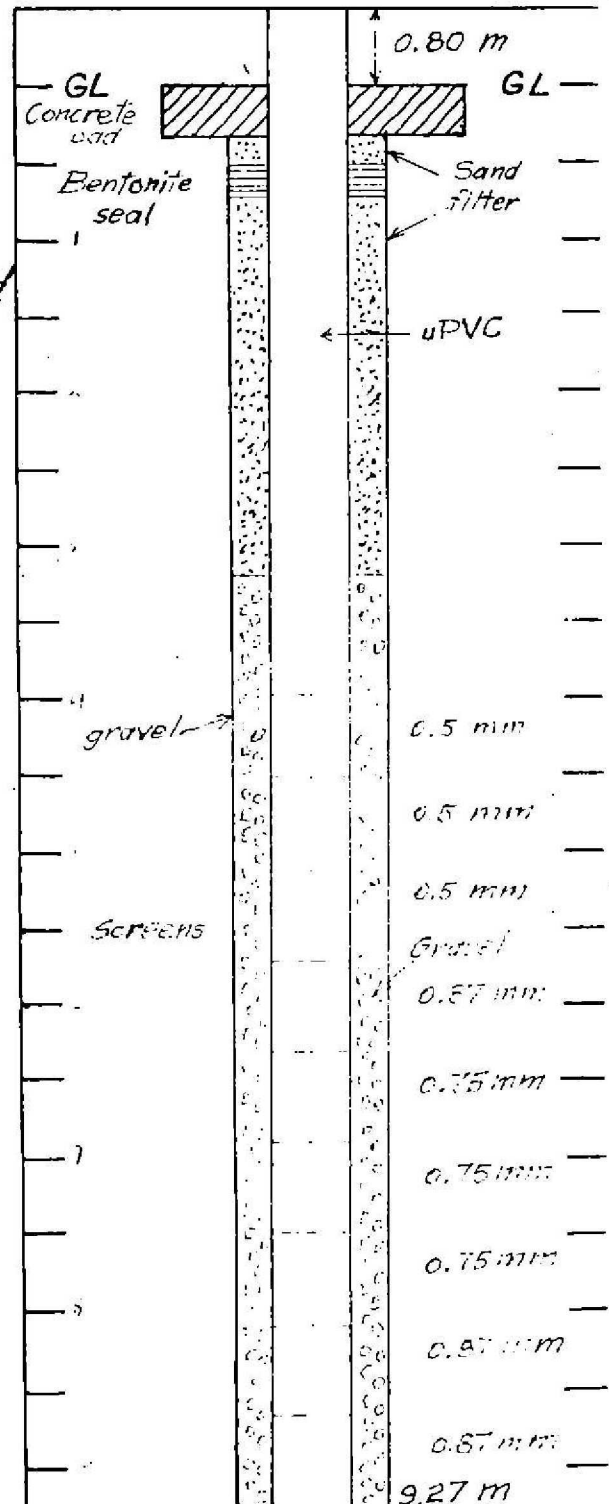
CASING TOP: 0.50 m above collar  
0.50 m above ground

DEPTH INTERVAL (m)	CASING DESCRIPTION
0.00 to 3.90	uPVC 100mm dia.
3.90 to 9.27	Stainless Steel screen
	3 x 0.6 m x 0.5 mm ap.
	3 x 0.6 m x 0.57 mm ap.
	3 x 0.6 m x 0.75 mm ap.
	(extra length due to weld joints)
	100 mm dia.

\* Below ground level

## GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
3.20 to 9.27	3/8" crushed aggregate



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DEPARTMENT OF TRANSPORT AND WORKS  
WATER DIVISION

PROJECT : COOPER CASING SCHEDULE

BORE No: N'4

R.N.:

LOCATION (map + grid) : CANPELLI 1100000 SHEET 5573

IN:

PREPARED BY: F. A. KELLY/BMR

Goords 166 396

MATERIAL TYPE : BLANK upvc  
SCREENS stainless steel

RL COLLAR:

CASING TOP:

m above ground  
0.60 m above collar

[illegible]

\* Below ground level

## GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
2.20 TO 5.00	3/4" crushed aggregate.

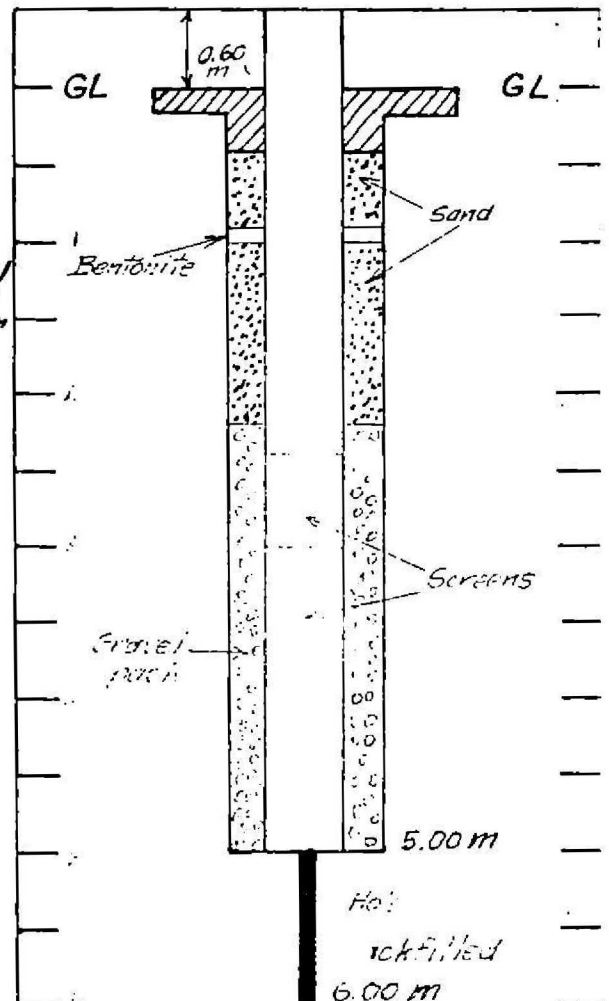
CONFIDENTIAL UNCLASSIFIED

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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : Cooper CASING SCHEDULE

BORE No: N5

R.N.:

LOCATION (map + grid) : DENPELLI 1:100 000 SHEET 5573  
COORDS 197413

IN:

PREPARED BY: KELLY / B.M.R.

MATERIAL TYPE : BLANK upvc

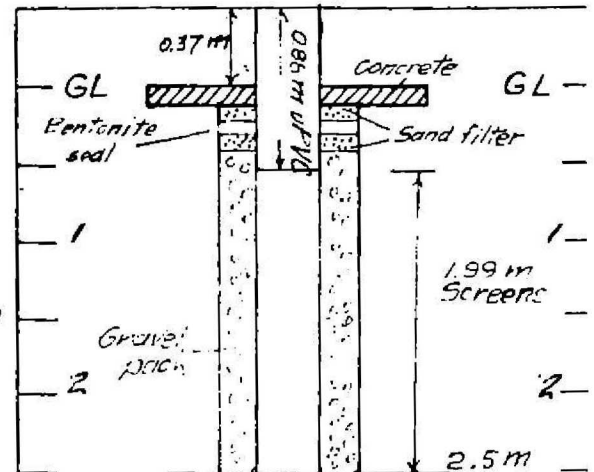
SCREENS st. steel

R.L. COLLAR

CASING TOP

0.37 m above collar  
above ground

DEPTH INTERVAL (m)	CASING DESCRIPTION
<u>0 - 0.51</u>	<u>140 mm upvc</u>
<u>0.51 - 2.50</u>	<u>1.2m x 5mm aperture</u> <u>100 mm diam</u>



GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
<u>0.47 - 2.50</u>	<u>3/8" crushed aggregate</u>

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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

BORE No: N6

PROJECT COOPER CASING SCHEDULE

R.N.:

LOCATION (map & grid): OENPELLI 1:100 000 SHEET 5573 I.N.:

PREPARED BY: KELLY / BMR COORDS 198413

MATERIAL TYPE: BLANK UPVC  
SCREENS st. steel

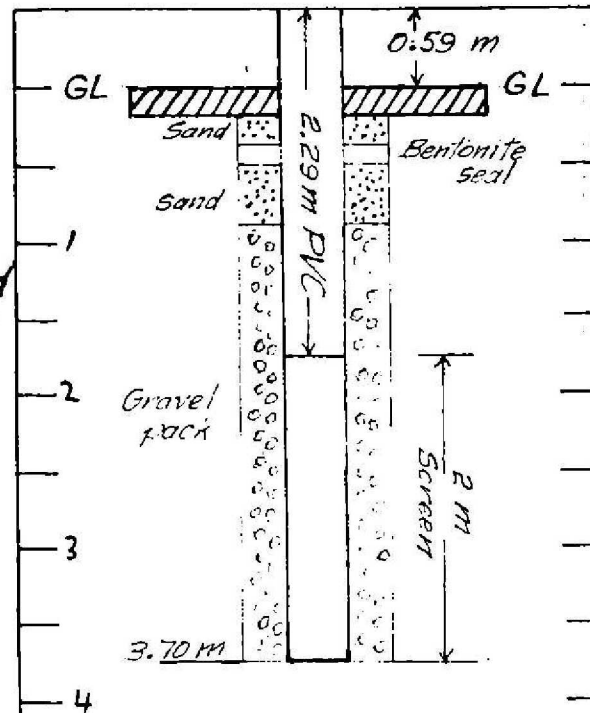
R.L. COLLAR:

CASING TOP: 0.59 m above ground  
0.59 m above collar

DEPTH INTERVAL (m)	CASING DESCRIPTION
0 - 1.7	140 mm UPVC
1.7 - 3.7	Screens
	1 x 2 m x 1.5 mm aperture
	100 mm diam

GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
0.9 - 3.7	3/8" crushed aggregate



Status 29.9.79. Water clear, ready for sampling. Concrete collar to be installed.

PREPARED BY: KELLY, BMR

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DEPARTMENT OF TRANSPORT AND WORKS  
WATER DIVISION

# PROJECT COOPER CASING SCHEDULE

BORE No: N7

R.N.:

LOCATION (map + grid): CENPELLI 1100000 SHEET 5573  
COORDS 199413

IN:

PREPARED BY: Kelly / BMR

MATERIAL TYPE : BLANK UPVC  
SCREENS st. steel

R.L. COLLAR:

CASING TOP: 0.50m <sup>in above ground</sup> above collar

[illegible]

## GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
0.6 - 3.1	3/8" crushed aggregate

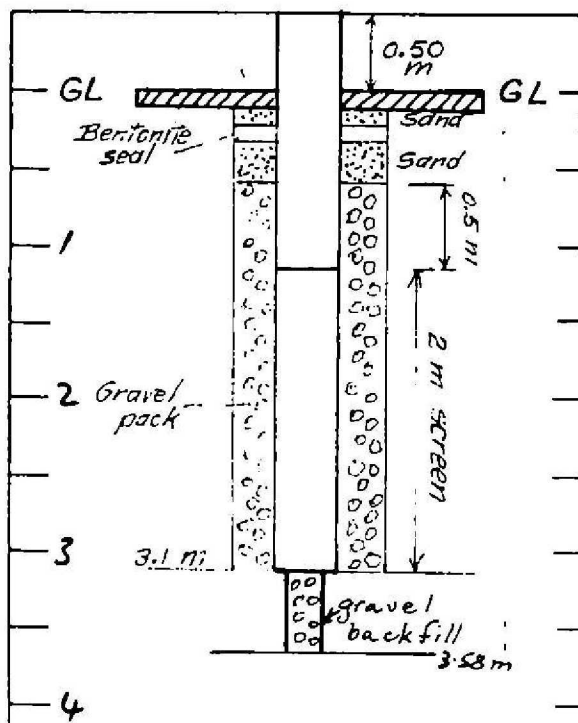
PRELIMINARY, UNEDITED

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Status 29 9 79. Water clear, ready for sampling. Concrete collar to be installed.

# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

BORE No: N8

PROJECT : COOPER CASING SCHEDULE

R.N.:

LOCATION (map + grid): CEN PELL 1:100 000 SHEET 5573

I.N.:

PREPARED BY: KELLY / B.M.R. COORDS 194388

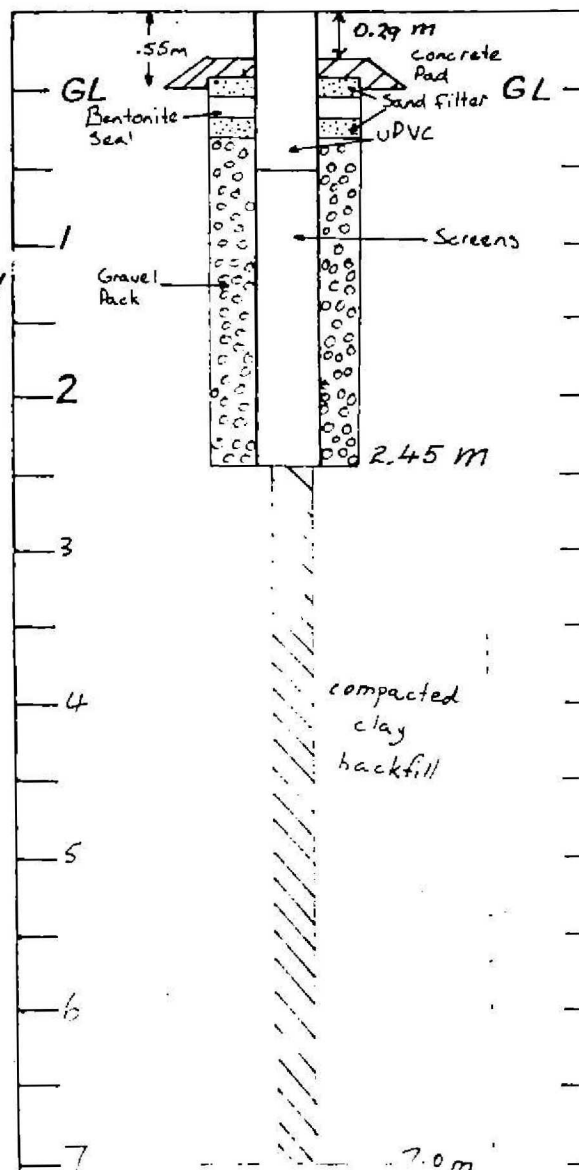
MATERIAL TYPE: BLANK UPVC

SCREENS st. steel.

R.L. COLLAR:

CASING TOP: 0.55 m above ground  
0.29 m above collar

DEPTH INTERVAL (m)	CASING DESCRIPTION
0 - 0.45 m	140 mm UPVC
0.45 - 2.45	SCREENS 1 x 3 m / 1.5 mm aperture 100 mm diam



## GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
0.20 - 2.45	3/8" crushed aggregate

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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT COOPER CASING SCHEDULE

BORE No: N.9

R.N.:

LOCATION (map = grid): OENPELLI 1:100 000 SHEET 5573  
COORDS 200385

I.N.:

PREPARED BY: F.A. Kelly P/L / BMR

MATERIAL TYPE: BLANK upvc  
SCREENS st. steel

RL COLLAR:

0.35 m above ground  
CASING TOP: 0.35 m above collar

DEPTH INTERVAL <sup>*</sup> (m)	CASING DESCRIPTION
0 - 5.75	140 mm upvc
5.75 - 9.75	SCREENS
	2 x 2m x 1.15mm aperture
	100mm diameter

\* Below ground level

## GRAVEL PACK

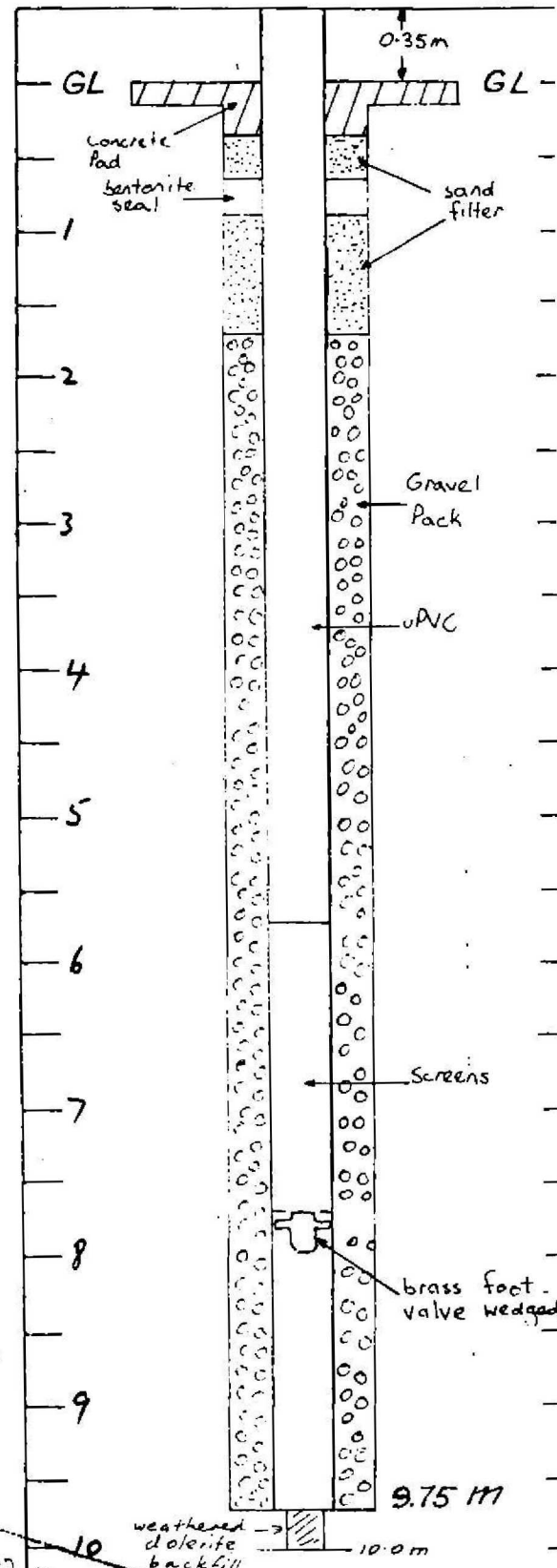
DEPTH INTERVAL <sup>*</sup> (m)	DESCRIPTION
1.6 - 9.75	3/8" crushed gravel

STATUS 28.9.79: Water clear, ready for sampling. Concrete collar to be installed.

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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : COOPER CASING SCHEDULE

BORE No: N10

R.N.:

LOCATION (map + grid) : CENPELLE 1:100 000 SHEET 5573  
COORDS 166394

I.N.:

PREPARED BY: F.A. KELLY/BMR

MATERIAL TYPE : BLANK uPVC  
SCREENS Stainless Steel

RL COLLAR:

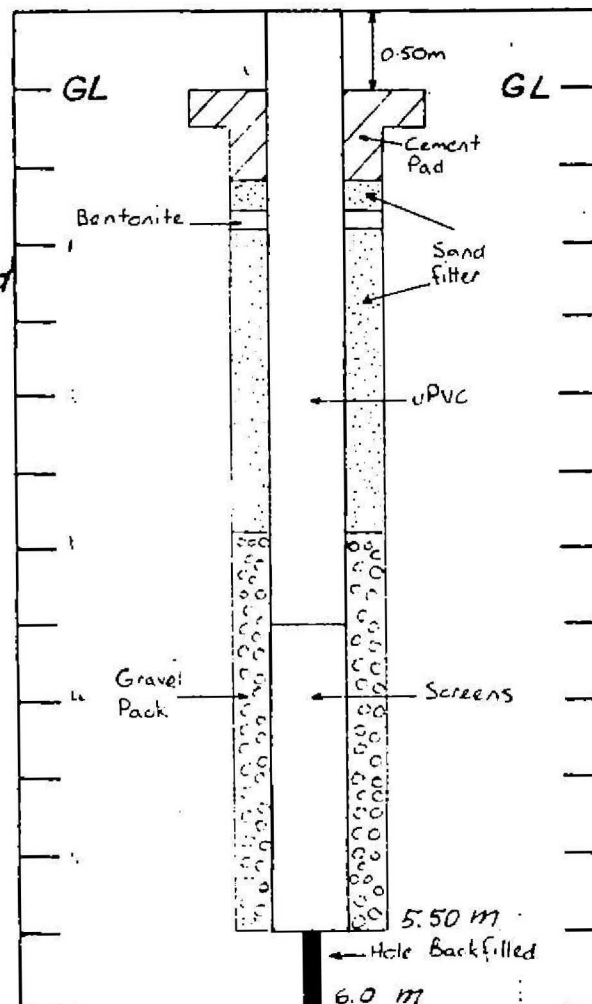
CASING TOP: 0.50 m above collar  
0.50 m above ground

DEPTH INTERVAL (m) *	CASING DESCRIPTION
0.00 to 3.50	uPVC 140 mm dia.
3.50 to 5.50	Screen 1 x 2 m x 1.5 mm aperture 100 mm dia.

\* Below ground level

## GRAVEL PACK

DEPTH INTERVAL (m) *	DESCRIPTION
2.90 to 5.50	5/8" crushed aggregate



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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT COOPER CASING SCHEDULE

BORE No: M11

R.N.:

LOCATION (map + grid): DENPELLI 1:100 000 SHEET 5573  
COORDS 171369

I.N.:

PREPARED BY: Kelly/BMR

MATERIAL TYPE: BLANK UPVC  
SCREENS st. steel

RL COLLAR

CASING TOP 0.53 m above ground  
0.53 m above collar

DEPTH INTERVAL (m)	CASING DESCRIPTION
0-2.07	140 mm upvc
2.07-6.07	SCREENS: 2 x 2m x 1.5 mm aperture 100 mm dia

\* Below ground level

GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
0.52-6.07	3/4" crushed aggregate

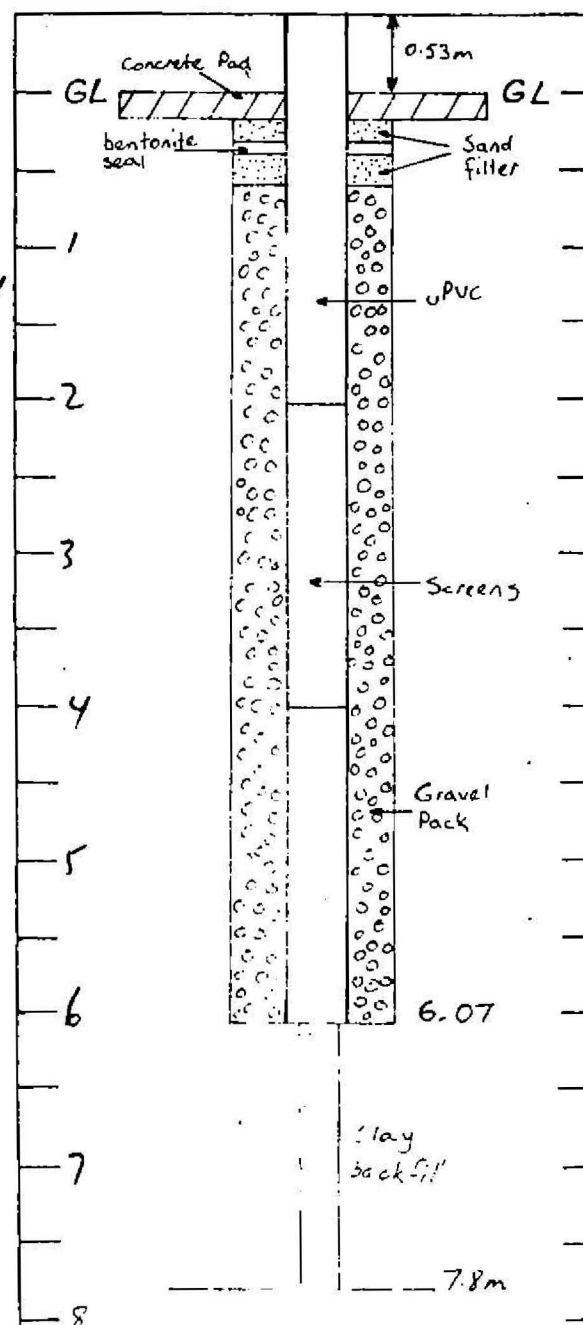
Status 30.9.79: Pumped 4hrs intermittently. Water improving but still not clear enough for sampling. Water has unpleasant odour. Concrete collar to be installed.

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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT **COOPER** CASING SCHEDULE

BORE No: **N12**

R.N.:

LOCATION (map = grid): **ORNIPELLI 1:100 000 SHEET 5573**  
**COORDS 179375**

IN:

PREPARED BY: **Kelly / BMR**

MATERIAL TYPE: **BLANK upvc**  
**SCREENS st steel**

RL COLLAR:

CASING TOP: **0.14 m above ground**  
**0.14 m above collar**

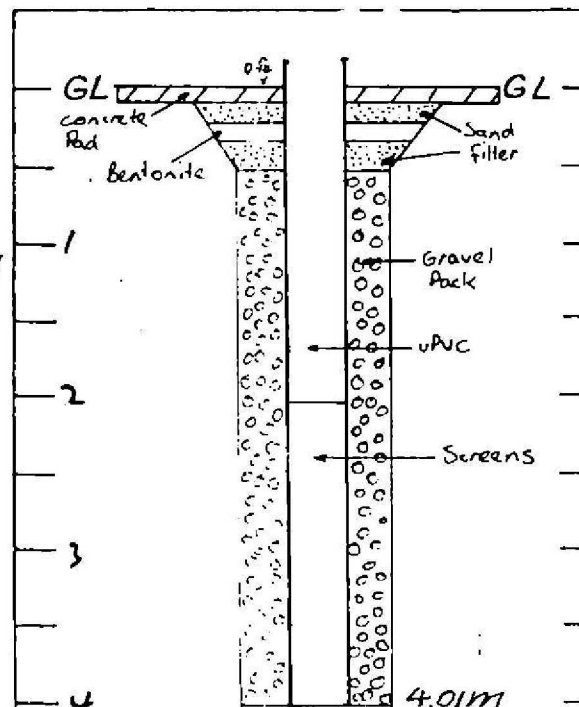
DEPTH INTERVAL <sup>*</sup> (m)	CASING DESCRIPTION
0 - 2.0	140 mm upvc
2.0 - 4.01	Screens:
	1.2m x 1.5mm aperture
	100mm diam

\* Below ground level

GRAVEL PACK

DEPTH INTERVAL <sup>*</sup> (m)	DESCRIPTION
0.5 - 4.0	3/8" crushed aggregate

status 30.9.79: No cap. No concrete collar.  
Requires more pumping before sampling



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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT COOPER CASING SCHEDULE

BORE No: N/13

LOCATION (map + grid): CEN PELLI 1:100 000 SHEET 5573  
COORDS 201386

RN:

IN:

PREPARED BY: F.A. Kelly / BMR

MATERIAL TYPE: BLANK upvc  
SCREENS stainless steel

RL COLLAR:

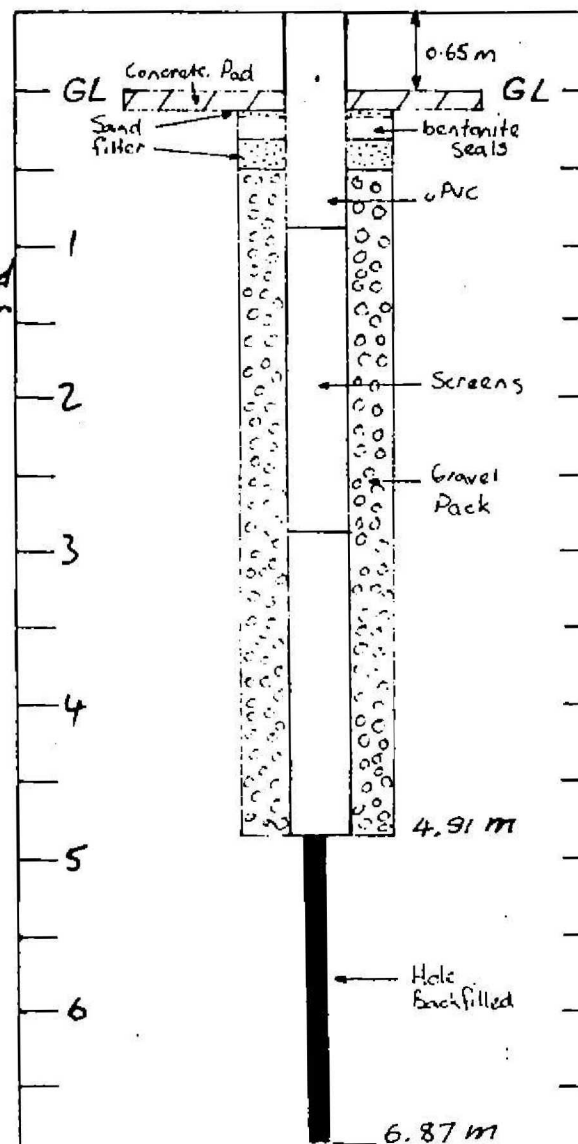
CASING TOP: 0.65 m above collar  
m above ground

DEPTH INTERVAL (m) *	CASING DESCRIPTION
0 - 0.91	140 mm upvc
0.91 - 4.91	Screens: 2 x 2 m x 1.15 m gradient 100 mm diam.

\* Below ground level

## GRAVEL PACK

DEPTH INTERVAL (m) *	DESCRIPTION
0.5 - 4.91	3/8" crushed aggregate (granite)



Status 28.9.79: Concrete collar to be installed. Requires bore cap.  
Water still turbid after approx 5 hrs intermittent pumping.  
Not ready for sampling.

29.9.79 3 hrs intermittent pumping, water still turbid, yield increasing  
Office use only 30.9.79 1 hr intermittent pumping, yield increasing  
water quality improving but not ready for sampling

DATE RECEIVED:

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Bore cap fitted

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# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

PROJECT : COOPER CASING SCHEDULE

BORE No: N14

LOCATION (map + grid) : OETIPELLI 1:100 000 SHEET 5513  
COORDS 193394

R.N.:

I.N.:

PREPARED BY: F. A. KELLY

MATERIAL TYPE : BLANK UPVC  
SCREENS : stainless steel

FL COLLAR

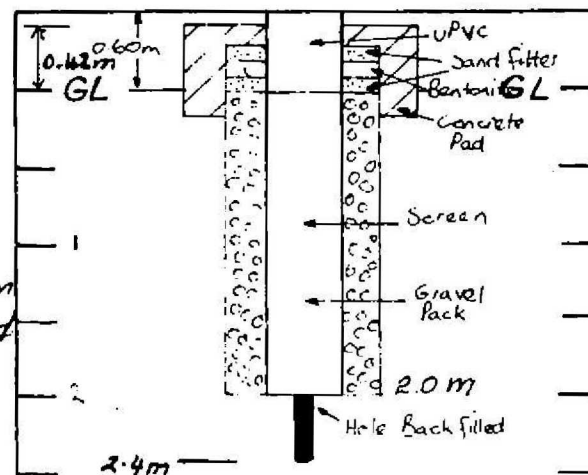
CASING TOP 0.18 m above collar  
0.60 m above ground

DEPTH INTERVAL (m)	CASING DESCRIPTION
0.00 TO 2.00	1.5 200 x 1.15 mm upvc casing 100 mm diameter

\* Below ground level

GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
0.00 TO 2.00	3/4 graded aggregate



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Record 1979/88

# DEPARTMENT OF TRANSPORT AND WORKS WATER DIVISION

BORE No: N15

PROJECT : COOPER CASING SCHEDULE

R.N.:

LOCATION (map + grid) : GENDALL 1:100 000 SHEET 5573  
COORDS 193451

I.N.:

PREPARED BY: F.A. KELLY/BMR

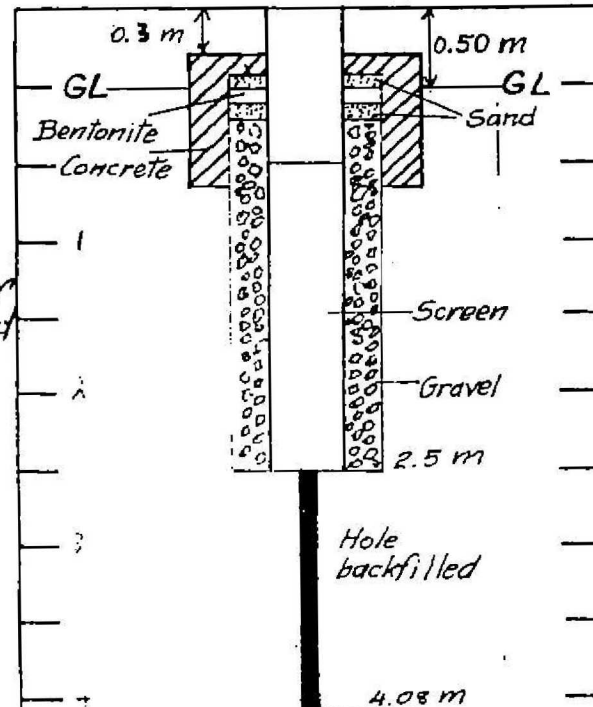
MATERIAL TYPE : BLANK UPVC  
SCREENS Stainless Steel

R.L. COLLAR:

CASING TOP: 0.30 m above collar  
0.50 m above ground

DEPTH INTERVAL (m)	CASING DESCRIPTION
0.00 TO 0.50	UPVC 140 mm dia
0.50 TO 2.50	Screen 1 x 2mm / 15mm aperture 100mm dia

\* Below ground level



**PRELIMINARY, UNEDITED**

GRAVEL PACK

DEPTH INTERVAL (m)	DESCRIPTION
0.27 TO 2.50	3/8" Crushed Aggregate

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Record 1379/88

## Appendix 4

### Tie Points, Coordinates and levels of Boreholes

#### Appendix 4

#### TIE POINTS FOR SURVEY OF BOREHOLES

<u>TRAVERSE OR BOREHOLE NO</u>	<u>TIE POINT COORDINATES AND LEVELS</u>
TRAVERSE 1	Special Mining Lease Boundary Marker No 27 for Ranger Uranium Mine Easting: 275839.10 Northing: 8595524.77 Level: 17.599 m ASL
TRAVERSE 2	Aerial Survey Target No 820 Easting: 269632.0 Northing: 8605882.0 Level: 7.05 m
TRAVERSE 3	As for Traverse 2
TRAVERSE 4	Special Mining Lease Boundary Marker No 52 for Ranger Uranium Mine Easting: 270675.00 Northing: 8593696.86 Level: 20.285 m
M26	Not Surveyed 1979
M27 and M28	Station No 3010 Surveyed by Pan Continental Easting: 271674.21 Northing: 8610570.84 No level given
M29	Station No 3139 Surveyed by Pan Continental Easting: 271201.54 Northing: 8610105.17 Level: 5.82 m ASL
M30 and M31	Station No 3146 Surveyed by Pan Continental Level: 14.97 m ASL
M32, M33 and M34	Station No 3061 Surveyed by Pan Continental Easting: 275665.53 Northing: 8607540.43 Level: 11.49 m ASL

**COORDINATES AND LEVELS FOR BORE-HOLES  
MAGELA CREEK CATCHMENT**

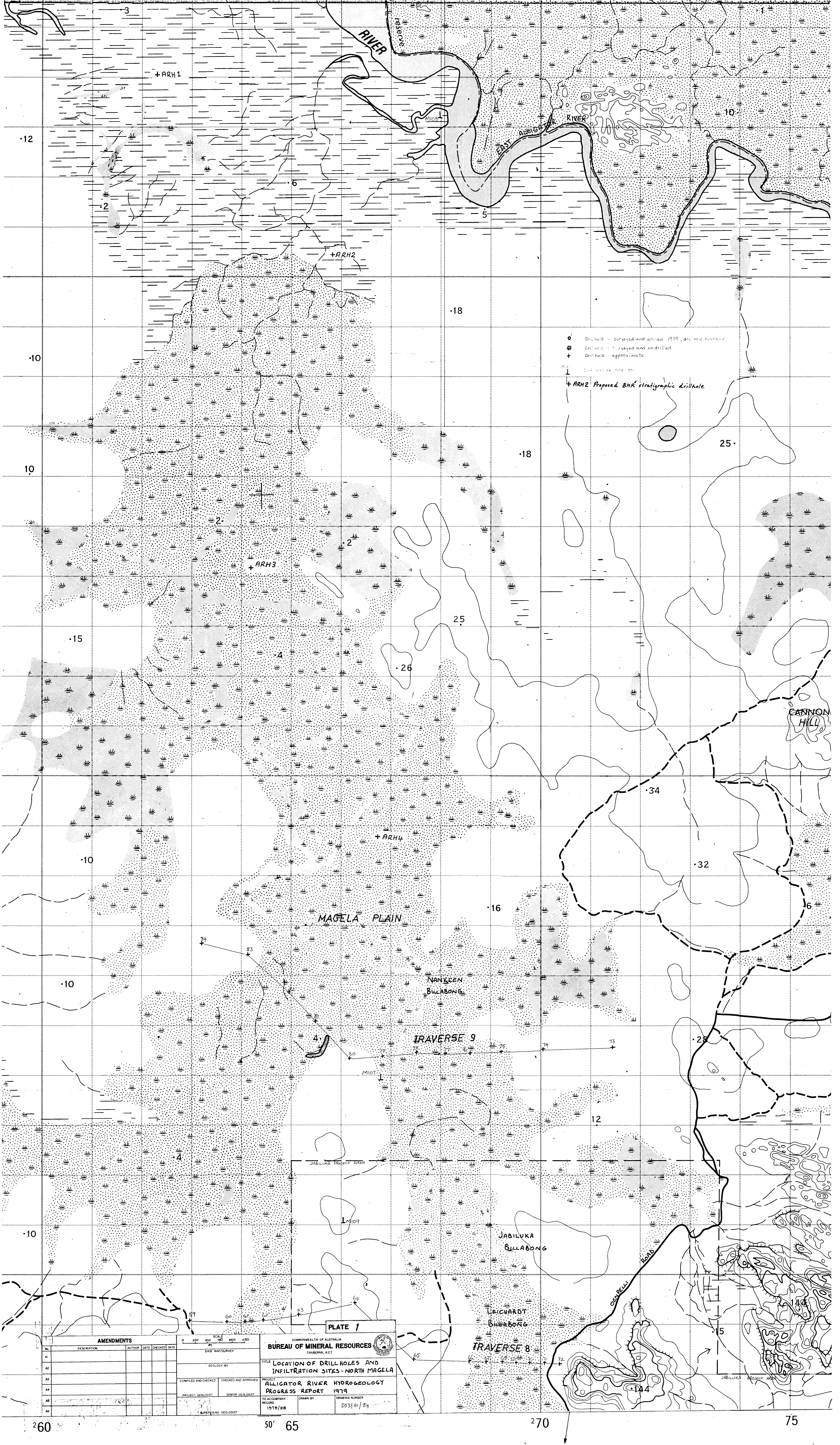
BMR BORE-HOLE NUMBERS	WATER DIVISION* WELL NUMBERS	GRID REFERENCE		HEIGHT (m) A.S.L.	REMARKS
<u>TRAVERSE 1</u>					
M1		276631.3	8595541.5	15.70	
M2		276719.8	8595622.3	16.16	
M3		276992.4	8596119.5	15.46	
M4		-	-	-	Not Surveyed 1979
M5		277862.5	8596870.3	18.05	
M6		-	-	-	Not Surveyed 1979
M7		-	-	-	Not Drilled or Surveyed 1979
M8		-	-	-	"
M9		-	-	-	"
<u>TRAVERSE 2</u>					
M10		270377.0	8603912.6	8.00	Not Drilled 1979
M11		270002.7	8603870.1	8.42	"
M12		269578.7	8603692.9	8.78	"
<u>TRAVERSE 3</u>					
M13		268879.6	8606020.8	13.38	Not Drilled 1979
M14		269299.4	8605961.2	6.01	"
M15		269562.0	8606042.9	6.79	"
M16		270065.6	8606056.6	7.38	"
M17		270720.9	8606174.4	7.02	"
M18		271270.2	8606226.2	13.46	"
<u>TRAVERSE 4</u>					
M19		270697.5	8593628.1	20.78	Not Drilled 1979
M20		270578.5	8593673.7	19.40	
M21		270467.9	8593716.2	19.49	
M22		270333.8	8593784.3	19.53	
M23		270158.3	8593868.8	19.90	
M24		269992.1	8593920.7	19.60	
M25		269825.9	8594004.3	22.38	
M26		-	-	-	Not Drilled or Surveyed 1979
M27		271720.2	8610665.2	-	Not Drilled, No R.L. found 1979
M28		271782.9	8610773.3	-	Not Drilled, No R.L. found 1979

\*Water Division well numbers not available.

BMR BORE-HOLE NUMBER	WATER DIVISION* WELL NUMBERS	GRID REFERENCE		HEIGHT (m) A.S.L.	REMARKS
		EASTING	NORTHING		
M29		271184.0	8610095.8	5.14	Not Drilled 1979
M30		-	-	4.61	Not Drilled 1979 No Grid Ref. found
M31		-	-	5.16	Not Drilled 1979 No Grid Ref. found 1979
<u>TRAVERSE 5</u>					
M32		276006.0	8607531.5	11.76	Not Drilled 1979
M33		276032.3	8607675.2	13.63	"
M34		276074.1	8607967.5	12.02	"

\*Water Division well numbers not available.



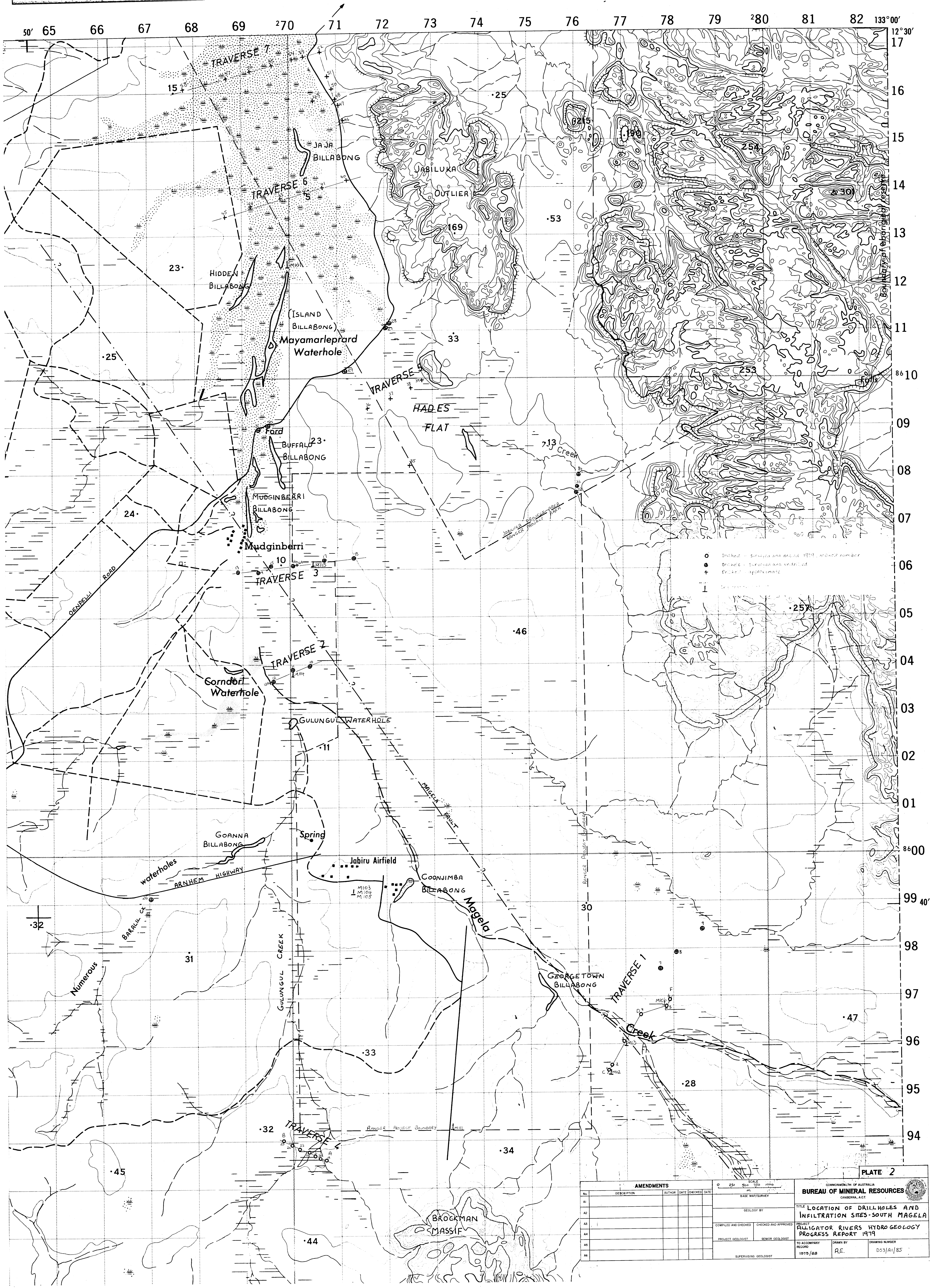


AMENDMENTS				
No.	DESCRIPTION	AUTHOR	DATE	CHECKED DATE
A1				
A2				
A3				
A4				
A5				
A6				

SCALE	0 250 500 1000 1500
BASE MAP/SURVEY	
GEOLOGY BY	
COMPILED AND CHECKED	CHECKED AND APPROVED
PROJECT GEOLOGIST	SENIOR GEOLOGIST
SUPERVISING GEOLOGIST	

COMMONWEALTH OF AUSTRALIA	
BUREAU OF MINERAL RESOURCES	
CANBERRA, A.C.T.	
TITLE LOCATION OF DRILL HOLES AND INFILTRATION SITES - NORTH MAGELA	
PROJECT ALLIGATOR RIVER HYDROGEOLOGY	
PROGRESS REPORT 1979	
TO ACCOMPANY RECORD 1979/88	DRAWN BY DS3/A1/84





AMENDMENTS					SCALE		BUREAU OF MINERAL RESOURCES	
NO.	DESCRIPTION	AUTHOR	DATE	CHECKED	DATE	0 250 500 1000 m	COMMONWEALTH OF AUSTRALIA CANBERRA, ACT	
A1							TITLE LOCATION OF DRILLHOLES AND INFILTRATION SITES-SOUTH MAGELA PROJECT ALLIGATOR RIVERS HYDRO GEOLOGY PROGRESS REPORT 1979	
A2								
A3							COMPILED AND CHECKED PROJECT GEOLOGIST	
A4								
A5							CHECKED AND APPROVED SENIOR GEOLOGIST	
A6								
						TO ACCOMPANY RECORD 1979/80		
						DRAWN BY A.E.		
						DRAWING NUMBER DS3/A/85		